



TOWN OF EXETER, NEW HAMPSHIRE

10 FRONT STREET • EXETER, NH • 03833-3792 • (603) 778-0591 • FAX 772-4709

www.exeternh.gov

EXETER PLANNING BOARD MEETING
JULY 13, 2023
7:00 Pm.

Nowak Room
10 Front Street
Exeter, N.H.

PLANNING BOARD CASE #23- 2

BLIND TIGER, LLC
(EXETER COUNTRY CLUB)

58 JADY HILL AVENUE

ADDITIONAL MATERIALS RECEIVED ON 7/7/23

OWNER

EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833

APPLICANT

BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

CIVIL ENGINEER

EMANUEL ENGINEERING, INC.
118 PORTSMOUTH AVENUE, SUITE A202
STRATHAM, NH 03885

WETLAND SCIENTIST

GOVE ENVIRONMENTAL SERVICES, INC.
8 CONTINENTAL DRIVE, BUILDING 2, UNIT H
EXETER, NH 03833

LAND SURVEYOR

JAMES VERRA & ASSOCIATES, INC.
101 SHATTUCK WAY, SUITE 8
NEWINGTON, NH 03801

ARCHITECT

DYNAMIC DESIGNS, P.C.
259 EAST FRANK STREET
BIRMINGHAM, MI 48009

LIGHTING

EXPOSURE LIGHTING
6 SCOTT ROAD, UNIT A
HAMPTON, NH 03842

LANDSCAPING

THORN AND THISTLE GARDENS
480 MIDDLE ROAD
BRENTWOOD, NH 03833

SITE PLAN FOR BLIND TIGER, LLC

EXETER TAX MAP 52 LOT 1
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

APPROVED BY THE TOWN OF EXETER PLANNING BOARD
CHAIRMAN _____ DATE _____

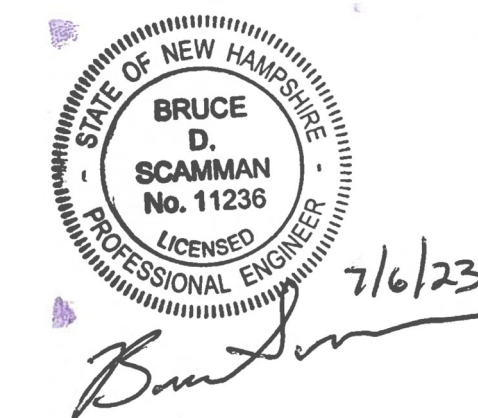
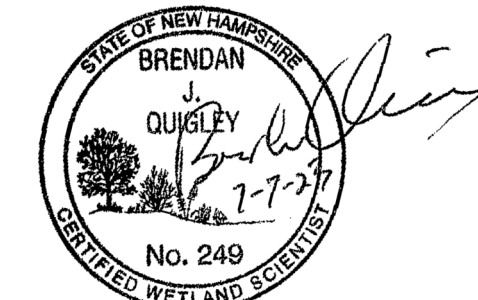


PROJECT DRAWING SET:

- COVER SHEET
- 1 & 2 OF 2 PLAN OF LAND IN EXETER, NH
- EXETER COUNTRY CLUB (BY GREAT BAY ENGINEERING, INC.)
- B1 ABUTTER KEY
- C1 EXISTING CONDITIONS
- C2 SITE PLAN
- C3A & C3B GRADING & DRAINAGE PLAN
- C4A UTILITIES PLAN
- C4B PROPOSED SEPTIC PROFILE
- C5 PAVING & CURBING PLAN
- D1 NOTES & EROSION CONTROL DETAILS
- D2 DRAINAGE DETAILS
- D3 UTILITY DETAILS
- D4 SITE DETAILS
- L01 LIGHTING PLAN (BY EXPOSURE LIGHTING)
- LA1 LANDSCAPING PLAN (BY THORN AND THISTLE GARDENS)

PROJECT LOCUS PLAN

1" = 1,000'



4	JULY 6, 2023	FOR APPROVAL	
3	MAY 23, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		

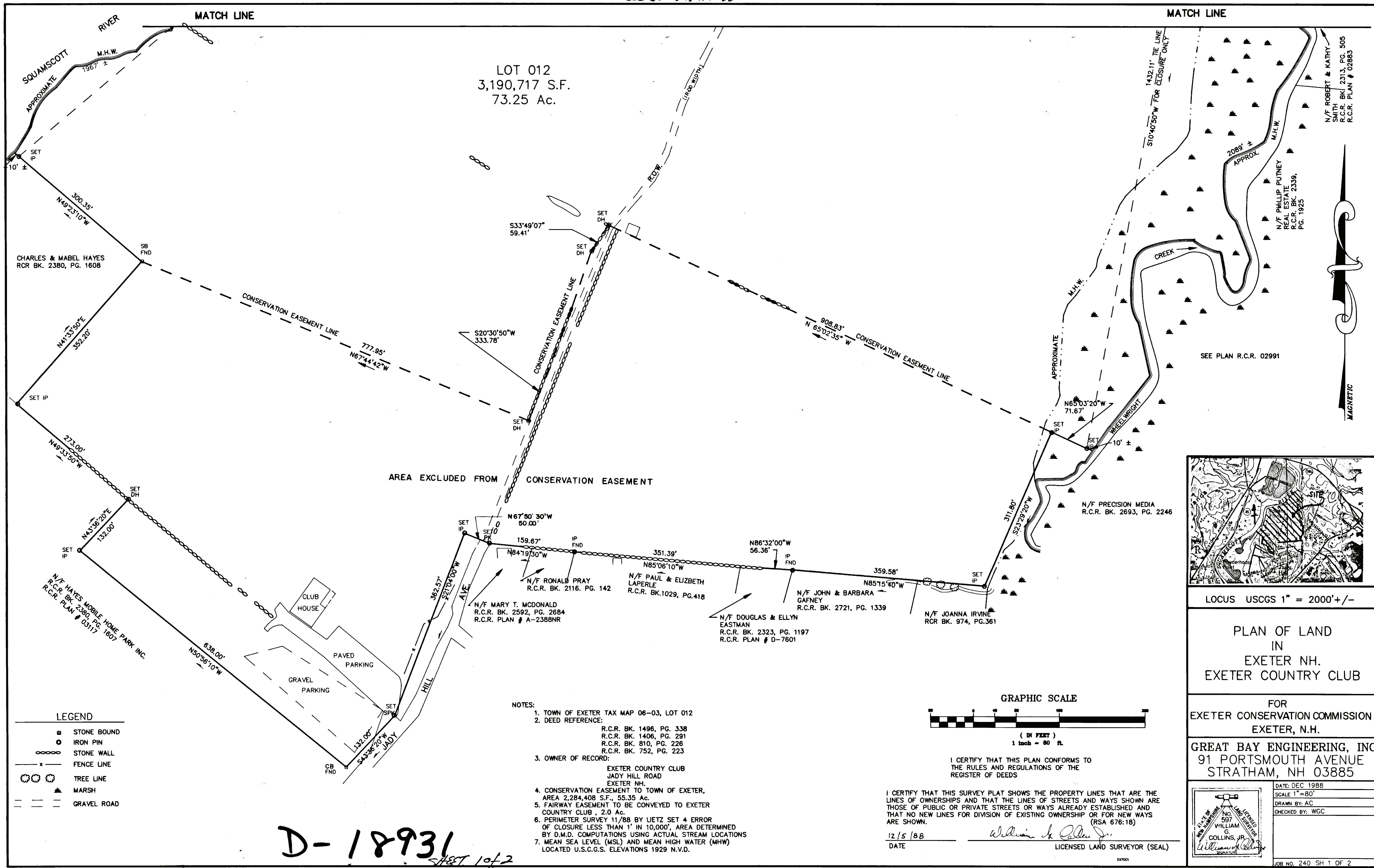
EMANUEL ENGINEERING
civil & structural consultants, land planners
118 PORTSMOUTH AVENUE, A202
STRATHAM, NH 03885
P: 603-772-4400 F: 603-772-4487
WWW.EMANUELENGINEERING.COM

CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

TITLE:
COVER SHEET
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

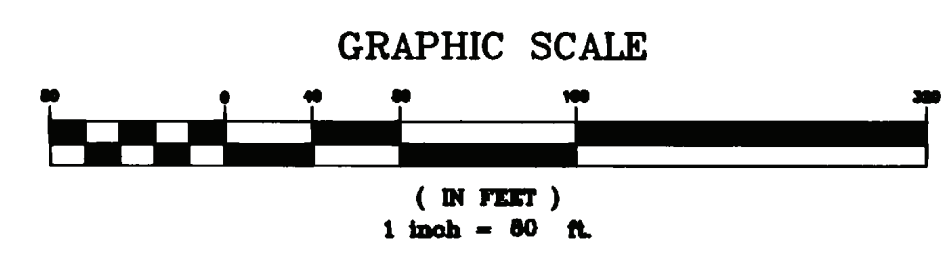
PROJECT:	SCALE:	SHEET:
21-157	AS SHOWN	COVER

DEC 30 3 45 PM '88
 64258
 DEC 30 3 45 PM '88



- LEGEND**
- STONE BOUND
 - IRON PIN
 - ⊖ STONE WALL
 - x- FENCE LINE
 - ⊙ TREE LINE
 - ▲ MARSH
 - - - GRAVEL ROAD

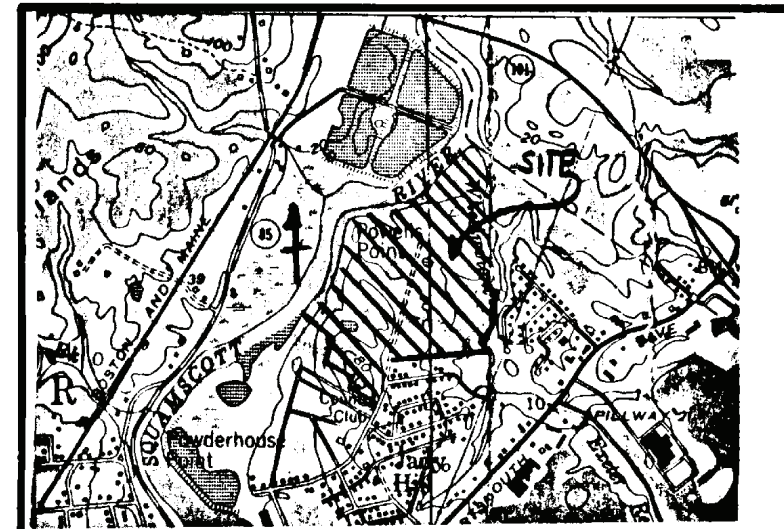
- NOTES:**
1. TOWN OF EXETER TAX MAP 06-03, LOT 012
 2. DEED REFERENCE:
 R.C.R. BK. 1496, PG. 338
 R.C.R. BK. 1406, PG. 291
 R.C.R. BK. 810, PG. 226
 R.C.R. BK. 752, PG. 223
 3. OWNER OF RECORD:
 EXETER COUNTRY CLUB
 JADY HILL ROAD
 EXETER NH.
 4. CONSERVATION EASEMENT TO TOWN OF EXETER, AREA 2,284,408 S.F., 55.35 AC.
 5. FAIRWAY EASEMENT TO BE CONVEYED TO EXETER COUNTRY CLUB, 2.0 AC.
 6. PERIMETER SURVEY 11/88, BY LIETZ SET 4 ERROR OF CLOSURE LESS THAN 1' IN 10,000', AREA DETERMINED BY D.M.D. COMPUTATIONS USING ACTUAL STREAM LOCATIONS
 7. MEAN SEA LEVEL (MSL) AND MEAN HIGH WATER (MHW) LOCATED U.S.C.G.S. ELEVATIONS 1929 N.V.D.



I CERTIFY THAT THIS PLAN CONFORMS TO THE RULES AND REGULATIONS OF THE REGISTER OF DEEDS

I CERTIFY THAT THIS SURVEY PLAT SHOWS THE PROPERTY LINES THAT ARE THE LINES OF OWNERSHIPS AND THAT THE LINES OF STREETS AND WAYS SHOWN ARE THOSE OF PUBLIC OR PRIVATE STREETS OR WAYS ALREADY ESTABLISHED AND THAT NO NEW LINES FOR DIVISION OF EXISTING OWNERSHIP OR FOR NEW WAYS ARE SHOWN. (RSA 676:18)

12/5/88 DATE
 [Signature] LICENSED LAND SURVEYOR (SEAL)



LOCUS USCGS 1" = 2000'+/-

**PLAN OF LAND
 IN
 EXETER NH.
 EXETER COUNTRY CLUB**

FOR
 EXETER CONSERVATION COMMISSION
 EXETER, N.H.

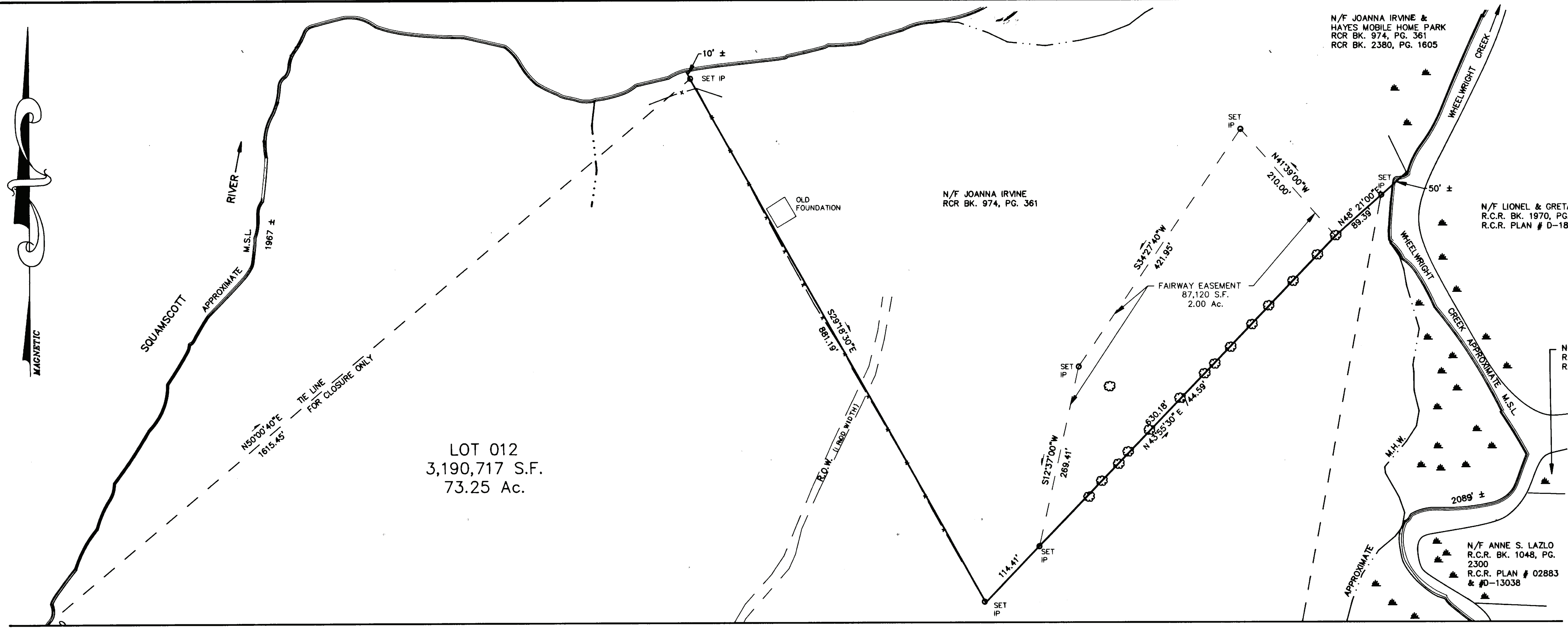
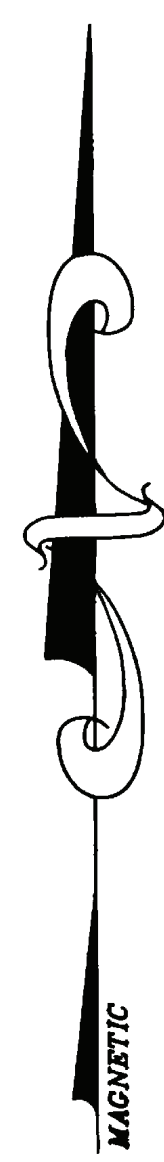
GREAT BAY ENGINEERING, INC
 91 PORTSMOUTH AVENUE
 STRATHAM, NH 03885

	DATE: DEC 1988
	SCALE 1"=80'
	DRAWN BY: AC
	CHECKED BY: WGC
JOB NO. 240 SH 1 OF 2	

D-18931
 SHEET 1 of 2

Dec 30 3:45 PM '88

Dec 30 3:45 PM '88



LOT 012
3,190,717 S.F.
73.25 Ac.

MATCH LINE

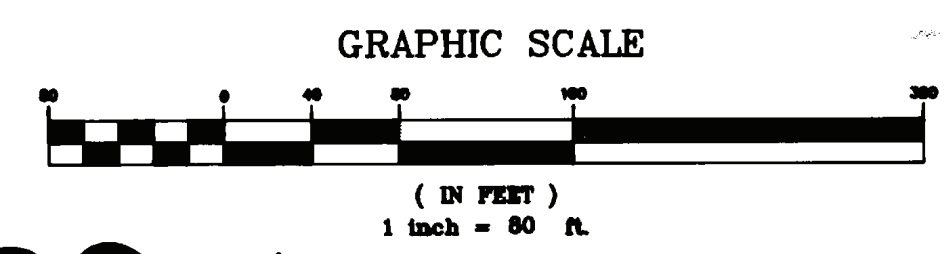
MATCH LINE

SEE SHEET 1 FOR LEGEND AND NOTES.

I CERTIFY THAT THIS PLAN CONFORMS TO THE RULES AND REGULATIONS OF THE REGISTER OF DEEDS

I CERTIFY THAT THIS SURVEY PLAT SHOWS THE PROPERTY LINES THAT ARE THE LINES OF OWNERSHIPS AND THAT THE LINES OF STREETS AND WAYS SHOWN ARE THOSE OF PUBLIC OR PRIVATE STREETS OR WAYS ALREADY ESTABLISHED AND THAT NO NEW LINES FOR DIVISION OF EXISTING OWNERSHIP OR FOR NEW WAYS ARE SHOWN. (RSA 676:18)

DATE 12/15/88
WILLIAM G. COLLINS, JR.
LICENSED LAND SURVEYOR (SEAL)

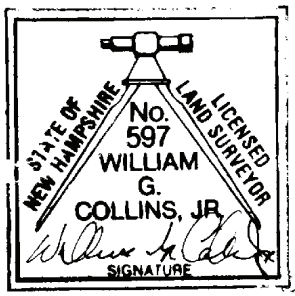


D-18931 Sheet 2 of 2

PLAN OF LAND
IN
EXETER NH.
EXETER COUNTRY CLUB

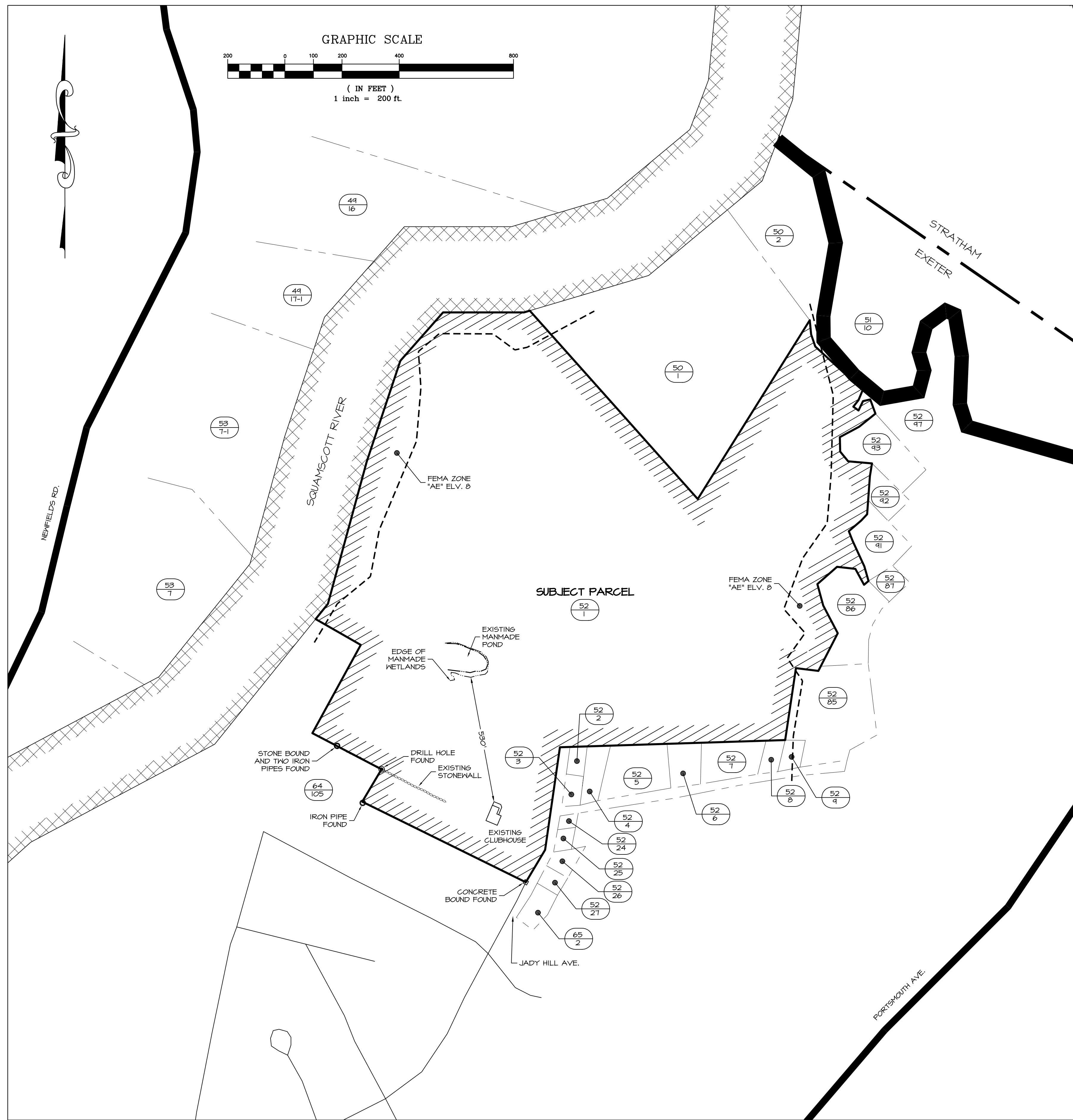
FOR
EXETER CONSERVATION COMMISSION
EXETER NH.

GREAT BAY ENGINEERING, INC
91 PORTSMOUTH AVENUE
STRATHAM, NH 03885



DATE: DEC 1988
SCALE: 1"=80'
DRAWN BY: AC
CHECKED BY: WGC

JOB NO. 240 SH 2 OF 2



SUBJECT PARCEL:

TAX MAP 52 LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 PG0290
USE: GOLF CRSE

ABUTTERS:

TAX MAP 49 LOT 16
RUSSELL F. FREDERICKSEN
11 NEWFIELDS ROAD
EXETER, NH 03833
RCRD BK3488 PG2807
USE: MULTI HSE

TAX MAP 49 LOT 17-1
ABIGAIL A. PHILLIPS
REVOCABLE TRUST
9 NEWFIELDS ROAD
EXETER, NH 03833
RCRD BK3502 PG1209
USE: SINGLE FAM

TAX MAP 50 LOT 1
TOWN OF EXETER
10 FRONT STREET
EXETER, NH 03833
RCRD BK2782 PG0389
USE: EXEMPT FARMLAND

TAX MAP 50 LOT 2
TOWN OF EXETER
10 FRONT STREET
EXETER, NH 03833
RCRD BK2782 PG0400
USE: MUNICIPAL

TAX MAP 51 LOT 10
LABONTE INVESTMENT REALTY LLC
355 ROUTE 125
BRENTWOOD, NH 03833
RCRD BK5561 PG1932
USE: RES ACNUD

TAX MAP 52 LOT 2
JANET A. CHECK
79 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK3241 PG1941
USE: SINGLE FAM

TAX MAP 52 LOT 3
MARIA GEORGE CARRASQUILLO
77 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK5985 PG0579
USE: SINGLE FAM

TAX MAP 52 LOT 4
MARSHMAN FAMILY TRUST
2 WEBSTER AVENUE
EXETER, NH 03833
RCRD BK6402 PG1467
USE: SINGLE FAM

TAX MAP 52 LOT 5
GREGORY MCCARTHY
4 WEBSTER AVENUE
EXETER, NH 03833
RCRD BK5383 PG1401
USE: SINGLE FAM

TAX MAP 52 LOT 6
DOUGLAS B. EASTMAN
12 WEBSTER AVENUE
EXETER, NH 03833
RCRD BK3172 PG0776
USE: SINGLE FAM

TAX MAP 52 LOT 7
JACQUES P. WAGEMAKER
14 WEBSTER AVENUE
EXETER, NH 03833
RCRD BK5517 PG0405
USE: SINGLE FAM

TAX MAP 52 LOT 8
TOWN OF EXETER
10 FRONT STREET
EXETER, NH 03833
RCRD BK2887 PG2683
USE: MUNICIPAL

(ABUTTERS: CONTINUED):

TAX MAP 52 LOT 9
TOWN OF EXETER
10 FRONT STREET
EXETER, NH 03833
RCRD BK3680 PG2336
USE: MUNICIPAL

TAX MAP 52 LOT 24
LEON N. MORSE
73 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK2533 PG0786
USE: SINGLE FAM

TAX MAP 52 LOT 25
MICHAEL S. JOHNSON
71 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK5497 PG0681
USE: SINGLE FAM

TAX MAP 52 LOT 26
TIMOTHY GAGNON
69 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK5601 PG2630
USE: SINGLE FAM

TAX MAP 52 LOT 27
THOMAS NASH
67 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK5954 PG1279
USE: SINGLE FAM

TAX MAP 52 LOT 85
ARUBA CAPITAL
PO BOX 1540
EXETER, NH 03833
RCRD BK4988 PG1827
USE: OFFICE BLD

TAX MAP 52 LOT 86
TRACY J. MIDDLETON
FAMILY TRUST
7 DOWNING COURT
EXETER, NH 03833
RCRD BK6247 PG2422
USE: SINGLE FAM

TAX MAP 52 LOT 87
CLIVE TOMLINSON
1803 TUALATIN STREET
ST. HELENS, OREGON 97051
RCRD BK5873 PG0742
USE: SINGLE FAM

TAX MAP 52 LOT 91
BARRY W SPRACKLIN
2 MELODY LANE
EXETER, NH 03833
RCRD BK2921 PG1464
USE: SINGLE FAM

TAX MAP 52 LOT 92
WILLIAM C. UNGER
1 MELODY LANE
EXETER, NH 03833
RCRD BK5325 PG1379
USE: SINGLE FAM

TAX MAP 52 LOT 93
ANNE S. LASZLO
27 ALLEN STREET
EXETER, NH 03833
RCRD BK2300 PG1084
USE: SINGLE FAM

(ABUTTERS: CONTINUED):

TAX MAP 52 LOT 97
TOWN OF EXETER
10 FRONT STREET
EXETER, NH 03833
RCRD BK2828 PG1102
USE: MUNICIPAL

TAX MAP 53 LOT 7
CATHLEEN A. TOOMEY
REVOCABLE TRUST
1 NEWFIELDS ROAD
EXETER, NH 03833
RCRD BK6361 PG1158
USE: SINGLE FAM

TAX MAP 53 LOT 7-1
CARPE DIEM TRUST
5 NEWFIELDS ROAD
EXETER, NH 03833
RCRD BK5828 PG1557
USE: SINGLE FAM

TAX MAP 64 LOT 105
HAYES MOBILE HOME PARK INC
56 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK3178 PG0286
USE: SINGLE FAM

TAX MAP 65 LOT 2
DANIEL W. CHARTRAND
63 JADY HILL AVENUE
EXETER, NH 03833
RCRD BK5319 PG1110
USE: SINGLE FAM

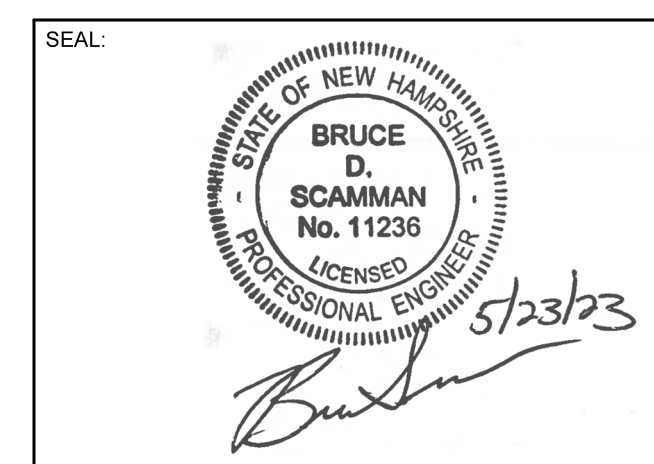
NOTES:

- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 PG0290
- THE INTENT OF THIS PLAN IS TO SHOW ALL THE ABUTTERS AND THEIR INFORMATION OF THE SUBJECT PARCEL LOCATED AT EXETER, NH TAX MAP 52, LOT 1. IT IS ALSO THE INTENT TO SHOW THE SURVEYED BOUNDS OF THE SUBJECT PARCEL THAT ARE NOT IN THE VICINITY OF THE PROPOSED IMPROVEMENTS ON SITE. ONLY A PORTION OF THE BOUNDARY WAS SURVEYED. THIS IS NOT A BOUNDARY PLAN.
- PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
- A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL. 0) AND "X"; REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
- FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID; NAVD 1988.
- WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DEPICTED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
- THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.

WETLAND NOTES:

- WETLANDS WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES INC. IN SPRING 2022 UTILIZING THE FOLLOWING STANDARDS:
- REGIONAL SUPPLEMENT TO THE CORPUS OF ENGINEERS WETLAND DELINEATION MANUAL, NORTH-CENTRAL AND NORTHEAST REGION, (VERSION 2.0) JANUARY 2012, U.S. ARMY CORPUS OF ENGINEERS.
 - FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND DELINEATING HYDRIC SOILS, VERSION 0.2, UNITED STATES DEPARTMENT OF AGRICULTURE (2010).
 - NEW ENGLAND HYDRIC SOILS TECHNICAL COMMITTEE, 2020 VERSION 4, FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION, LOVELL, MA, U.S. ARMY CORPUS OF ENGINEERS NATIONAL WETLAND PLANT LIST, VERSION 3.5. (2020)

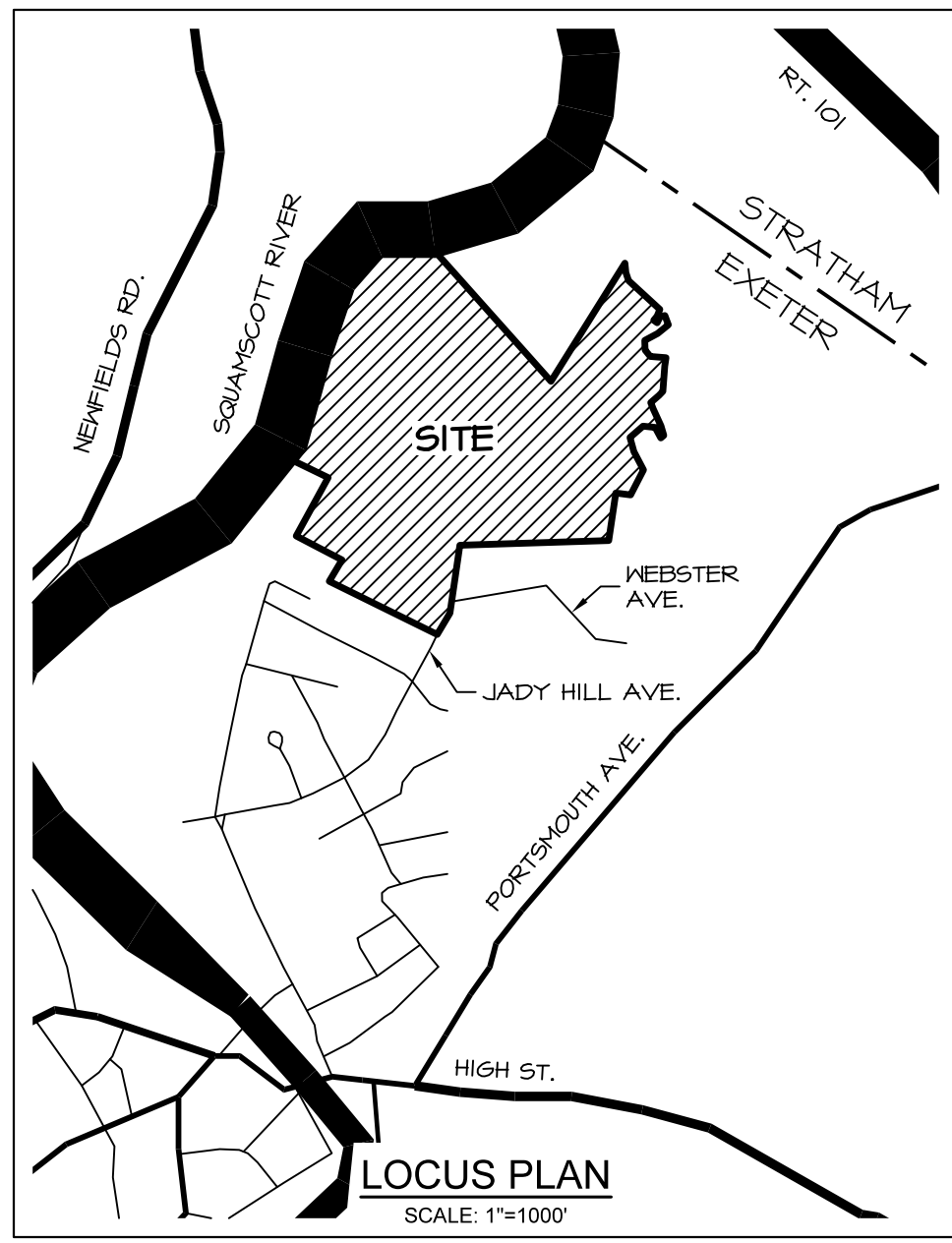
3	MAY 23, 2023	FOR APPROVAL	
2	APR 20, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK:
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		



CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

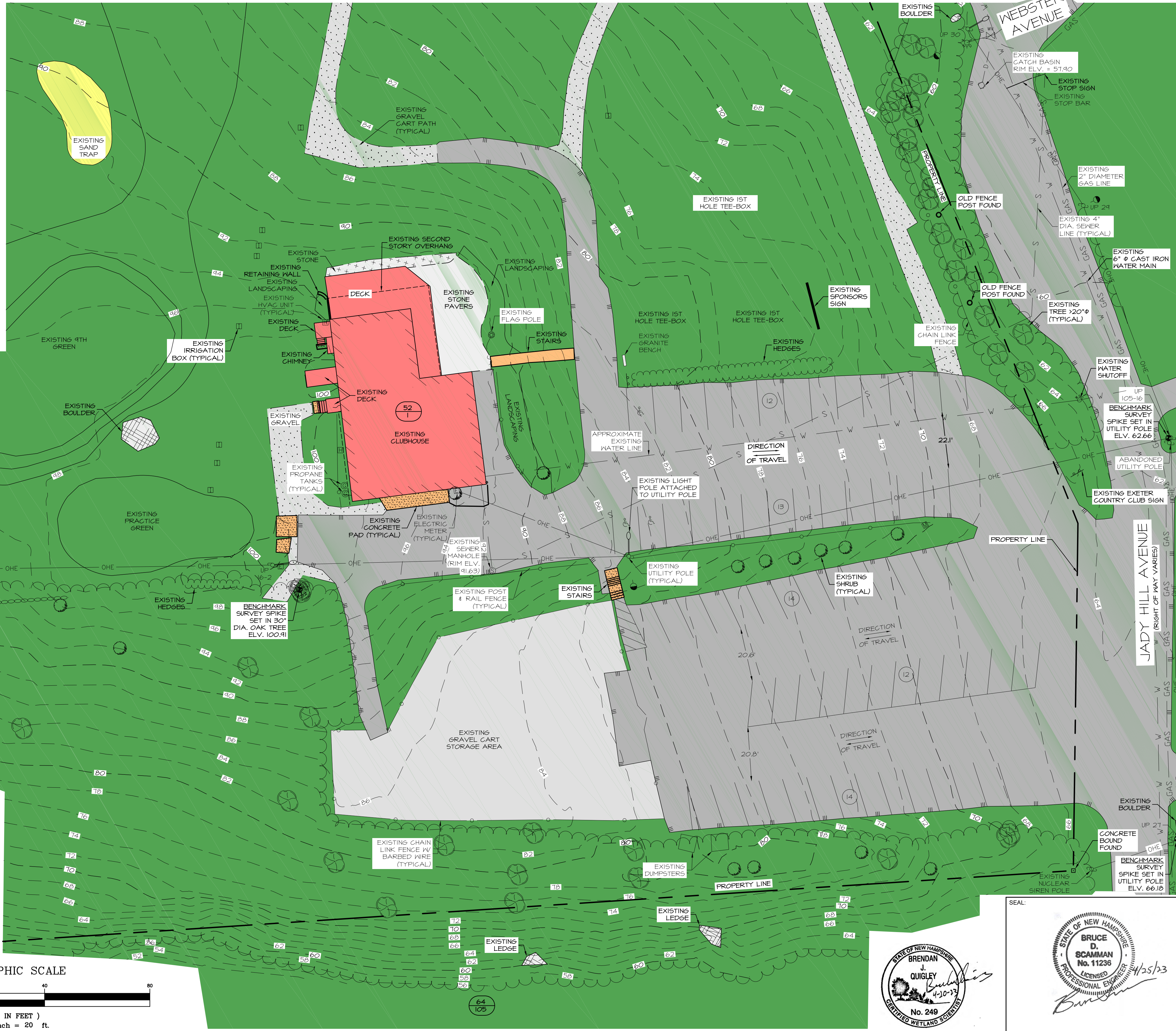
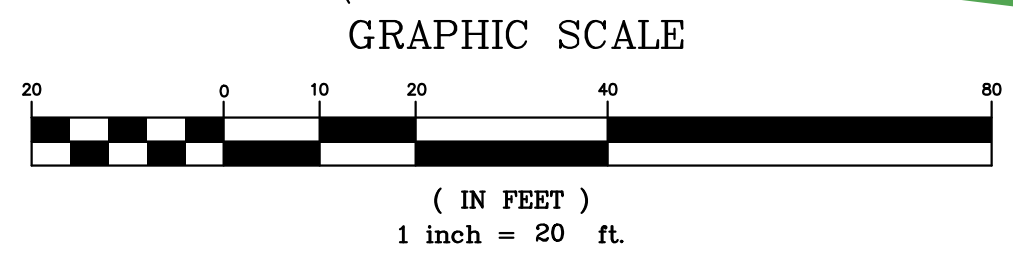
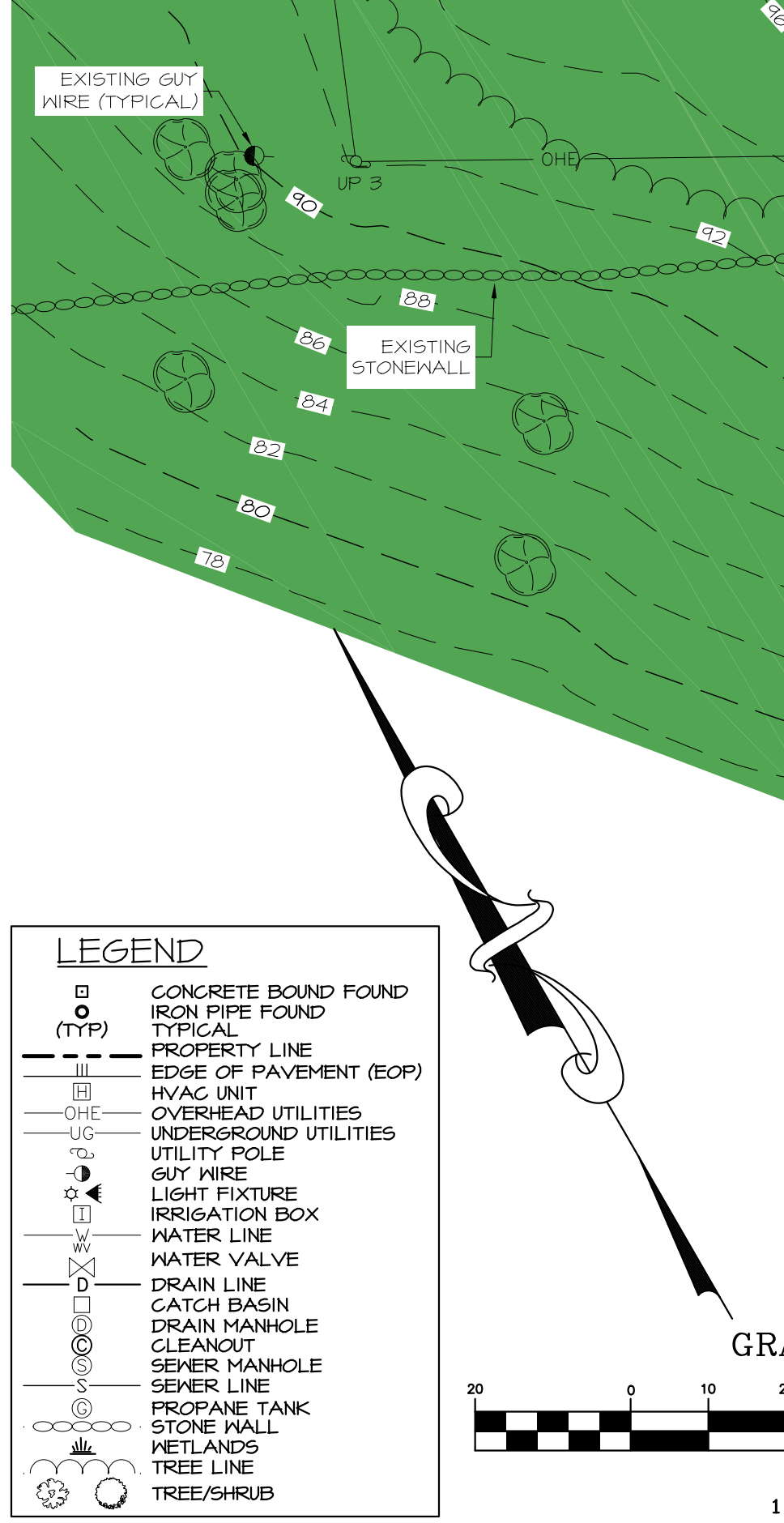
TITLE:
ABUTTER KEY
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=200'	B1



WETLAND NOTES:

- WETLANDS WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022 UTILIZING THE FOLLOWING STANDARDS:
- REGIONAL SUPPLEMENT TO THE CORPUS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION, (VERSION 2.0) JANUARY 2012, U.S. ARMY CORPS OF ENGINEERS.
 - FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND DELINEATING HYDRIC SOILS, VERSION 8.2, UNITED STATES DEPARTMENT OF AGRICULTURE (2016).
 - NEW ENGLAND HYDRIC SOILS TECHNICAL COMMITTEE, 2020 VERSION 4, FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION, LOWELL, MA, U.S. ARMY CORPS OF ENGINEERS NATIONAL WETLAND PLANT LIST, VERSION 3.5, (2020)



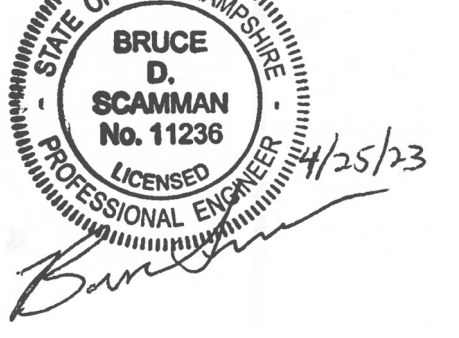
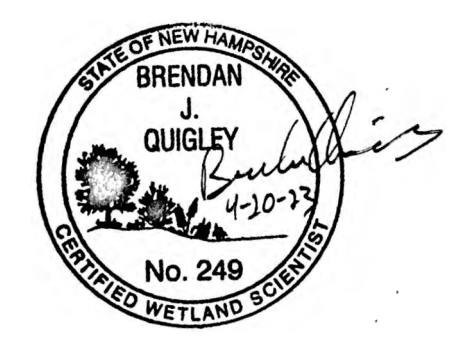
- NOTES:**
- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 P60240
 - THE INTENT OF THIS PLAN IS TO SHOW THE EXISTING CONDITIONS AND TOPOGRAPHY FOR EXETER, NH TAX MAP 52, LOT 1.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL 8) AND "X", REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
 - FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID; NAVD 1988.
 - WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DEPICTED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
 - PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
 - ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
 - THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
 - BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 12 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
 - ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.
 - THE LOCATION OF THE EXISTING WATER AND GAS LINES WITHIN THE TOWN RIGHT OF WAY WERE LOCATED PER REF. PLAN #3.

- REFERENCE PLANS:**
- "PLAN OF LAND IN EXETER, NH, EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC, DATED DECEMBER 1988; SCALE: 1"=80'; RCRD D-18431.
 - "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE, EXETER COUNTRY CLUB" BY MILLENNIUM ENGINEERING, INC, DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
 - "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-5) BY WRIGHT-PEIRCE (UNDATED); SCALE: 1"=20'; NOT RECORDED.

2	APR 20, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN:	JJM	DESIGN:	JJM
CHECKED:	BDS	CHECKED:	BDS

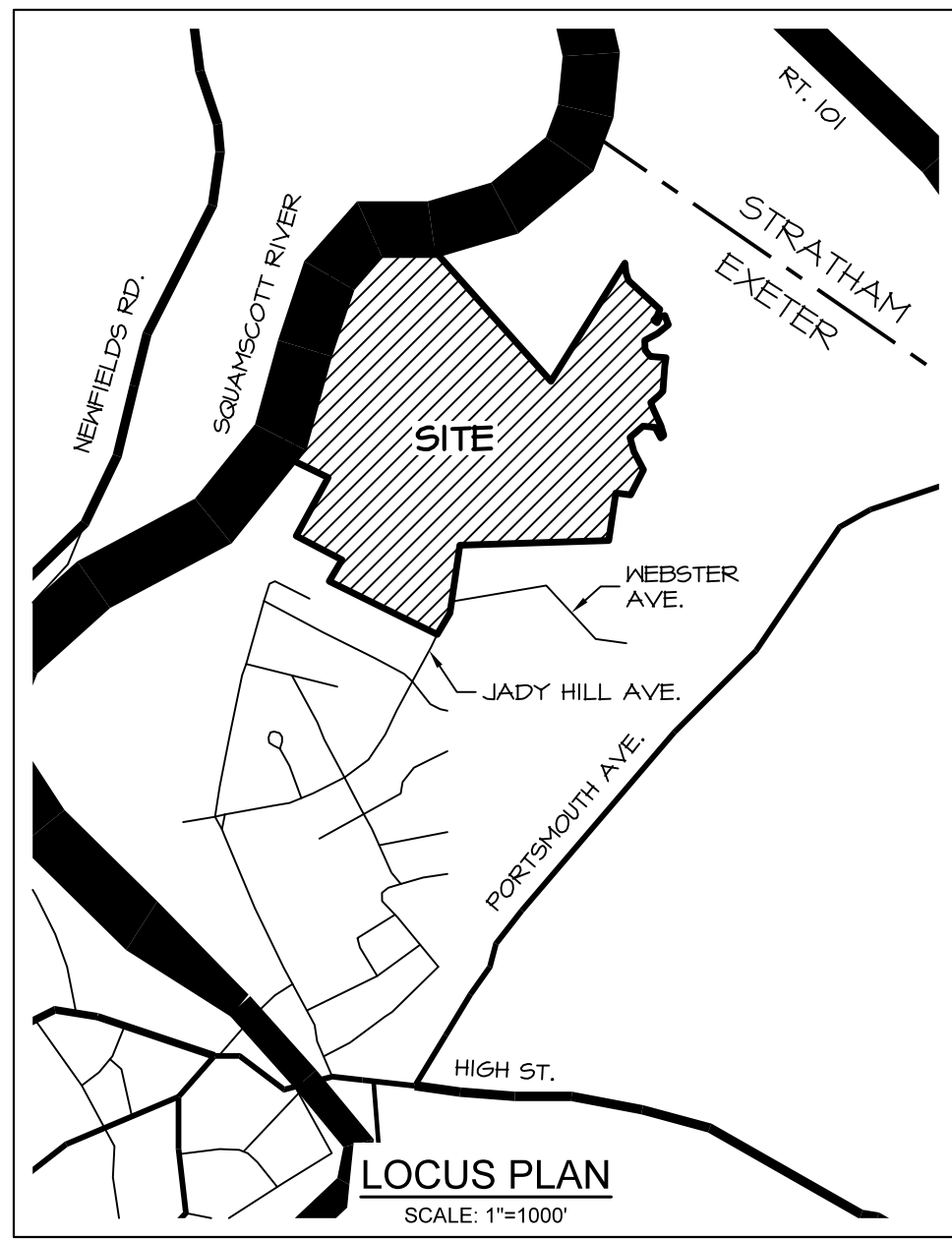


CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833



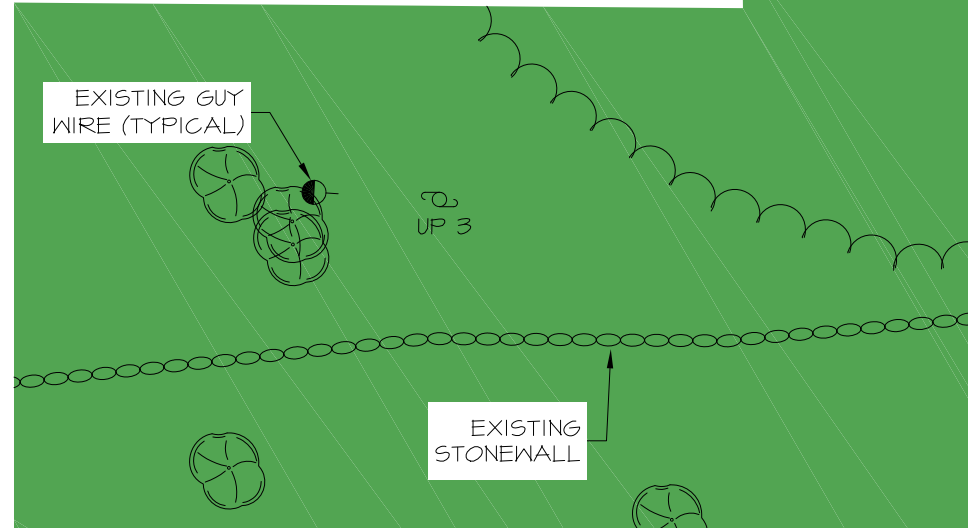
TITLE:
EXISTING CONDITIONS
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=20'	C1



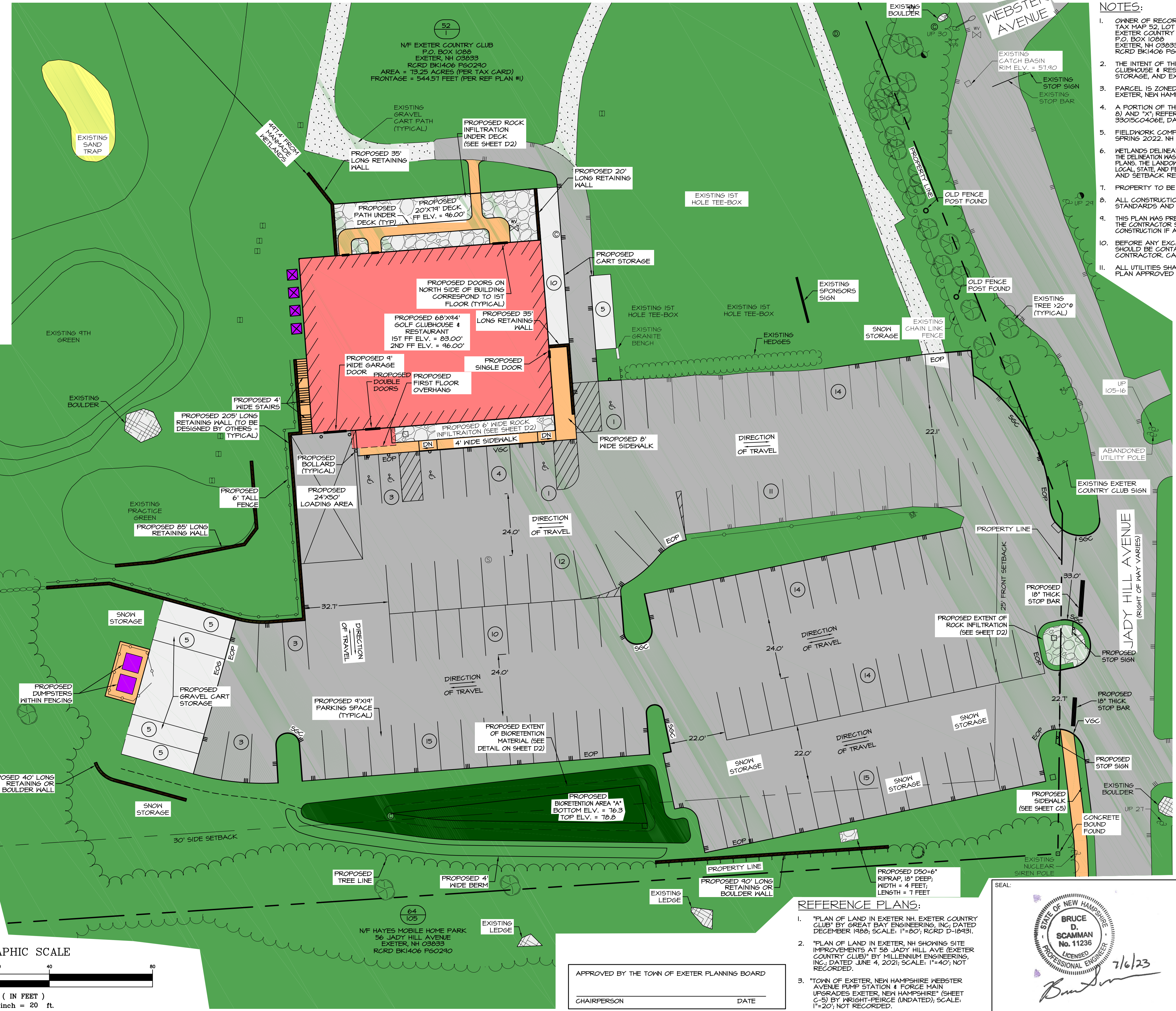
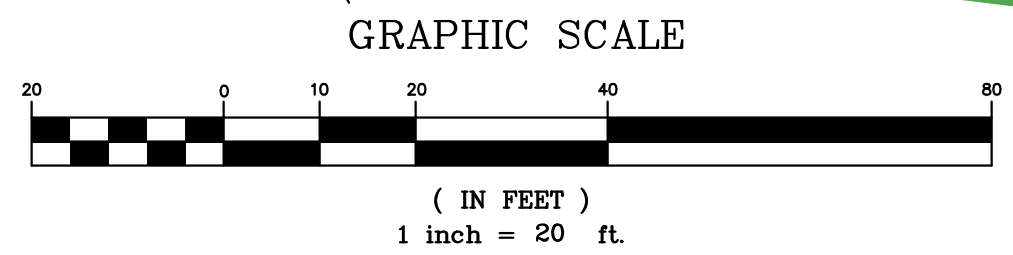
WETLAND NOTES:
 WETLANDS WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022 UTILIZING THE FOLLOWING STANDARDS:

- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION, (VERSION 2.0) JANUARY 2012, U.S. ARMY CORPS OF ENGINEERS.
- FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND DELINEATING HYDRIC SOILS, VERSION 8.2, UNITED STATES DEPARTMENT OF AGRICULTURE (2016).
- NEW ENGLAND HYDRIC SOILS TECHNICAL COMMITTEE, 2020 VERSION 4, FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION, LOWELL, MA, U.S. ARMY CORPS OF ENGINEERS NATIONAL WETLAND PLANT LIST, VERSION 3.5, (2020)



LEGEND

□	CONCRETE BOUND FOUND
○	IRON PIPE FOUND
---	TYPICAL PROPERTY LINE
---	EDGE OF PAVEMENT (EOP)
---	HVAC UNIT
---	OVERHEAD UTILITIES
---	UNDERGROUND UTILITIES
---	UTILITY POLE
---	GUY WIRE
---	LIGHT FIXTURE
---	IRRIGATION BOX
---	WATER LINE
---	WATER VALVE
---	DRAIN LINE
---	CATCH BASIN
---	DRAIN MANHOLE
---	CLEANOUT
---	SEWER MANHOLE
---	PROPANE TANK
---	STONE WALL
---	WETLANDS
---	TREE LINE
---	TREE/SHRUB



- NOTES:**
- OWNER OF RECORD:
 TAX MAP 52, LOT
 EXETER COUNTRY CLUB
 P.O. BOX 1088
 EXETER, NH 03833
 RCRD BK1406 PG0240
 - THE INTENT OF THIS PLAN IS TO SHOW A PROPOSED 68'x141' GOLF CLUBHOUSE & RESTAURANT WITH ATTACHED 20'x141' DECK, GOLF CART STORAGE, AND EXPANDED PARKING.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2014 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES 'AE' (EL B) AND 'X', REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
 - FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID; NAVD 1988.
 - WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DEPICTED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
 - PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
 - ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
 - THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
 - BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 12 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
 - ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.

- TOWN OF EXETER ZONING REQUIREMENTS FOR THE 'R-2' DISTRICT, PER THE TOWN OF EXETER NH ZONING ORDINANCE DATED MARCH 2022:
 - MINIMUM LOT AREA = 15000 SF (0.344 ACRES)
 - LOT AREA PROVIDED = 73,25 ACRES
 - MINIMUM LOT WIDTH = 100 FEET
 - LOT WIDTH PROVIDED = 33' 100 FEET
 - MINIMUM LOT DEPTH = 100 FEET
 - MINIMUM FRONT SETBACK = 25 FEET
 - MINIMUM SIDE SETBACK = 15 FT (ONE) 30 FT (BOTH)
 - MINIMUM REAR SETBACK = 25 FEET
 - MAXIMUM BUILDING COVERAGE = 25%
 - BUILDING COVERAGE PROVIDED = 0.25%
 - MINIMUM OPEN SPACE = 40%
 - INTERVIOUS AREA (EXCLUDES CART PATHS) = 54545 SF
 - OPEN SPACE PROVIDED = 49.15%
- PARKING REQUIREMENTS PER THE TOWN OF EXETER, NH SITE PLAN REVIEW & SUBDIVISION REGULATIONS AS AMENDED FEBRUARY 2022, AND THE ZONING ORDINANCE AMENDED THROUGH MARCH 2022:
 - PARKING SPACE DIMENSIONS = 10'x14' (MIN)
 - PARKING ANGLE MINIMUM AISLE WIDTH
 45-DEGREES 16- FEET
 60-DEGREES 20- FEET
 90-DEGREES 22- FEET
 REQUIRED:
 - FOR RESTAURANTS/BARS = 1 SPACE PER 3 SEATS
 - PROPOSED SEATS = 127 SEATS
 - 127 SEATS x (1 SPACE / 3 SEATS) = 43 SPACES
 - FUNCTION ROOM = 1 SPACE PER 3 SEATS
 - PROPOSED SEATS = 36
 - 36 SEATS x (1 SPACE / 3 SEATS) = 12 SPACES
 - GOLF COURSE (ESTIMATED) = 5 SPACES PER HOLE
 - 9 HOLES x (5 SPACES/HOLE) = 45 SPACES
 - 20 EMPLOYEES x (1 SPACE/EMPLOYEE) = 20 SPACES
 - TOTAL REQUIRED = 43 + 12 + 45 + 20 = 120 SPACES PROVIDED:
 - PROPOSED PARKING SPACES PROVIDED = 120 SPACES (INCLUDES 3 EV READY SPACES)
- ALL KNOX BOX, FIRE ALARM SYSTEM AND FIRE SPRINKLER INSTALLATION & INSPECTIONS TO BE COORDINATED WITH THE EXETER FIRE DEPARTMENT.
- ALL MUSIC TO MEET THE TOWN OF EXETER ZONING ORDINANCE AT THE TIME OF APPROVAL.
- ALL LIGHTING TO BE TURNED OFF OR REDUCED IN INTENSITY AT 10 PM PER SECTION 4.20.4.4 OF THE 2022 SITE PLAN REVIEW AND SUBDIVISION REGULATIONS FOR THE TOWN OF EXETER.

6	JULY 6, 2023	FOR APPROVAL	
5	JUNE 16, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:	CHK.	
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		



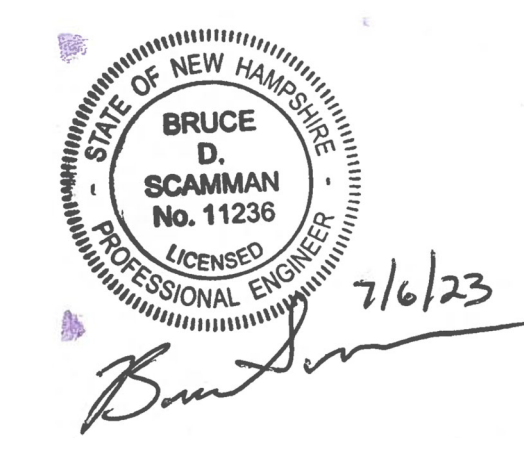
CLIENT:
BLIND TIGER, LLC
 3 WRIGHT LANE
 EXETER, NH 03833

SITE PLAN
 FOR
EXETER COUNTRY CLUB
 58 JADY HILL AVENUE (SITE)
 EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=20'	C2

APPROVED BY THE TOWN OF EXETER PLANNING BOARD
 CHAIRPERSON _____ DATE _____

- REFERENCE PLANS:**
- "PLAN OF LAND IN EXETER NH, EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC. DATED DECEMBER 1488, SCALE: 1"=80'; RCRD D-184931.
 - "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE (EXETER COUNTRY CLUB)" BY MILLENNIUM ENGINEERING, INC. DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
 - "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-9) BY WRIGHT-FERGIE (UNDATED); SCALE: 1"=20'; NOT RECORDED.



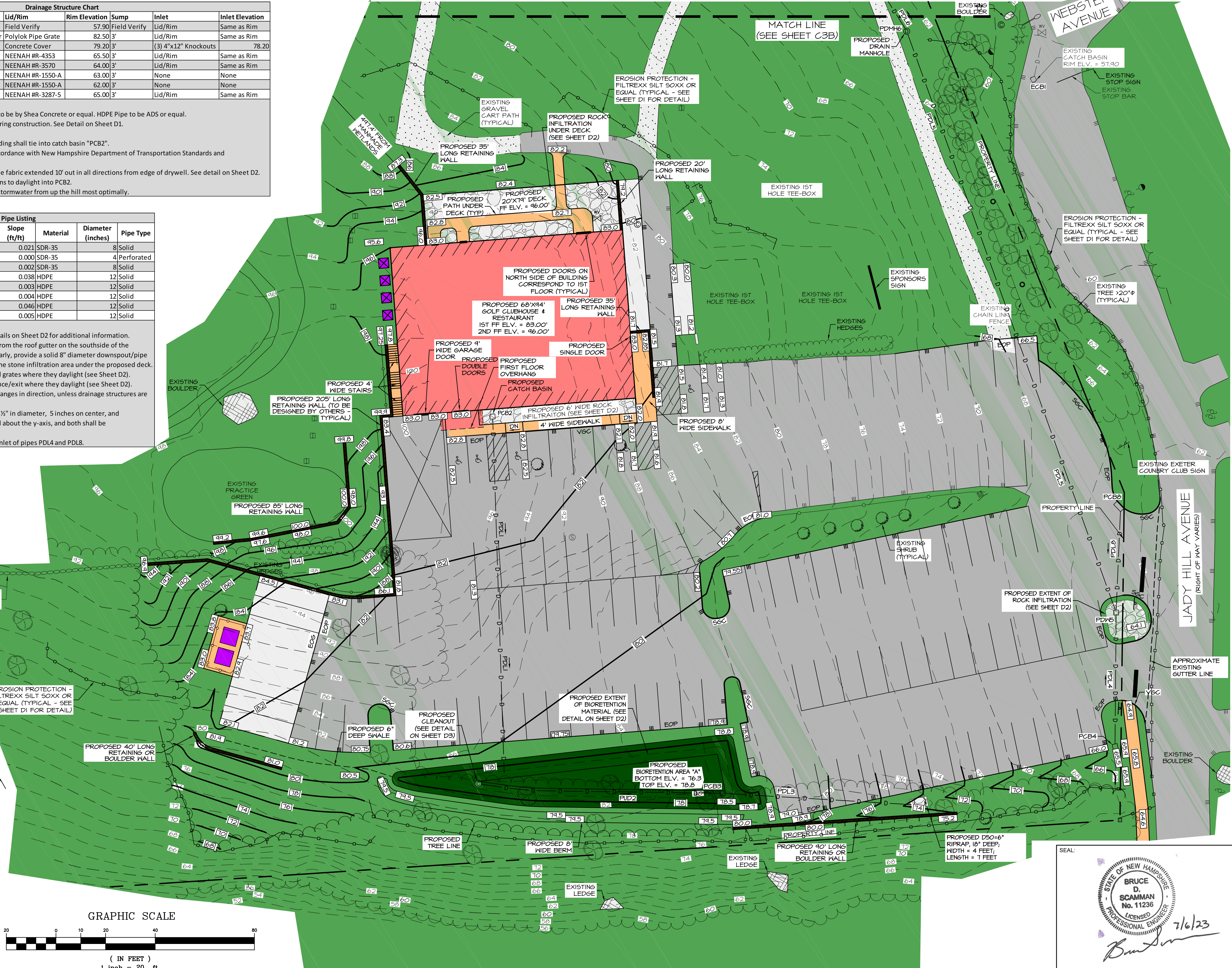
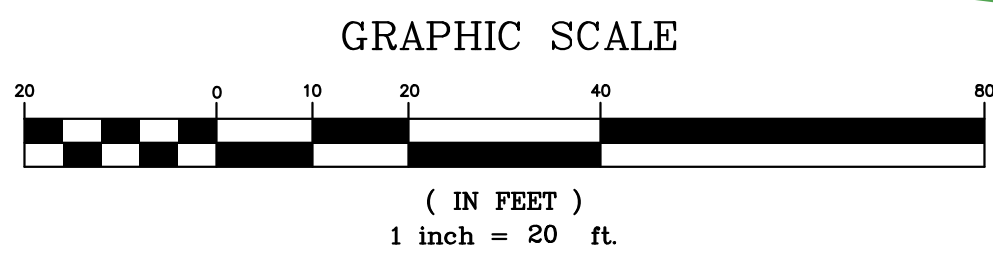
Drainage Structure Chart						
Structure #	Structure	Size	Lid/Rim	Rim Elevation	Sump	Inlet Elevation
ECB1	Concrete Catch Basin	Field Verify	Field Verify	57.90	Field Verify	Lid/Rim Same as Rim
PCB2	HDPE Pipe	12" Diameter	Polylok Pipe Grate	82.50	3'	Lid/Rim Same as Rim
PCB3	Concrete Catch Basin	4' Diameter	Concrete Cover	79.20	3'	(3) 4"x12" Knockouts 78.20
PCB4	Concrete Catch Basin	4' Diameter	NEENAH HR-4353	65.50	3'	Lid/Rim Same as Rim
PDW5	Concrete Drywell	4' Diameter	NEENAH HR-3570	64.00	3'	Lid/Rim Same as Rim
PDMH6	Concrete Catch Basin	4' Diameter	NEENAH HR-1550-A	63.00	3'	None
PDMH7	Concrete Catch Basin	4' Diameter	NEENAH HR-1550-A	62.00	3'	None
PCB8	Concrete Catch Basin	6' Diameter	NEENAH HR-3287-5	65.00	3'	Lid/Rim Same as Rim

- Notes:**
- Proposed concrete catch basins and drywells to be by Shea Concrete or equal. HDPE Pipe to be ADS or equal.
 - Provide SiltSack Type C within ECB1 & PCB4 during construction. See Detail on Sheet D1.
 - See Sheet D2 for knockout details.
 - Gutters on the southside of the proposed building shall tie into catch basin "PCB2".
 - All concrete structures must be installed in accordance with New Hampshire Department of Transportation Standards and specifications for Road and Bridge Construction.
 - PDW5 to be underlain by Mirafi 140N geotextile fabric extended 10' out in all directions from edge of drywell. See detail on Sheet D2.
 - Foundation drain and retaining wall underdrains to daylight into PCB2.
 - The rim for PCB8 shall be oriented as to catch stormwater from up the hill most optimally.

Drainage Pipe Listing						
Pipe #	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	Material	Pipe Type
PDL1	80.00	77.00	145	0.021	SDR-35	8 Solid
PUD2	74.05	74.05	95	0.000	SDR-35	4 Perforated
PDL3	74.05	73.90	75	0.002	SDR-35	8 Solid
PDL4	63.00	61.10	50	0.038	HDPE	12 Solid
PDL5	61.00	60.35	245	0.003	HDPE	12 Solid
PDL6	60.25	58.50	420	0.004	HDPE	12 Solid
PDL7	58.40	46.10	265	0.046	HDPE	12 Solid
PDL8	61.30	61.10	40	0.005	HDPE	12 Solid

- Notes:**
- See bioretention area and rock infiltration details on Sheet D2 for additional information.
 - Provide a solid 8" diameter downspout/pipe from the roof gutter on the southside of the proposed building into catch basin "PCB2". Similarly, provide a solid 8" diameter downspout/pipe on the northside of the proposed building into the stone infiltration area under the proposed deck.
 - Pipes PDL1, PDL3, & PDL7 to have animal guard grates where they daylight (see Sheet D2).
 - Pipes PDL1, PDL3, & PDL7 to have flared entrance/exit where they daylight (see Sheet D2).
 - Cleanouts to be provided at ends of pipe or changes in direction, unless drainage structures are already provided. See detail on Sheet D3.
 - Perforated pipes shall have two rows of holes 1/2" in diameter, 5 inches on center, and spaced 120" apart. Perforations shall be mirrored about the y-axis, and both shall be located on the bottom half of the pipe.
 - An 8" diameter plate shall be installed at the inlet of pipes PDL4 and PDL8.

LEGEND	
	CONCRETE BOUND FOUND
	IRON PIPE FOUND
	TYPICAL PROPERTY LINE
	EDGE OF PAVEMENT (EOP)
	HVAC UNIT
	OVERHEAD UTILITIES
	UNDERGROUND UTILITIES
	UTILITY POLE
	GUY WIRE
	LIGHT FIXTURE
	IRRIGATION BOX
	WATER LINE
	WATER VALVE
	DRAIN LINE
	CATCH BASIN
	DRAIN MANHOLE
	CLEANOUT
	SEWER MANHOLE
	SEWER LINE
	PROPANE TANK
	STONE WALL
	WETLANDS
	TREE LINE
	TREE/SHRUB



- NOTES:**
- OWNER OF RECORD: TAX MAP 55, LOT 1 EXETER COUNTRY CLUB P.O. BOX 1088 EXETER, NH 03833 RCRD BK1406 P60240
 - THE INTENT OF THIS PLAN IS TO SHOW THE PROPOSED GRADING AND DRAINAGE ON SITE ASSOCIATED WITH THE PROPOSED SITE IMPROVEMENTS.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL 8) AND "X"; REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
 - FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID; NAVD 1988.
 - WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DESCRIBED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
 - PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
 - ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
 - THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
 - BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 72 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
 - ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.
 - ALL WATER, SEWER, ROAD (INCLUDING PARKING LOT), AND DRAINAGE WORK SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 4.5 GRADING, DRAINAGE, AND EROSION & SEDIMENT CONTROL AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION OF PUBLIC UTILITIES IN EXETER, NEW HAMPSHIRE.
 - PRIOR TO CONSTRUCTION, IT IS NECESSARY TO SUBMIT A NOTICE OF INTENT (NOI) AND IMPLEMENT A STORM WATER POLLUTION PREVENTION PLAN (SWPPP). THE SWPPP INSPECTOR MUST BE A "QUALIFIED PERSON" WHO IS CERTIFIED ACCORDING TO THE LATEST REGULATIONS GOVERNING SUCH CERTIFICATIONS.
 - TOTAL PROPOSED SITE DISTURBANCE = 16,600 SF

- REFERENCE PLANS:**
- "PLAN OF LAND IN EXETER NH. EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC. DATED DECEMBER 1988; SCALE: 1"=80'; RCRD D-10431.
 - "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE (EXETER COUNTRY CLUB)" BY MILLENNIUM ENGINEERING, INC. DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
 - "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-5) BY WRIGHT-PERCE (UNDATED); SCALE: 1"=20'; NOT RECORDED.

5	JULY 6, 2023	FOR APPROVAL	
4	JUNE 16, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
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EMANUEL ENGINEERING
civil & structural consultants, land planners
 118 PORTSMOUTH AVENUE, A202
 STRATHAM, NH 03885
 P: 603-772-4400 F: 603-772-4487
 WWW.EMANUELENGINEERING.COM

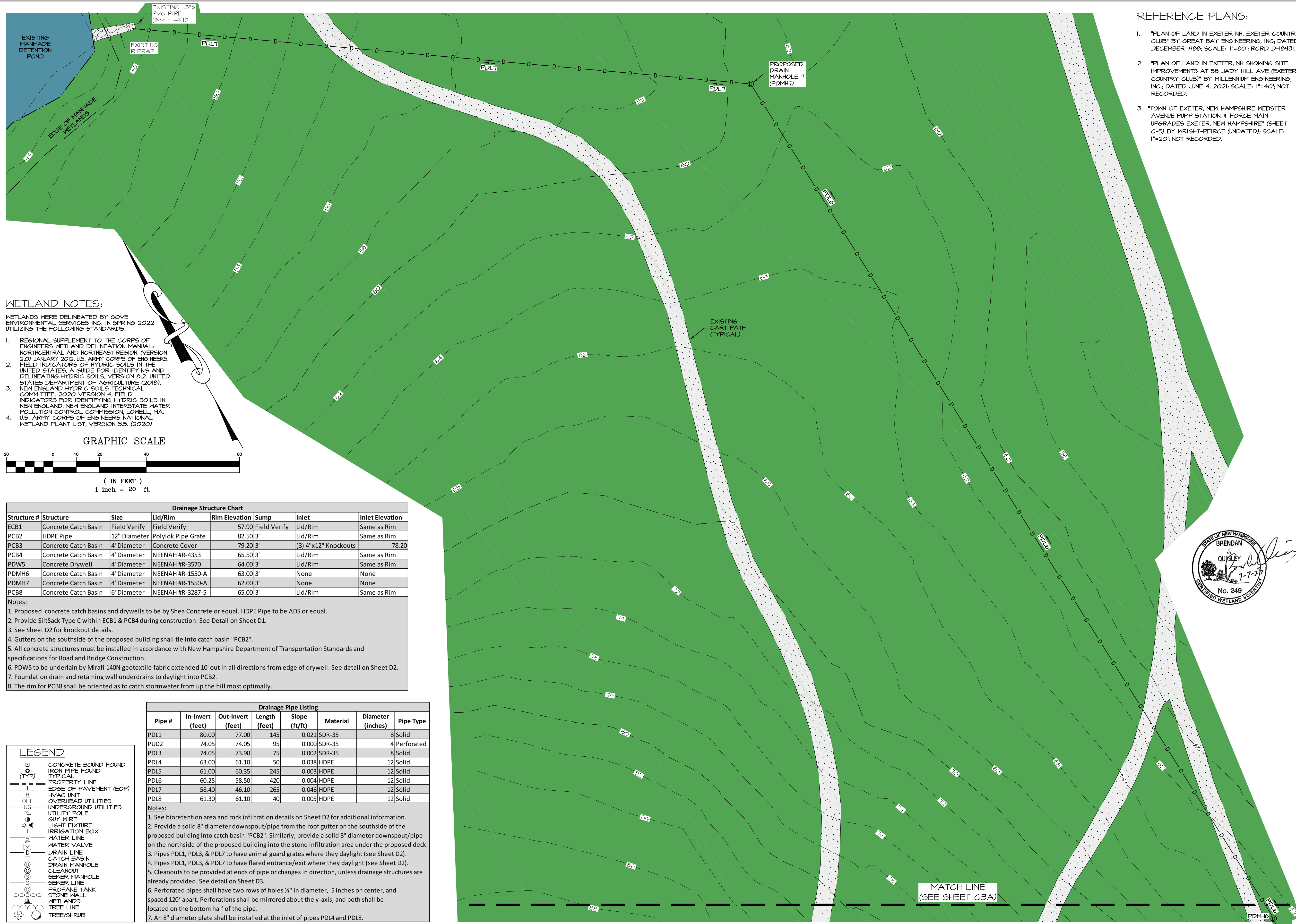
CLIENT:
BLIND TIGER, LLC
 3 WRIGHT LANE
 EXETER, NH 03833

SEAL:

 BRUCE D. SCAMMAN
 No. 11236
 LICENSED PROFESSIONAL ENGINEER
 7/6/23

TITLE:
GRADING & DRAINAGE PLAN
 FOR
 EXETER COUNTRY CLUB
 58 JADY HILL AVENUE (SITE)
 EXETER, NH 03833

PROJECT: 21-157 SCALE: 1"=20' SHEET: C3A



REFERENCE PLANS:

- "PLAN OF LAND IN EXETER, NH, EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC, DATED DECEMBER 1988; SCALE: 1"=80'; RCRD D-10493.
- "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE (EXETER COUNTRY CLUB)" BY MILLENNIUM ENGINEERING, INC.; DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
- "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-5) BY WRIGHT-PEIRCE (UNDATED); SCALE: 1"=20'; NOT RECORDED.

NOTES:

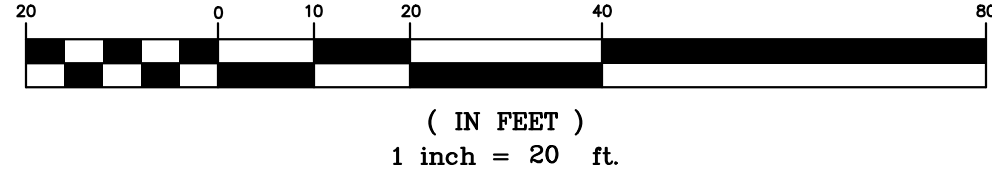
- OWNER OF RECORD: TAX MAP 52, LOT 1, EXETER COUNTRY CLUB, P.O. BOX 1088, EXETER, NH 03833, RCRD BK1406 P50240
- THE INTENT OF THIS PLAN IS TO SHOW THE PROPOSED DRAINAGE ON SITE ASSOCIATED WITH THE PROPOSED SITE IMPROVEMENTS CONVEYING STORMWATER TO THE EXISTING DETENTION POND NORTH OF THE PROPOSED IMPROVEMENTS.
- PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
- A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL B) AND "X"; REFERENCE FLOOD INSURANCE RATE MAPS 33015G0402E & 33015G0406E, DATED MAY 17, 2005.
- FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID, NAVD 1988.
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- PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
- ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
- THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
- BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 72 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
- ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.
- ALL WATER, SEWER, ROAD (INCLUDING PARKING LOT), AND DRAINAGE WORK SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 9.5 GRADING, DRAINAGE, AND EROSION & SEDIMENT CONTROL AND THE STANDARD SPECIFICATIONS FOR CONSTRUCTION OF PUBLIC UTILITIES IN EXETER, NEW HAMPSHIRE.
- PRIOR TO CONSTRUCTION, IT IS NECESSARY TO SUBMIT A NOTICE OF INTENT (NOI) AND IMPLEMENT A STORM WATER POLLUTION PREVENTION PLAN (SWPPP). THE SWPPP INSPECTOR MUST BE A "QUALIFIED PERSON" WHO IS CERTIFIED ACCORDING TO THE LATEST REGULATIONS GOVERNING SUCH CERTIFICATIONS.
- TOTAL PROPOSED SITE DISTURBANCE = 76600 SF

WETLAND NOTES:

WETLANDS WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES INC. IN SPRING 2022 UTILIZING THE FOLLOWING STANDARDS:

- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION (VERSION 2.0) JANUARY 2012, U.S. ARMY CORPS OF ENGINEERS.
- FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND DELINEATING HYDRIC SOILS, VERSION 6.2, UNITED STATES DEPARTMENT OF AGRICULTURE (2010).
- NEW ENGLAND HYDRIC SOILS TECHNICAL COMMITTEE, 2020 VERSION 4, FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION, LOWELL, MA.
- U.S. ARMY CORPS OF ENGINEERS NATIONAL WETLAND PLANT LIST, VERSION 3.5, (2020)

GRAPHIC SCALE



Structure #	Structure	Size	Lid/Rim	Rim Elevation	Sump	Inlet	Inlet Elevation
ECB1	Concrete Catch Basin	Field Verify	Field Verify	57.90	Field Verify	Lid/Rim	Same as Rim
PCB2	HDPE Pipe	12" Diameter	Polylok Pipe Grate	82.50	3'	Lid/Rim	Same as Rim
PCB3	Concrete Catch Basin	4' Diameter	Concrete Cover	79.20	3'	(3) 4"x12" Knockouts	78.20
PCB4	Concrete Catch Basin	4' Diameter	NEENAH #R-4353	65.50	3'	Lid/Rim	Same as Rim
PDW5	Concrete Drywell	4' Diameter	NEENAH #R-3570	64.00	3'	Lid/Rim	Same as Rim
PDMH6	Concrete Catch Basin	4' Diameter	NEENAH #R-1550-A	63.00	3'	None	None
PDMH7	Concrete Catch Basin	4' Diameter	NEENAH #R-1550-A	62.00	3'	None	None
PCB8	Concrete Catch Basin	6' Diameter	NEENAH #R-3287-5	65.00	3'	Lid/Rim	Same as Rim

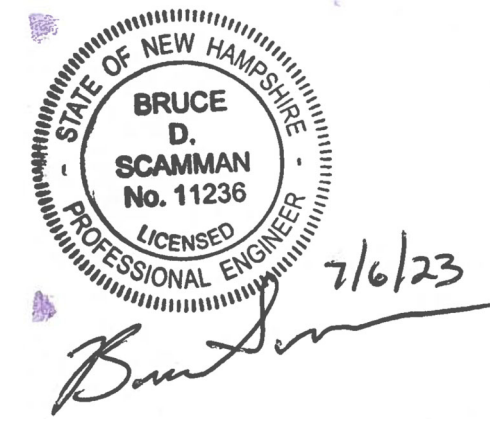
- Notes:**
- Proposed concrete catch basins and drywells to be by Shea Concrete or equal. HDPE Pipe to be ADS or equal.
 - Provide SiltSack Type C within ECB1 & PCB4 during construction. See Detail on Sheet D1.
 - See Sheet D2 for knockout details.
 - Gutters on the southside of the proposed building shall tie into catch basin "PCB2".
 - All concrete structures must be installed in accordance with New Hampshire Department of Transportation Standards and specifications for Road and Bridge Construction.
 - PDW5 to be underlain by Mirafi 140N geotextile fabric extended 10' out in all directions from edge of drywell. See detail on Sheet D2.
 - Foundation drain and retaining wall underdrains to daylight into PCB2.
 - The rim for PCB8 shall be oriented as to catch stormwater from up the hill most optimally.

Pipe #	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	Material	Diameter (inches)	Pipe Type
PDL1	80.00	77.00	145	0.021	SDR-35	8	Solid
PDL2	74.05	74.05	95	0.000	SDR-35	4	Perforated
PDL3	74.05	73.90	75	0.002	SDR-35	8	Solid
PDL4	63.00	61.10	50	0.038	HDPE	12	Solid
PDL5	61.00	60.35	245	0.003	HDPE	12	Solid
PDL6	60.25	58.50	420	0.004	HDPE	12	Solid
PDL7	58.40	46.10	265	0.046	HDPE	12	Solid
PDL8	61.30	61.10	40	0.005	HDPE	12	Solid

- Notes:**
- See bioretention area and rock infiltration details on Sheet D2 for additional information.
 - Provide a solid 8" diameter downspout/pipe from the roof gutter on the southside of the proposed building into catch basin "PCB2". Similarly, provide a solid 8" diameter downspout/pipe on the northside of the proposed building into the stone infiltration area under the proposed deck.
 - Pipes PDL1, PDL3, & PDL7 to have animal guard grates where they daylight (see Sheet D2).
 - Pipes PDL1, PDL3, & PDL7 to have flared entrance/exit where they daylight (see Sheet D2).
 - Cleanouts to be provided at ends of pipe or changes in direction, unless drainage structures are already provided. See detail on Sheet D3.
 - Perforated pipes shall have two rows of holes 1/4" in diameter, 5 inches on center, and spaced 120" apart. Perforations shall be mirrored about the y-axis, and both shall be located on the bottom half of the pipe.
 - An 8" diameter plate shall be installed at the inlet of pipes PDL4 and PDL8.

LEGEND

- CONCRETE BOUND FOUND
- IRON PIPE FOUND
- (TYP) TYPICAL
- PROPERTY LINE
- EDGE OF PAVEMENT (EOP)
- HVAC UNIT
- OVERHEAD UTILITIES
- UNDERGROUND UTILITIES
- UTILITY POLE
- GUY WIRE
- LIGHT FIXTURE
- IRRIGATION BOX
- WATER LINE
- WATER VALVE
- DRAIN LINE
- CATCH BASIN
- DRAIN MANHOLE
- CLEANOUT
- SEWER MANHOLE
- SEWER LINE
- PROPANE TANK
- STONE WALL
- WETLANDS
- TREE LINE
- TREE/SHRUB



3	JULY 6, 2023	FOR APPROVAL	
2	MAY 23, 2023	FOR APPROVAL	
1	APR 20, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		



CLIENT:

BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

TITLE:

GRADING & DRAINAGE PLAN
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=20'	C3B

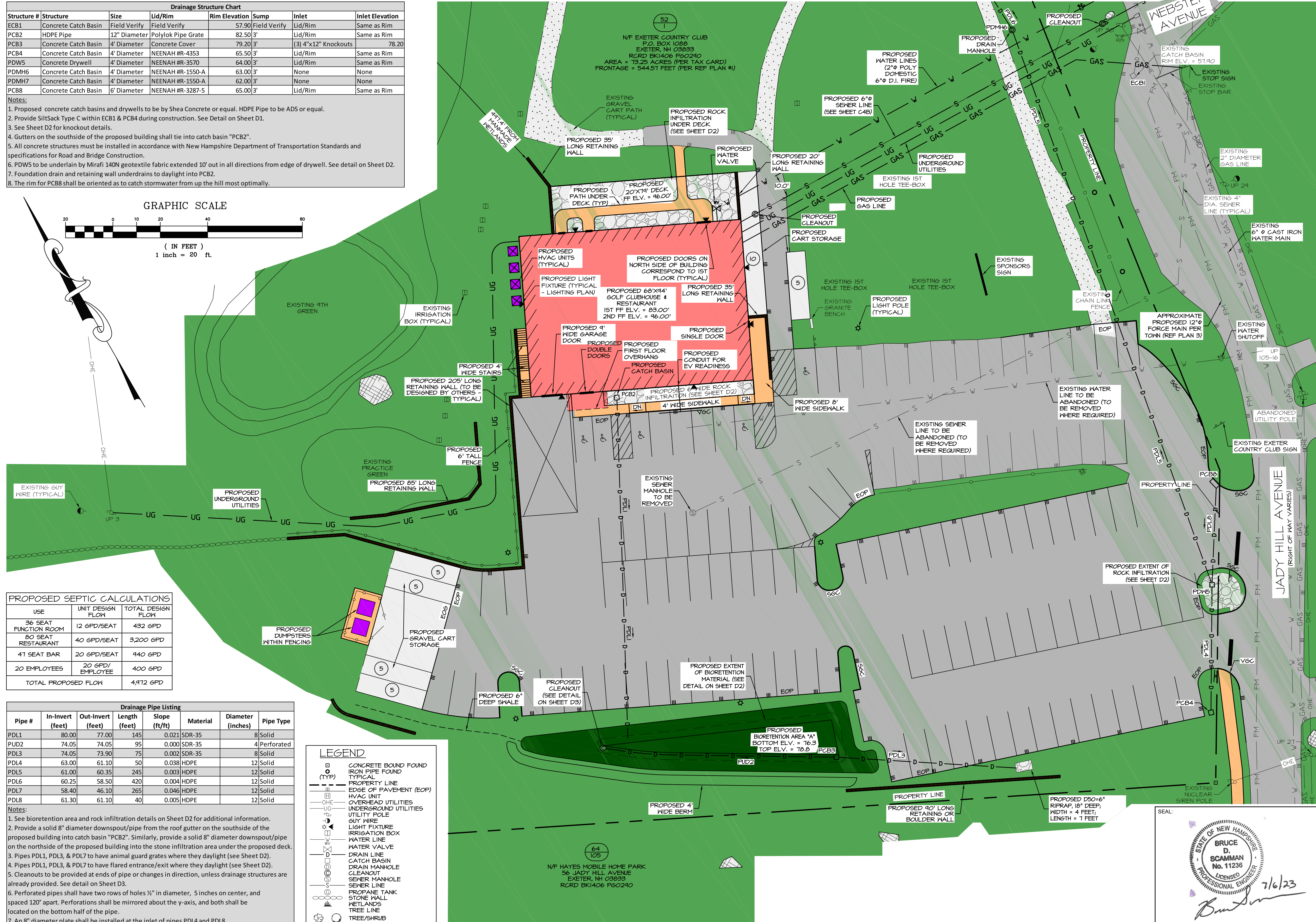
Drainage Structure Chart						
Structure #	Structure	Size	Lid/Rim	Rim Elevation	Sump	Inlet Elevation
ECB1	Concrete Catch Basin	Field Verify	Field Verify	57.90	Field Verify	Lid/Rim Same as Rim
PCB2	HDPE Pipe	12" Diameter	Polylok Pipe Grate	82.50	3'	Lid/Rim Same as Rim
PCB3	Concrete Catch Basin	4' Diameter	Concrete Cover	79.20	3'	(3) 4"x12" Knockouts 78.20
PCB4	Concrete Catch Basin	4' Diameter	NEENAH HR-4353	65.50	3'	Lid/Rim Same as Rim
PDW5	Concrete Drywell	4' Diameter	NEENAH HR-3570	64.00	3'	Lid/Rim Same as Rim
PDMH6	Concrete Catch Basin	4' Diameter	NEENAH HR-1550-A	63.00	3'	None
PDMH7	Concrete Catch Basin	4' Diameter	NEENAH HR-1550-A	62.00	3'	None
PCB8	Concrete Catch Basin	6' Diameter	NEENAH HR-3287-5	65.00	3'	Lid/Rim Same as Rim

- Notes:**
- Proposed concrete catch basins and drywells to be by Shea Concrete or equal. HDPE Pipe to be ADS or equal.
 - Provide SiltSack Type C within ECB1 & PCB4 during construction. See Detail on Sheet D1.
 - See Sheet D2 for knockout details.
 - Gutters on the southside of the proposed building shall tie into catch basin "PCB2".
 - All concrete structures must be installed in accordance with New Hampshire Department of Transportation Standards and specifications for Road and Bridge Construction.
 - PDW5 to be underlain by Mirafi 140N geotextile fabric extended 10' out in all directions from edge of drywell. See detail on Sheet D2.
 - Foundation drain and retaining wall underdrains to daylight into PCB2.
 - The rim for PCB8 shall be oriented as to catch stormwater from up the hill most optimally.

GRAPHIC SCALE



(IN FEET)
1 inch = 20 ft.



PROPOSED SEPTIC CALCULATIONS

USE	UNIT DESIGN FLOW	TOTAL DESIGN FLOW
36 SEAT FUNCTION ROOM	12 GPD/SEAT	432 GPD
80 SEAT RESTAURANT	40 GPD/SEAT	3,200 GPD
47 SEAT BAR	20 GPD/SEAT	940 GPD
20 EMPLOYEES	20 GPD/EMPLOYEE	400 GPD
TOTAL PROPOSED FLOW		4,972 GPD

Drainage Pipe Listing						
Pipe #	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	Material	Pipe Type
PDL1	80.00	77.00	145	0.021	SDR-35	8 Solid
PDL2	74.05	74.05	95	0.000	SDR-35	4 Perforated
PDL3	74.05	73.90	75	0.002	SDR-35	8 Solid
PDL4	63.00	61.10	50	0.038	HDPE	12 Solid
PDL5	61.00	60.35	245	0.003	HDPE	12 Solid
PDL6	60.25	58.50	420	0.004	HDPE	12 Solid
PDL7	58.40	46.10	265	0.046	HDPE	12 Solid
PDL8	61.30	61.10	40	0.005	HDPE	12 Solid

- Notes:**
- See bioretention area and rock infiltration details on Sheet D2 for additional information.
 - Provide a solid 8" diameter downspout/pipe from the roof gutter on the southside of the proposed building into catch basin "PCB2". Similarly, provide a solid 8" diameter downspout/pipe on the northside of the proposed building into the stone infiltration area under the proposed deck.
 - Pipes PDL1, PDL3, & PDL7 to have animal guard grates where they daylight (see Sheet D2).
 - Pipes PDL1, PDL3, & PDL7 to have flared entrance/exit where they daylight (see Sheet D2).
 - Cleanouts to be provided at ends of pipe or changes in direction, unless drainage structures are already provided. See detail on Sheet D3.
 - Perforated pipes shall have two rows of holes 1/2" in diameter, 5 inches on center, and spaced 120" apart. Perforations shall be mirrored about the y-axis, and both shall be located on the bottom half of the pipe.
 - An 8" diameter plate shall be installed at the inlet of pipes PDL4 and PDL8.

LEGEND

- ☐ CONCRETE BOUND FOUND
- ⊠ IRON PIPE FOUND
- (TYP) TYPICAL
- PROPERTY LINE
- EDGE OF PAVEMENT (EOP)
- HYAC UNIT
- OVERHEAD UTILITIES
- UNDERGROUND UTILITIES
- UTILITY POLE
- GUY WIRE
- LIGHT FIXTURE
- IRRIGATION BOX
- WATER LINE
- WATER VALVE
- DRAIN LINE
- CATCH BASIN
- DRAIN MANHOLE
- CLEANOUT
- SEWER MANHOLE
- SEWER LINE
- PROPANE TANK
- STONE WALL
- WETLANDS
- TREE LINE
- TREE/SHRUB

- NOTES:**
- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 P60240
 - THE INTENT OF THIS PLAN IS TO SHOW THE PROPOSED UTILITIES ON SITE ASSOCIATED WITH THE PROPOSED SITE IMPROVEMENTS.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL. 8) AND "X" REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
 - FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID, NAVD 1988.
 - WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DEPICTED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
 - PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
 - ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
 - THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
 - BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 72 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
 - ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.
 - SEE THE LIGHTING PLAN (SHEET L01) FOR EXPOSURE LIGHTING FOR LIGHTING DETAILS.
 - THE NEW GREASE INTERCEPTOR FOR THE PROPOSED RESTAURANT WITHIN THE CLUBHOUSE SHALL BE INSTALLED IN THE INTERIOR OF THE BUILDING.
 - ALL ABANDONED UTILITIES SHALL EITHER BE REMOVED, GAPPED, OR FILLED WITH FLOWABLE FILL.
 - ALL LIGHTING TO BE TURNED OFF OR REDUCED IN INTENSITY AT 10 PM PER SECTION 4.20.4.4 OF THE 2022 SITE PLAN REVIEW AND SUBDIVISION REGULATIONS FOR THE TOWN OF EXETER.

- REFERENCE PLANS:**
- "PLAN OF LAND IN EXETER, NH. EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC.; DATED DECEMBER 1988; SCALE: 1"=80'; RCRD D-18431.
 - "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE (EXETER COUNTRY CLUB)" BY MILLENNIUM ENGINEERING, INC.; DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
 - "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-5) BY WRIGHT-PEIRCE (UNDATED); SCALE: 1"=20'; NOT RECORDED.

6	JULY 6, 2023	FOR APPROVAL	
5	JUNE 16, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		

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 118 PORTSMOUTH AVENUE, A202
 STRATHAM, NH 03885
 P: 603-772-4400 F: 603-772-4487
 WWW.EMANUELENGINEERING.COM

CLIENT:
BLIND TIGER, LLC
 3 WRIGHT LANE
 EXETER, NH 03833

TITLE:
UTILITIES PLAN
 FOR
EXETER COUNTRY CLUB
 58 JADY HILL AVENUE (SITE)
 EXETER, NH 03833

PROJECT: 21-157 SCALE: 1"=20' SHEET: C4A

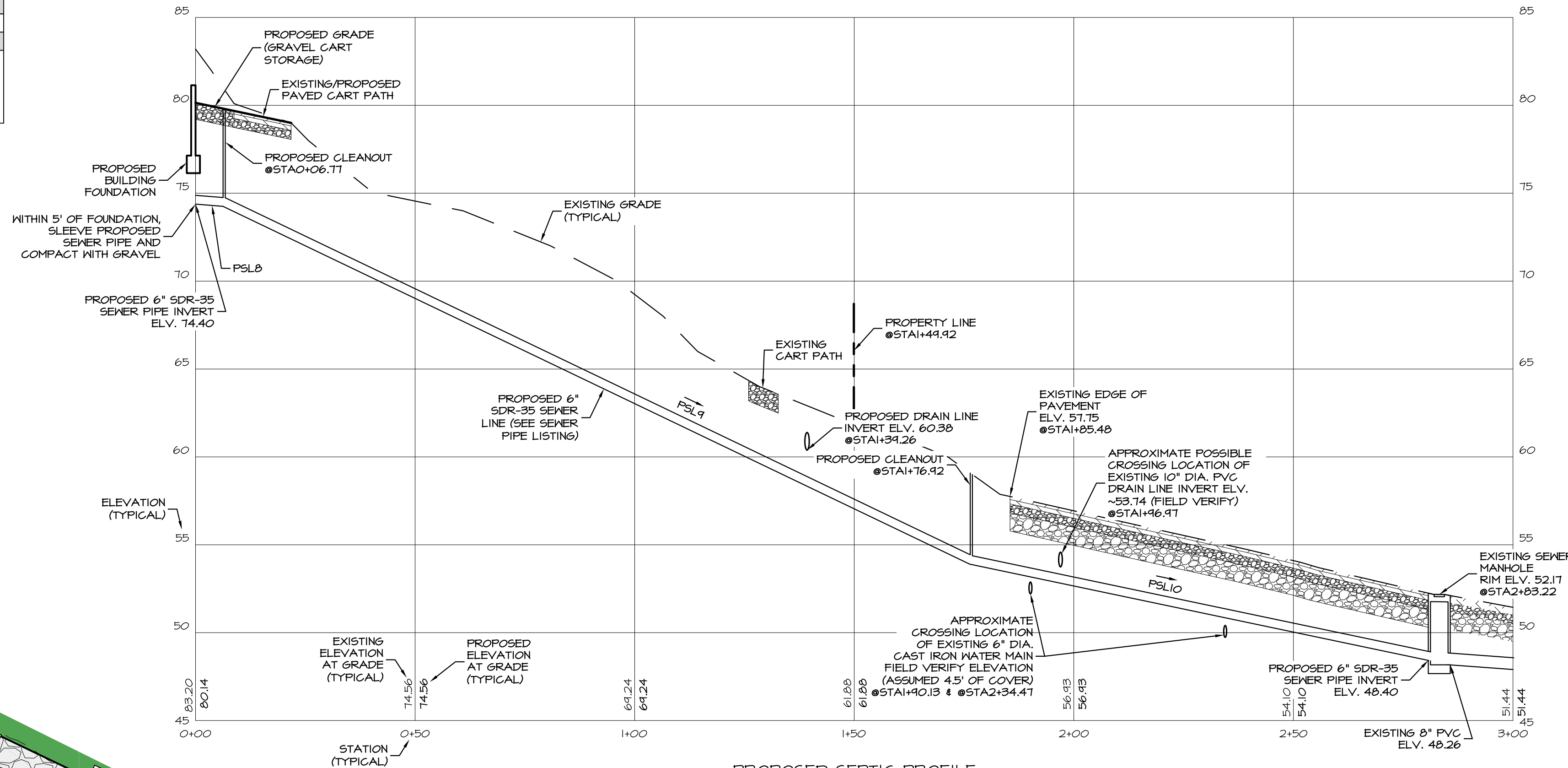
SEAL:

 BRUCE D. SCAMMAN
 No. 11236
 LICENSED PROFESSIONAL ENGINEER
 7/6/23

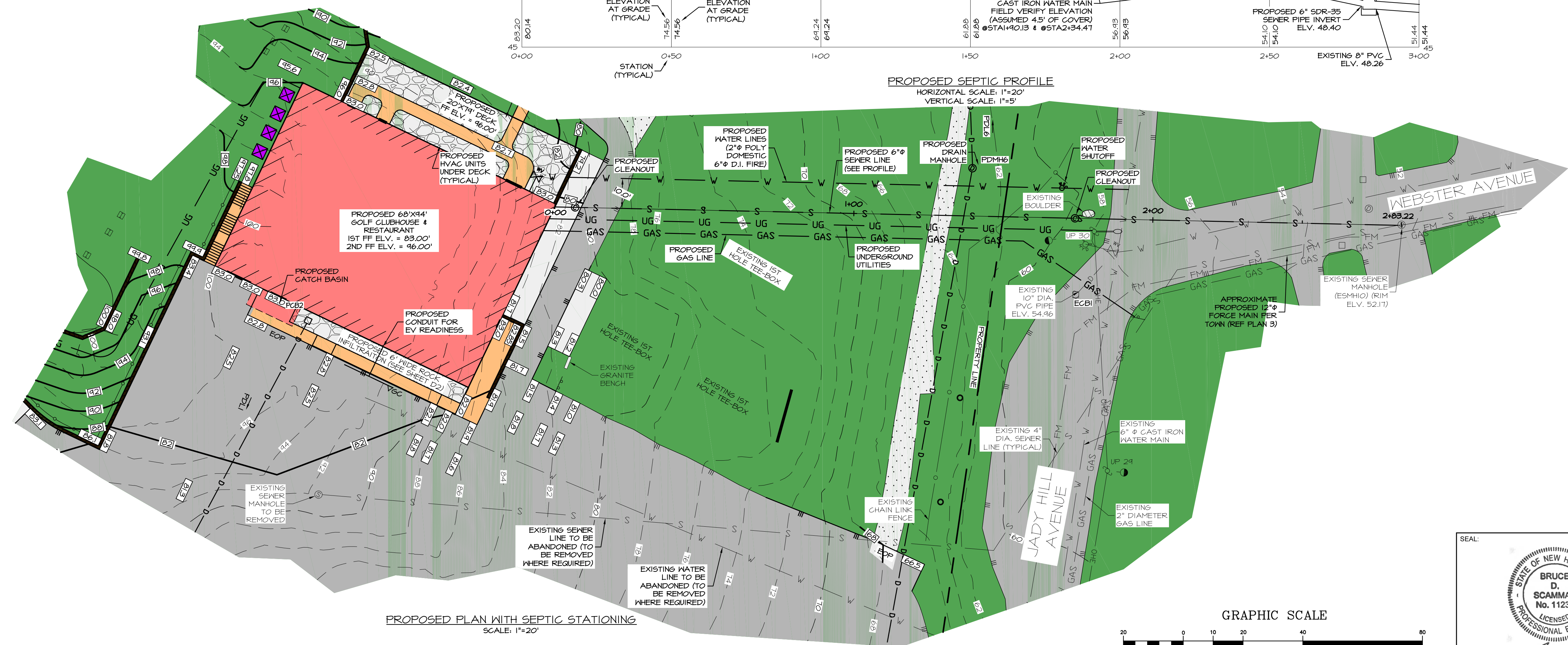
Septic Pipe Listing						
Pipe #	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	Material	Pipe Type
PSL8	74.40	74.25	7	0.021	SDR-35	6 Solid
PSL9	74.25	53.90	170	0.120	SDR-35	6 Solid
PSL10	53.90	48.40	150	0.037	SDR-35	6 Solid

Notes:
 1. Pipe PSL8 shall be sleeved and compacted with gravel within 5' of the proposed foundation.
 2. All joints, inlets, outlets, etc. To be sealed with a non-shrink grout, "water-plug" or equal.
 3. All pipes to be sleeved or encased in concrete within 10' of any water line crossing.

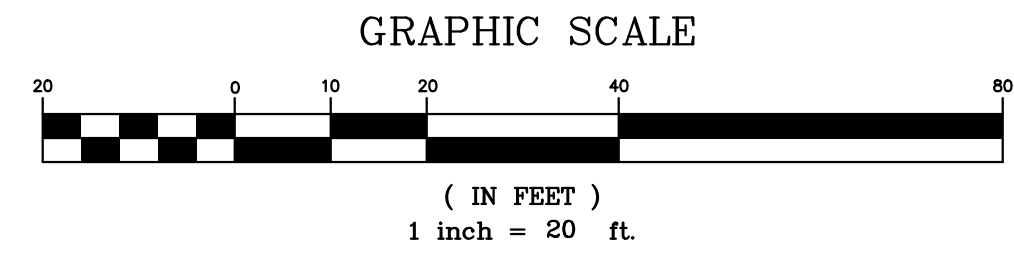
PROPOSED SEPTIC CALCULATIONS		
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TOTAL PROPOSED FLOW		4,972 GPD



PROPOSED SEPTIC PROFILE
 HORIZONTAL SCALE: 1"=20'
 VERTICAL SCALE: 1"=5'



PROPOSED PLAN WITH SEPTIC STATIONING
 SCALE: 1"=20'



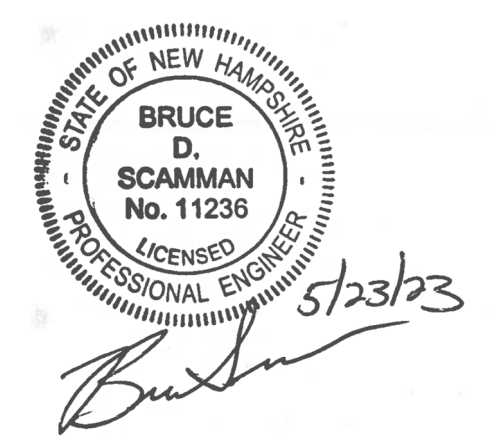
- NOTES:
- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 P60240
 - THE INTENT OF THIS PLAN IS TO SHOW THE PROFILE OF THE PROPOSED SEWER LINE REQUIRED FOR THE PROPOSED SITE IMPROVEMENTS.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2014 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL. 2) AND "X"; REFERENCE FLOOD INSURANCE RATE MAPS 33015G0402E & 33015G0406E, DATED MAY 17, 2005.
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- REFERENCE PLANS:
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3	MAY 23, 2023	FOR APPROVAL	
2	MAY 10, 2023	FOR APPROVAL	
1	APR 25, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN: JIM	DESIGN: JIM		
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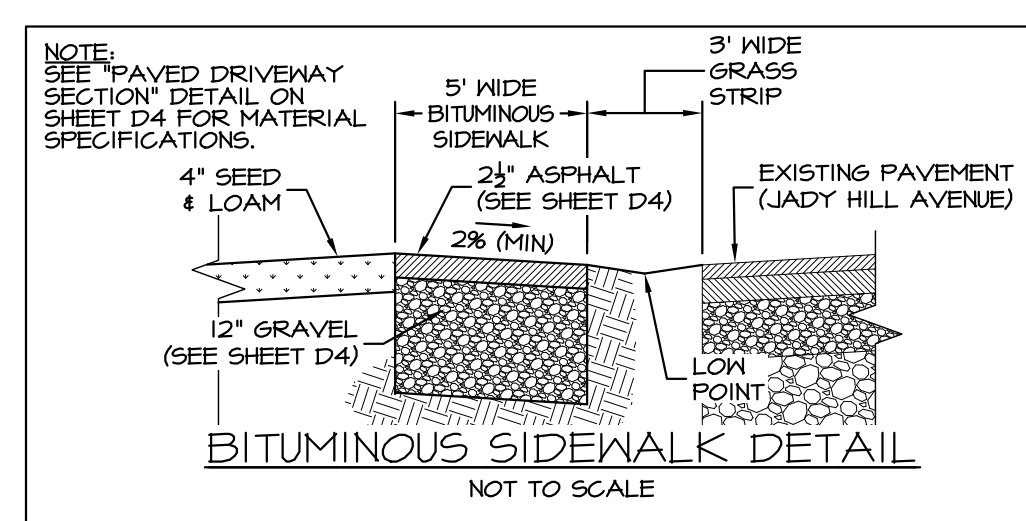
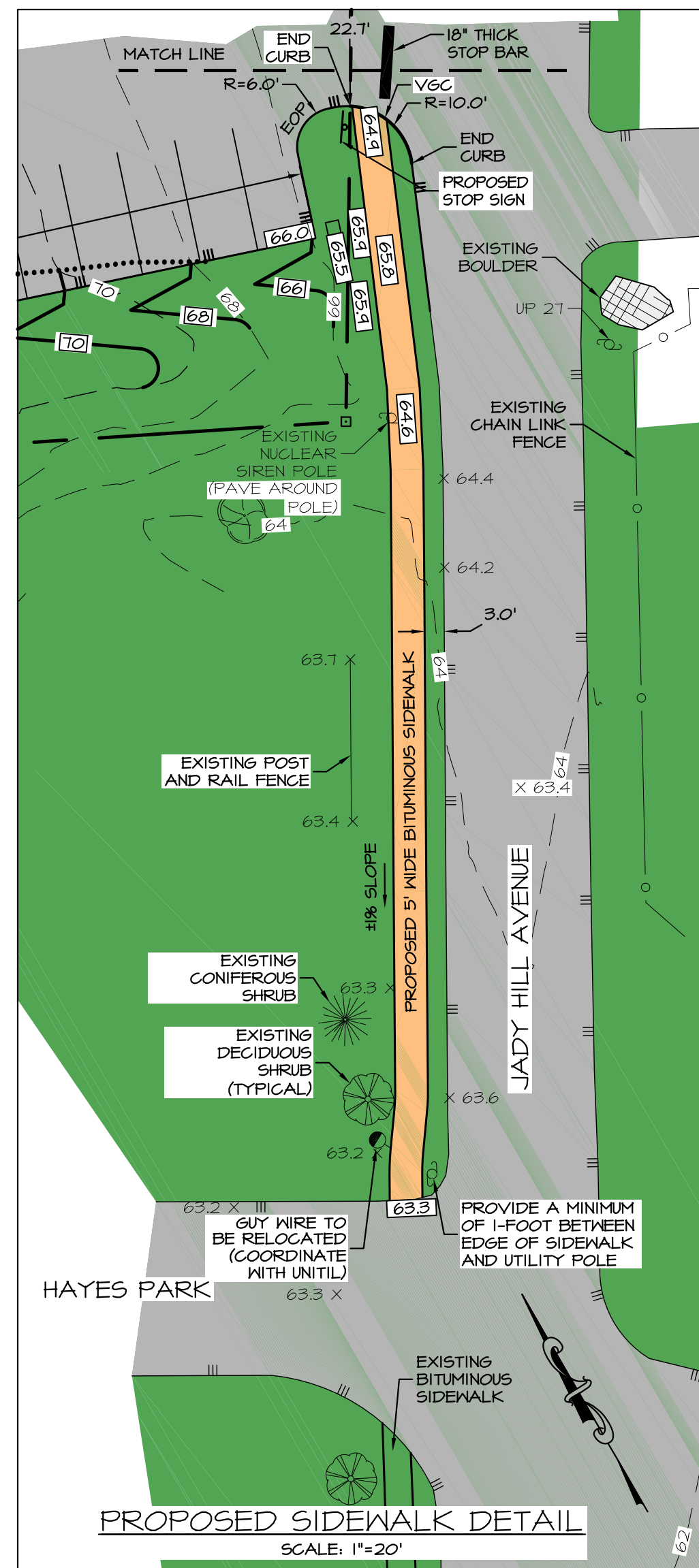


CLIENT:
BLIND TIGER, LLC
 3 WRIGHT LANE
 EXETER, NH 03833



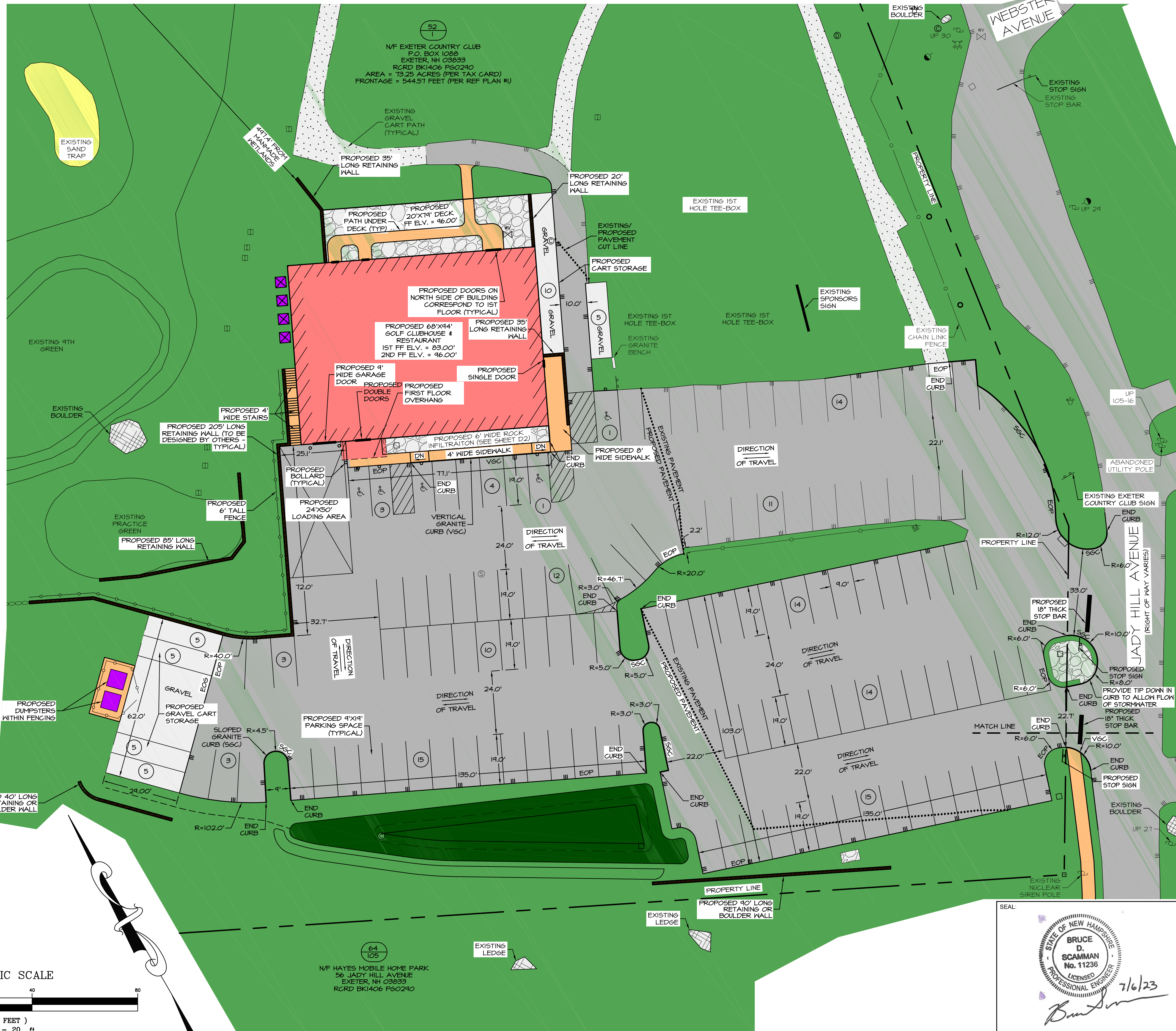
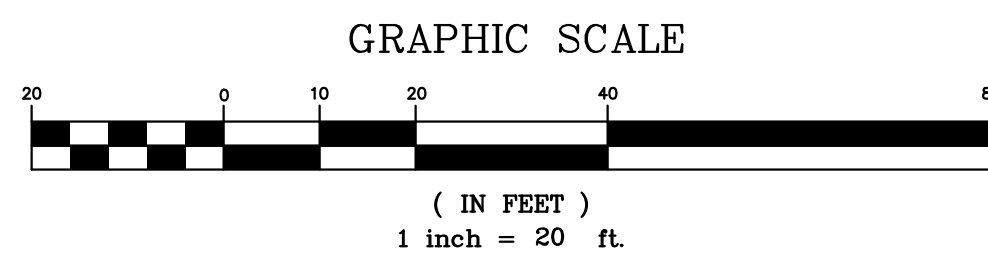
TITLE:
PROPOSED SEPTIC PROFILE FOR EXETER COUNTRY CLUB 58 JADY HILL AVENUE (SITE) EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=20'	C4B



LEGEND

	CONCRETE BOUND FOUND
	IRON PIPE FOUND
	TYPICAL PROPERTY LINE
	EDGE OF PAVEMENT (EOP)
	HVAC UNIT
	OVERHEAD UTILITIES
	UNDERGROUND UTILITIES
	UTILITY POLE
	GUY WIRE
	LIGHT FIXTURE
	IRRIGATION BOX
	WATER LINE
	WATER VALVE
	DRAIN LINE
	CATCH BASIN
	DRAIN MANHOLE
	CLEANOUT
	SEWER MANHOLE
	SEWER LINE
	PROPANE TANK
	STONE WALL
	WETLANDS
	TREE LINE
	TREE/SHRUB



- NOTES:**
- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 P60240
 - THE INTENT OF THIS PLAN IS TO SHOW THE DIMENSIONS FOR THE PROPOSED PARKING, TRAVELWAYS, CURBING, AND SIDEWALKS ON SITE.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL. 8) AND "X"; REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
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- REFERENCE PLANS:**
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STRATHAM, NH 03885
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WWW.EMANUELENGINEERING.COM

CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

SEAL:

TITLE:
PAVING & CURBING PLAN
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=20'	C5

EROSION AND SEDIMENTATION CONTROL - CONSTRUCTION PHASING AND SEQUENCING:

- SEE "EROSION AND SEDIMENTATION CONTROL GENERAL NOTES" WHICH ARE TO BE AN INTEGRAL PART OF THIS PROCESS.
- INSTALL SILT FENCING AND/OR HAY BALE BARRIERS AS PER DETAILS AND AT SEDIMENT MIGRATION.
- CONSTRUCT TREATMENT SWALES, LEVEL SPREADERS AND DETENTION STRUCTURES AS DEPICTED ON DRAWINGS.
- INSTALL TEMPORARY GRAVEL CONSTRUCTION ENTRANCE(S) AS PER DETAIL AND AT LOCATIONS SHOWN ON THE DRAWINGS. MAINTAIN (TOP DRESS) REGULARLY TO PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC STREETS.
- STRIP AND STOCKPILE TOPSOIL. STABILIZE PILES OF SOIL CONSTRUCTION MATERIAL.
- ROUGH GRADE SITE. INSTALL CULVERTS AND ROAD DITCHES.
- FINISH GRADE AND COMPACT SITE.
- RE-SPREAD AND ADD TOPSOIL TO ALL ROADSIDE SLOPES. TOTAL TOPSOIL THICKNESS TO BE A MINIMUM OF FOUR TO SIX INCHES.
- STABILIZE ALL AREAS OF BARE SOIL WITH MULCH AND SEEDING.
- RE-SEED PER EROSION AND SEDIMENTATION CONTROL GENERAL NOTES.
- SILT FENCING AND HAY BALES TO REMAIN AND BE MAINTAINED FOR TWENTY FOUR MONTHS AFTER CONSTRUCTION TO INSURE ESTABLISHMENT OF ADEQUATE SOIL STABILIZATION AND VEGETATIVE COVER. ALL SILT FENCING, HAY BALES AND TRAPPED SILT ARE THEN TO BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF.
- PERIMETER CONTROLS SHALL BE INSTALLED PRIOR TO EARTH MOVING OPERATIONS.
- PONDS AND SWALES SHALL BE INSTALLED EARLY ON IN THE CONSTRUCTION SEQUENCE - BEFORE ROUGH GRADING THE SITE.
- ALL DITCHES AND SWALES SHALL BE STABILIZED PRIOR TO DIRECTING RUNOFF TO THEM.
- ALL ROADWAYS AND PARKING LOTS SHALL BE STABILIZED WITHIN 12 HOURS OF ACHIEVING FINISHED GRADE.
- ALL CUT AND FILL SLOPES SHALL BE SEEDED/LOAMED WITHIN 12 HOURS OF ACHIEVING FINISH GRADE.
- ALL EROSION CONTROLS SHALL BE INSPECTED WEEKLY AND AFTER EVERY HALF-INCH OF RAINFALL.
- BUOYANCY CALCULATIONS HAVE NOT BEEN PERFORMED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ANCHORAGE FOR ALL TANKS WHERE REQUIRED.

WINTER CONSTRUCTION NOTES (OCTOBER 15 TO MAY 1):

- ALL PROPOSED VEGETATED AREAS WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH OR WHICH ARE DISTURBED AFTER OCTOBER 15TH SHALL BE STABILIZED BY SEEDING AND INSTALLING EROSION CONTROL BLANKETS ON SLOPES GREATER THAN 3:1, AND SEEDING AND PLACING 3 TO 4 TONS OF MULCH PER ACRE, SECURED WITH ANCHORED NETTING. ELSEWHERE, THE INSTALLATION OF EROSION CONTROL BLANKETS OR MULCH AND NETTING SHALL NOT OCCUR OVER ACCUMULATED SNOW OR ON FROZEN GROUND AND SHALL BE COMPLETED IN ADVANCE OF THAW OR SPRING MELT EVENT.
- ALL DITCHES OR SWALES WHICH DO NOT EXHIBIT A MINIMUM OF 85% VEGETATIVE GROWTH BY OCTOBER 15TH, OR WHICH ARE DISTURBED AFTER OCTOBER 15TH, SHALL BE STABILIZED TEMPORARILY WITH STONE OR EROSION CONTROL BLANKETS APPROPRIATE FOR THE DESIGN FLOW CONDITIONS.
- AFTER OCTOBER 15TH, INCOMPLETE ROAD OR PARKING SURFACES, WHERE WORK HAS STOPPED FOR THE WINTER SEASON, SHALL BE PROTECTED WITH A MINIMUM OF 3 INCHES OF CRUSHED GRAVEL PER MHDOT ITEM 304.3.

ROCK INFILTRATION AREA MAINTENANCE:

THE ROCK INFILTRATION AREA SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR STORM. IF THE ROCK HAS BEEN DISPLACED, UNDERMINED, OR DAMAGED, IT SHOULD BE REPAIRED IMMEDIATELY. THE CHANNEL IMMEDIATELY BELOW ANY OUTLET SHOULD BE CHECKED TO SEE THAT EROSION IS NOT OCCURRING. THE DOWNSTREAM CHANNEL SHOULD BE KEPT CLEAR OF OBSTRUCTIONS SUCH AS FALLEN TREES, DEBRIS, AND SEDIMENT THAT COULD CHANGE FLOW PATTERNS AND/OR TAIL WATER DEPTHS ON THE PIPES. REPAIRS MUST BE CARRIED OUT IMMEDIATELY TO AVOID ADDITIONAL DAMAGE TO THE OUTLET PROTECTION APRON.

EROSION AND SEDIMENTATION CONTROL - GENERAL NOTES:

- CONDUCT ALL CONSTRUCTION IN A MANNER AND SEQUENCE THAT CAUSES THE LEAST PRACTICAL DISTURBANCE OF THE PHYSICAL ENVIRONMENT, BUT IN NO CASE SHALL EXCEED 2 ACRES AT ANY ONE TIME BEFORE DISTURBED AREAS ARE STABILIZED.
- ALL EROSION AND SEDIMENTATION CONTROL MEASURES IN THE PLAN SHALL MEET THE DESIGN BASED ON NEW HAMPSHIRE STORMWATER MANUAL, VOLUMES 1-3, DATED DECEMBER 2008, PREPARED BY NHDES.
- AN AREA SHALL BE CONSIDERED STABLE IF ONE OF THE FOLLOWING HAS OCCURRED:
 - BASE COURSE GRAVELS HAVE BEEN INSTALLED IN AREAS TO BE PAVED.
 - A MINIMUM OF 85% VEGETATED GROWTH HAS BEEN ESTABLISHED.
 - A MINIMUM OF 3" OF NON-EROSIVE MATERIAL SUCH AS STONE OR RIP RAP HAS BEEN INSTALLED.
- EROSION CONTROL BLANKETS HAVE BEEN PROPERLY INSTALLED.
- ALL AREAS SHALL BE STABILIZED WITHIN 45 DAYS OF INITIAL DISTURBANCE.
- SEE WINTER CONSTRUCTION NOTES IF SCHEDULE AND DATES ARE APPLICABLE.
- ALL DITCHES, SWALES AND PONDS MUST BE STABILIZED PRIOR TO DIRECTING FLOW TO THEM.
- ALL GROUND AREAS OPENED UP FOR CONSTRUCTION WILL BE STABILIZED IN THE SHORTEST PRACTICAL TIME. ALL SOILS FINISH GRADED MUST BE STABILIZED WITHIN SEVENTY TWO HOURS OF DISTURBANCE.
- EMPLOY TEMPORARY EROSION AND SEDIMENTATION CONTROL DEVICES AS DETAILED ON THIS PLAN AS NECESSARY UNTIL ADEQUATE STABILIZATION HAS BEEN ASSURED.
- TEMPORARY & LONG TERM SEEDINGS, USE SEED MIXTURES, FERTILIZER, LIME AND MULCHING AS RECOMMENDED (SEE SEEDING AND STABILIZATION NOTES).
- STRAW OR HAY BALE BARRIERS AND SILTATION FENCING TO BE SECURELY EMBEDDED AND STAKED AS DETAILED. WHEREVER POSSIBLE A VEGETATED STRIP OF AT LEAST TWENTY FIVE FEET IS TO BE KEPT BETWEEN SILT FENCE AND ANY EDGE OF WET AREA.
- SEEDED AREAS WILL BE FERTILIZED AND RE-SEED AS NECESSARY TO ENSURE VEGETATIVE ESTABLISHMENT.
- SEDIMENT BASIN(S), IF REQUIRED, TO BE CHECKED AFTER EACH SIGNIFICANT RAINFALL AND CLEANED AS NEEDED TO RETAIN DESIGN CAPACITY.
- STRAW BALE AND/OR SILT FENCE BARRIERS WILL BE CHECKED REGULARLY AND AFTER EACH SIGNIFICANT RAINFALL. NECESSARY REPAIRS WILL BE MADE TO CORRECT UNDERMINING OR DETERIORATION OF THE BARRIER AS WELL AS CLEANING, REMOVAL AND PROPER DISPOSAL OF TRAPPED SEDIMENT.
- TREATMENT SWALES WILL BE CHECKED WEEKLY AND REPAIRED WHEN NECESSARY UNTIL ADEQUATE VEGETATIVE COVER HAS BEEN ESTABLISHED.
- THE PROJECT IS TO BE MANAGED IN A MANNER THAT MEETS THE REQUIREMENTS AND INTENT OF RSA 430:53 AND CHAPTER AGR 3800 RELATIVE TO INVASIVE SPECIES.
- TEMPORARY WATER DIVERSION (SWALES, BASINS, ETC.) MUST BE USED AS NECESSARY UNTIL AREAS ARE STABILIZED.

SEEDING AND STABILIZATION FOR LOAMED SITE:

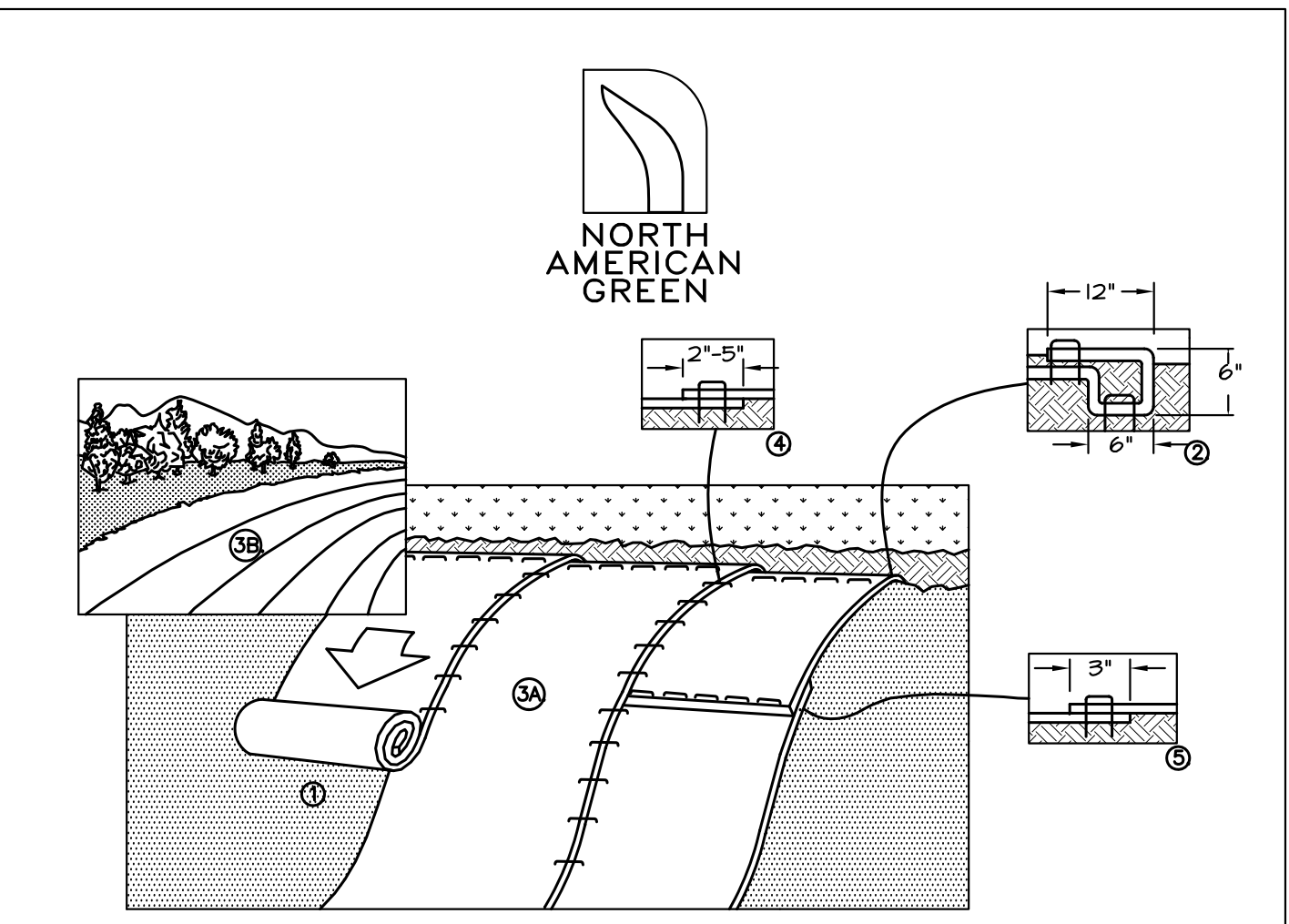
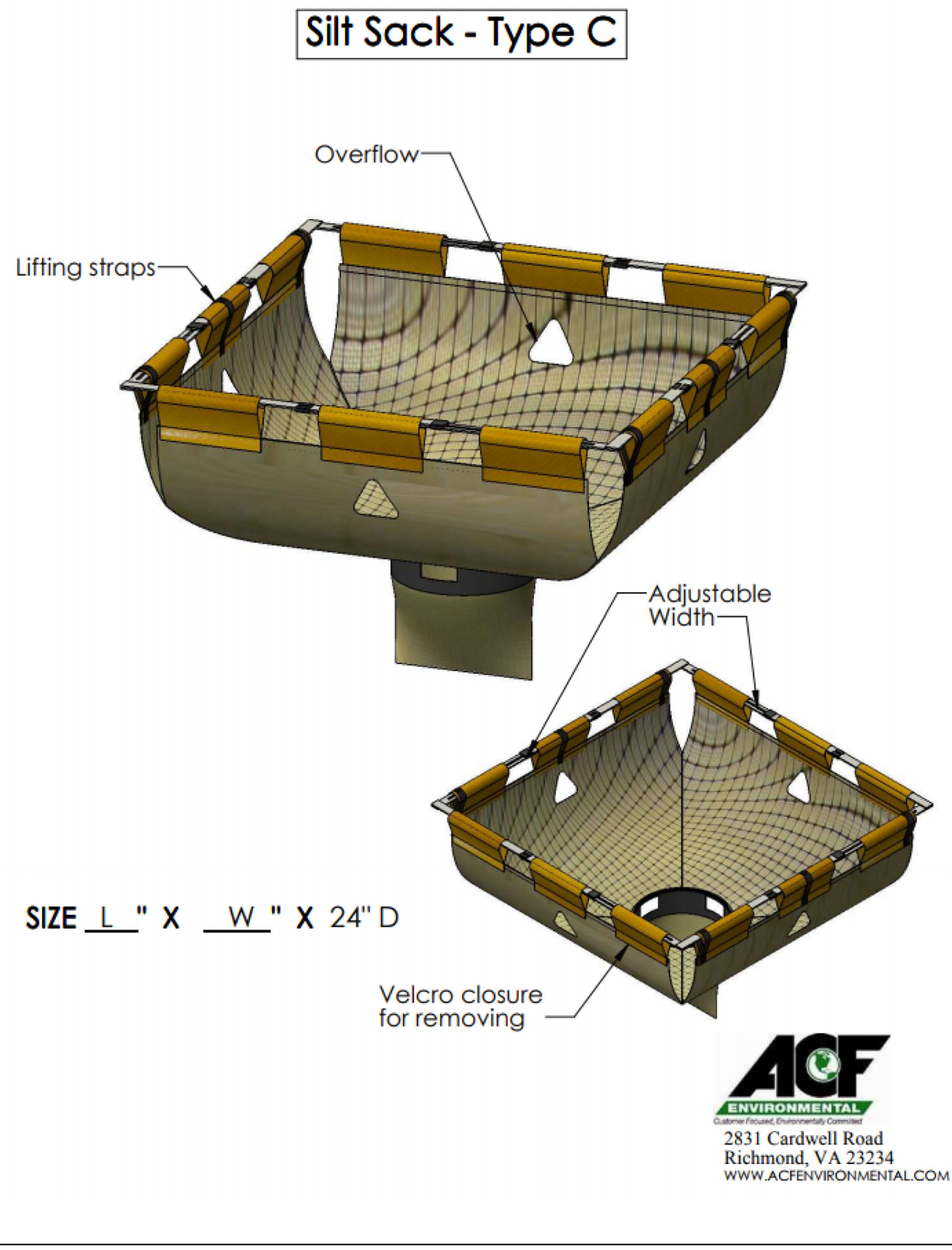
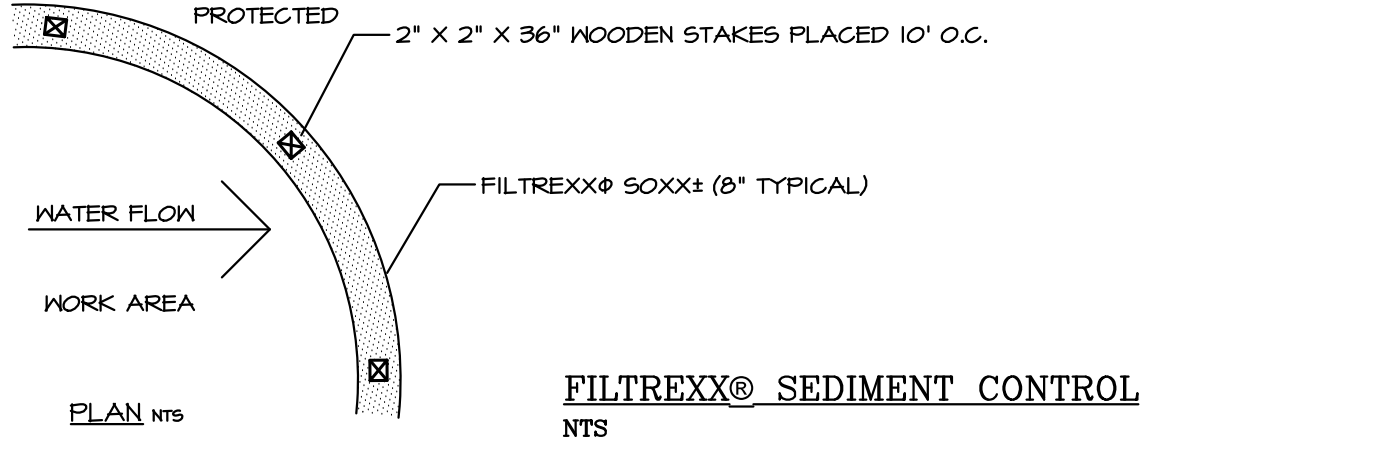
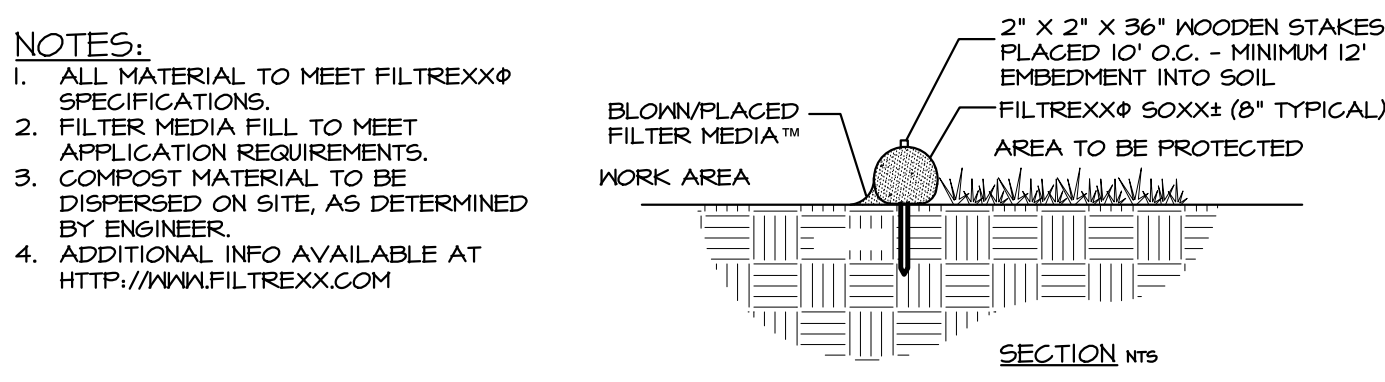
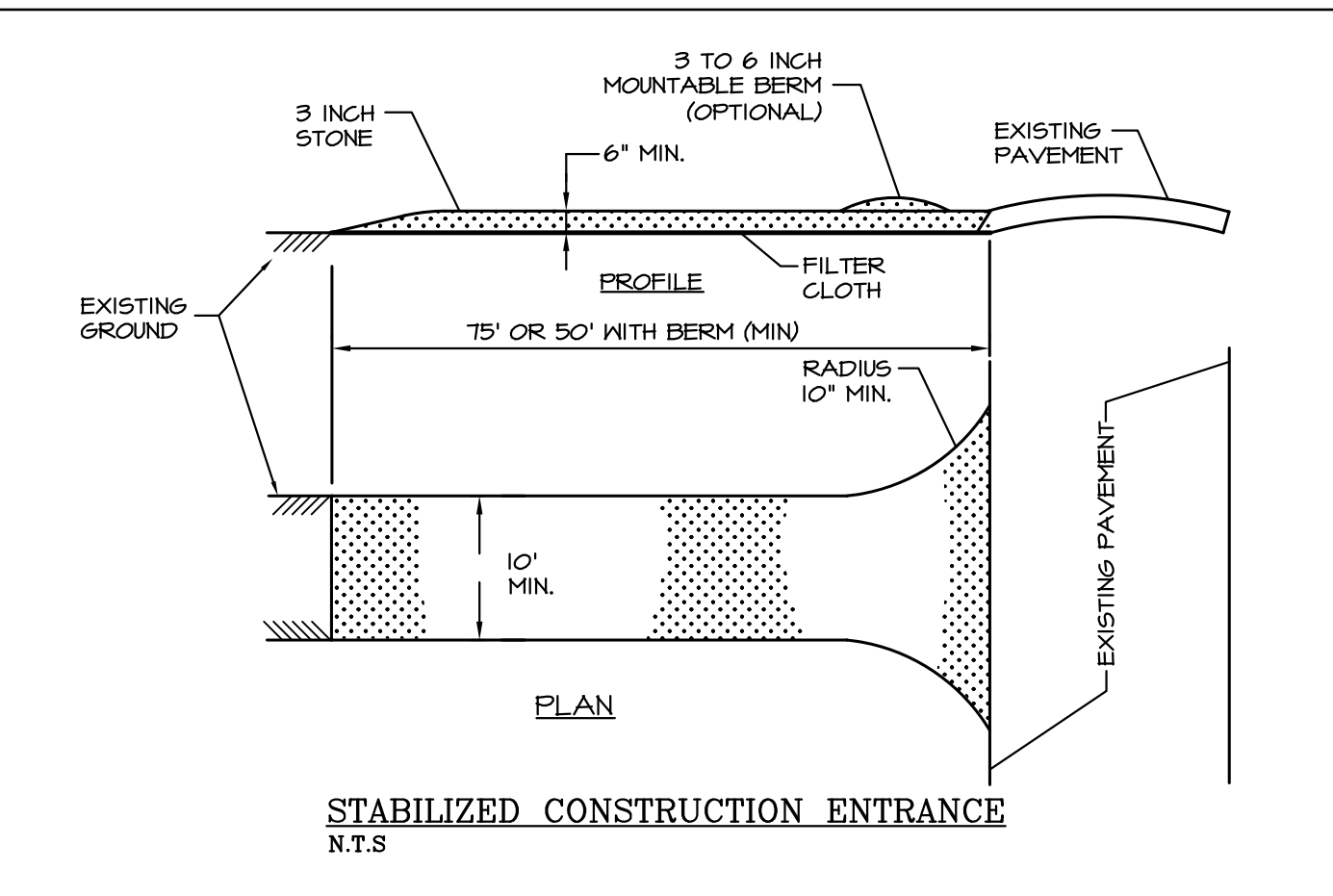
- FOR TEMPORARY & LONG TERM SEEDINGS (BY SEPTEMBER 15 OF THE SAME YEAR OF DISTURBANCE) USE AGWAY'S SOIL CONSERVATION GRASS SEED OR EQUAL.
- COMPONENTS: ANNUAL RYE GRASS, PERENNIAL RYE GRASS, WHITE CLOVER, 2 FESCUES, SEED AT A RATE OF 100 POUNDS PER ACRE.
- FERTILIZER & LIME: NITROGEN (N) 50 LBS/ACRE, PHOSPHATE (P2O5) 100 LBS/ACRE, POTASH (K2O) 100 LBS/ACRE, LIME 2000 LBS/ACRE.
- MULCH: HAY OR STRAW 15-2 TONS/ACRE.
- GRADING AND SHAPING: SLOPES SHALL NOT BE STEEPER THAN 2:1; 3:1 SLOPES OR FLATTER ARE PREFERRED. WHERE MOUING WILL BE DONE, 3:1 SLOPES OR FLATTER ARE RECOMMENDED.
- SEED BED PREPARATION - SURFACE AND SEEPAGE WATER SHOULD BE DRAINED OR DIVERTED FROM THE SITE TO PREVENT DROWNING OR WINTER KILLING OF THE PLANTS. - STONES LARGER THAN 4 INCHES AND TRASH SHOULD BE REMOVED BECAUSE THEY INTERFERE WITH SEEDING AND FUTURE MAINTENANCE OF THE AREA. WHERE FEASIBLE, THE SOIL SHOULD BE TILLED TO A DEPTH OF ABOUT 4 INCHES TO PREPARE A SEEDBED AND MIX FERTILIZER AND LIME INTO THE SOIL. THE SEEDBED SHOULD BE LEFT IN A REASONABLY FIRM AND SMOOTH CONDITION. THE LAST TILLAGE OPERATION SHOULD BE PERFORMED ACROSS THE SLOPE WHEREVER PRACTICAL.

STABILIZATION CONSTRUCTION ENTRANCE SPECIFICATIONS:

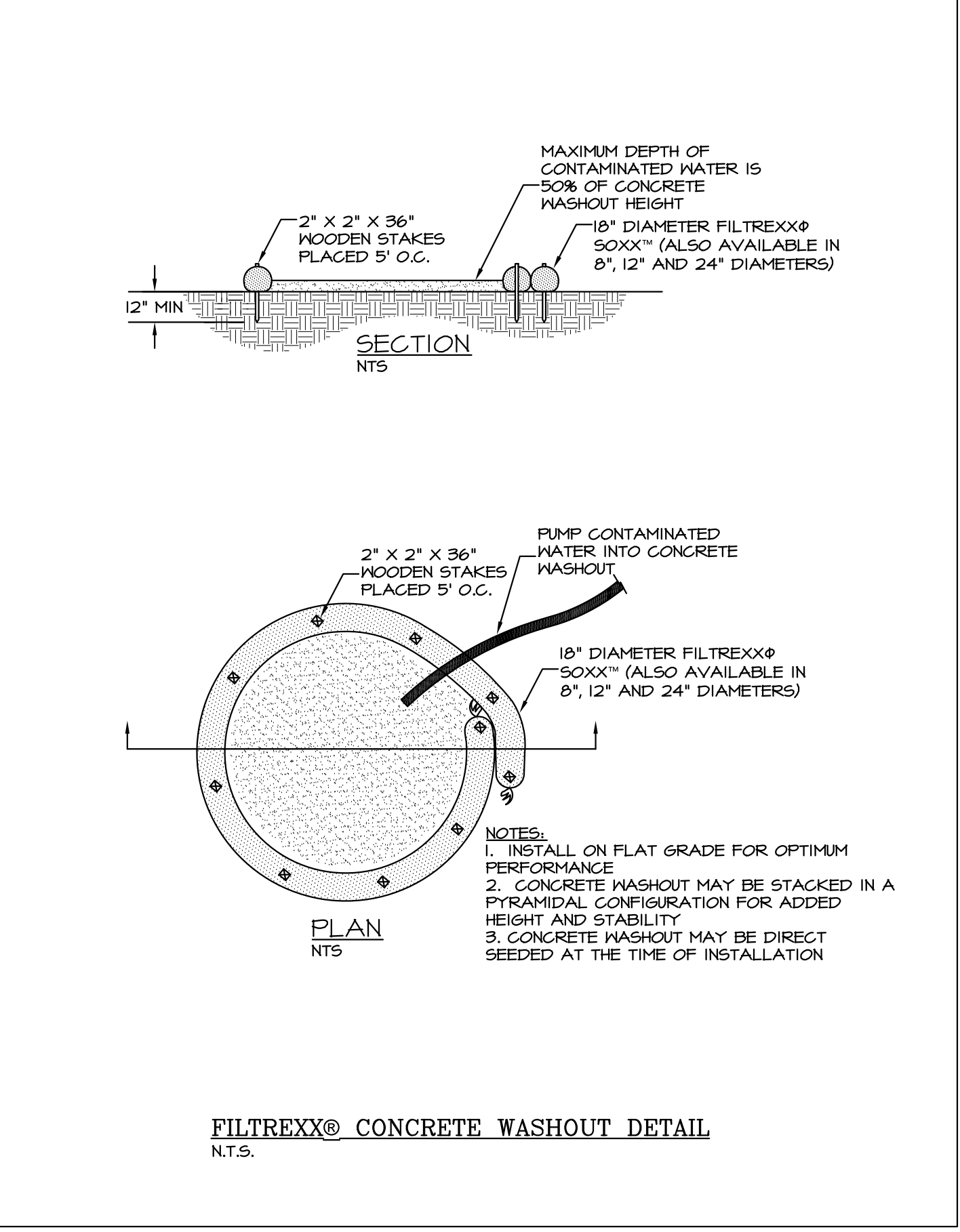
- STONE FOR A STABILIZED CONSTRUCTION ENTRANCE SHALL BE 3 INCH STONE (MINIMUM), RECLAIMED STONE, OR RECYCLED CONCRETE EQUIVALENT.
- THE LENGTH OF THE STABILIZED ENTRANCE SHALL NOT BE LESS THAN 15 FEET (OR 50 FEET WITH A 3 TO 6 INCH MOUNTABLE BERM).
- THE THICKNESS OF THE STONE FOR THE STABILIZATION ENTRANCE SHALL NOT BE LESS THAN 6 INCHES.
- THE WIDTH OF THE ENTRANCE SHALL NOT BE LESS THAN THE FULL WIDTH OF THE ENTRANCE WHERE INGRESS OR EGRESS OCCURS OR 10 FEET WHICH EVER IS GREATER.
- GEOTEXTILE FILTER CLOTH SHALL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING THE STONE.
- ALL SURFACE WATER THAT IS FLOWING TO OR DIVERTED TOWARDS THE CONSTRUCTION ENTRANCE SHALL BE PIPED BENEATH THE ENTRANCE. IF PIPING IS IMPRACTICAL, A BERM WITH 5:1 SLOPES THAT CAN BE CROSSED BY VEHICLES MAY BE SUBSTITUTED FOR THE PIPE.
- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING OF ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, WASHED, OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED PROMPTLY.
- WHEELS SHALL BE CLEANED TO REMOVE MUD PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.

FILTREXX LAND IMPROVEMENT SYSTEMS INSPECTION & MAINTENANCE:

- CONSULT FILTREXX SWPP CUT SHEETS FOR ALL FILTREXX PRODUCTS PRIOR TO INSTALLATION AND FOR MAINTENANCE GUIDELINES. [HTTP://WWW.FILTREXX.COM/DESIGN_CUT_SHEETS.HTM](http://www.filtrex.com/design_cut_sheets.htm)
- ROUTINE INSPECTION SHOULD BE CONDUCTED WITHIN 24 HRS OF A RUNOFF EVENT OR AS DESIGNATED BY THE REGULATING AUTHORITY. UNITS SHOULD BE REGULARLY INSPECTED TO MAKE SURE THEY MAINTAIN THEIR SHAPE AND ARE PRODUCING ADEQUATE HYDRAULIC FLOW-THROUGH, DITCH/CHANNEL EROSION CONTROL, AND SEDIMENT REMOVAL.
- IF PONDING BECOMES EXCESSIVE, ADDITIONAL CHECK DAMS, LEVEL SPREADERS, OR SEDIMENT CONTROL UNITS FOR SEDIMENT REMOVAL MAY BE REQUIRED.
- SEDIMENT ACCUMULATION SHOULD BE REMOVED ONCE IT REACHES THE HEIGHT OF THE CHECK DAM OR UNIT. ALTERNATIVELY, ANOTHER UNIT MAY BE INSTALLED SLIGHTLY UPSLOPE ON TOP OF THE EXISTING ONE. THIS PROCESS IS NOT CONSIDERED A SOIL DISTURBING ACTIVITY.
- STORM DEBRIS ACCUMULATION BEHIND CHECK DAMS, LEVEL SPREADER, SEDIMENT CONTROL UNITS, ETC. SHOULD NEVER BE HIGHER THAN THE SIDES OF THE CHECK DAM/UNIT. STORM RUNOFF OVERFLOW SHALL MAINTAIN THE UNITS IN A FUNCTIONAL CONDITION AT ALL TIMES AND IT SHALL BE ROUTINELY INSPECTED.
- IF A UNIT HAS BEEN DAMAGED, IT SHALL BE REPAIRED, OR REPLACED IF BEYOND REPAIR.
- THE CONTRACTOR SHALL REMOVE SEDIMENT AT THE BASE OF THE UPSLOPE SIDE OF UNITS WHEN ACCUMULATION HAS REACHED 1/2 OF THE EFFECTIVE HEIGHT OF THE SOXX, OR AS DIRECTED BY THE ENGINEER.
- AS AN ALTERNATIVE, ANOTHER SOXX UNIT MAY BE INSTALLED ADJACENT AND PARALLEL TO THE UPSLOPE SIDE OF THE ORIGINAL TO INCREASE SEDIMENT STORAGE CAPACITY. SOXX SEDIMENT BACKUP IN CENTER OF THE DITCH/CHANNEL SHALL REMAIN LOWER THAN THE SIDES.
- IF SOXX UNIT BECOMES CLOGGED WITH DEBRIS AND SEDIMENT, IMMEDIATE REMOVAL OF DEBRIS AND SEDIMENT SHOULD BE CONDUCTED TO ASSURE PROPER DRAINAGE AND WATER FLOW THROUGH THE DITCH OR CHANNEL. STORM RUNOFF OVERFLOW OF THE SOXX UNIT IS ACCEPTABLE.
- SOXX UNITS SHALL BE MAINTAINED UNTIL DISTURBED AREA AROUND THE DEVICE HAS BEEN PERMANENTLY STABILIZED AND CONSTRUCTION ACTIVITY HAS CEASED.
- THE FILTERMEDIUM MAY BE DISPENSED ON SITE ONCE DISTURBED AREA HAS PERMANENTLY STABILIZED, CONSTRUCTION ACTIVITY CEASED, OR DETERMINED BY THE ENGINEER.
- PERMANENT VEGETATED FILTER STRIPS WILL BE LEFT INTACT.



- PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED. NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA. CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
 - BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" (15cm) DEEP X 6" (15cm) WIDE TRENCH WITH APPROXIMATELY 12" (30cm) OF BLANKET EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH. ANCHOR THE BLANKET WITH A ROW OF STAPLES/STAKES APPROXIMATELY 12" (30cm) APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED SOIL AND FOLD REMAINING 12" (30cm) PORTION OF BLANKET BACK OVER SEED AND COMPACTED SOIL. SECURE BLANKET OVER COMPACTED SOIL WITH A ROW OF STAPLES/STAKES SPACED APPROXIMATELY 12" (30cm) APART ACROSS THE WIDTH OF THE BLANKET.
 - ROLL THE BLANKETS (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE. BLANKETS WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE SOIL SURFACE. ALL BLANKETS MUST BE SECURELY FASTENED TO SOIL SURFACE BY PLACING STAPLES/STAKES IN APPROPRIATE LOCATIONS AS SHOWN IN THE STAPLE PATTERN GUIDE. WHEN USING OPTIONAL DOT SYSTEM, STAPLES/STAKES SHOULD BE PLACED THROUGH EACH OF THE COLORED DOTS CORRESPONDING TO THE APPROPRIATE STAPLE PATTERN.
 - THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2"-5" (5cm-12.5cm) OVERLAP DEPENDING ON BLANKET TYPE. TO ENSURE PROPER SEAM ALIGNMENT, PLACE THE EDGE OF THE OVERLAPPING BLANKET (BLANKET BEING INSTALLED ON TOP) EVEN WITH THE COLORED SEAM MARK ON THE PREVIOUSLY INSTALLED BLANKET.
 - CONSECUTIVE BLANKETS SPLICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 3" (7.5cm) OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" (30cm) APART ACROSS ENTIRE BLANKET WIDTH.
- NOTE:
* IN LOOSE SOIL CONDITIONS, THE USE OF STAPLE OR STAKE LENGTHS GREATER THAN 6" (15cm) MAY BE NECESSARY TO PROPERLY SECURE THE BLANKETS.
- CRITICAL POINTS
A. OVERLAPS AND SEAMS
B. PROTECTED WATER LINE
C. CHANNEL BOTTOM/SIDE SLOPE VERTICES
- * HORIZONTAL STAPLE SPACING SHOULD BE ALTERED IF NECESSARY TO ALLOW STAPLES TO SECURE THE CRITICAL POINTS ALONG THE CHANNEL SURFACE.
** IN LOOSE SOIL CONDITIONS, THE USE OF STAPLE OR STAKE LENGTHS IN EXCESS OF 6" (15 CM) MAY BE NECESSARY TO PROPERLY ANCHOR THE BLANKETS.
- 16644 HIGHWAY 41 NORTH EVANSVILLE, INDIANA 47125
USA 1-800-712-2040 CANADA 1-800-448-2040
www.nagreen.com



SEAL

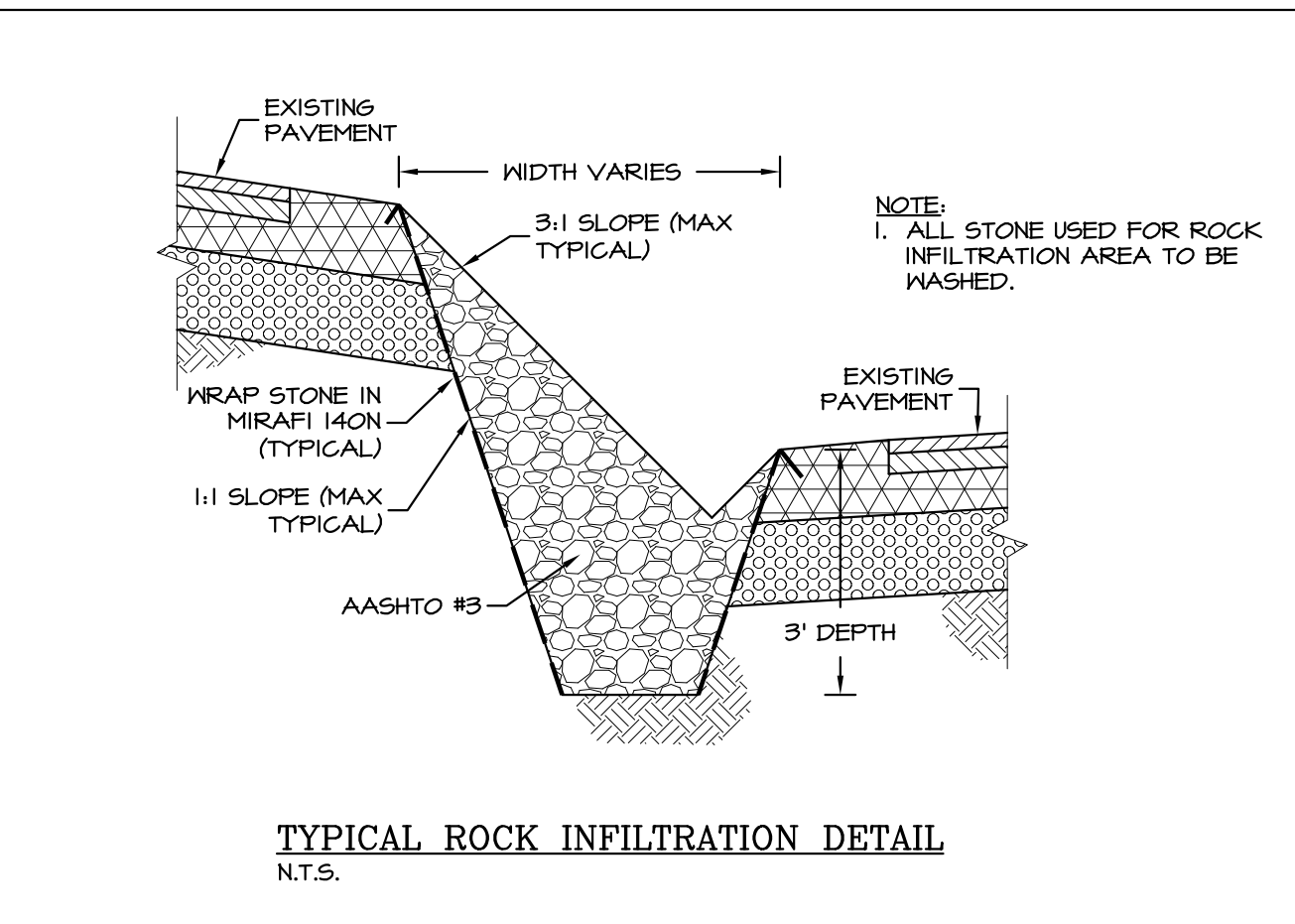
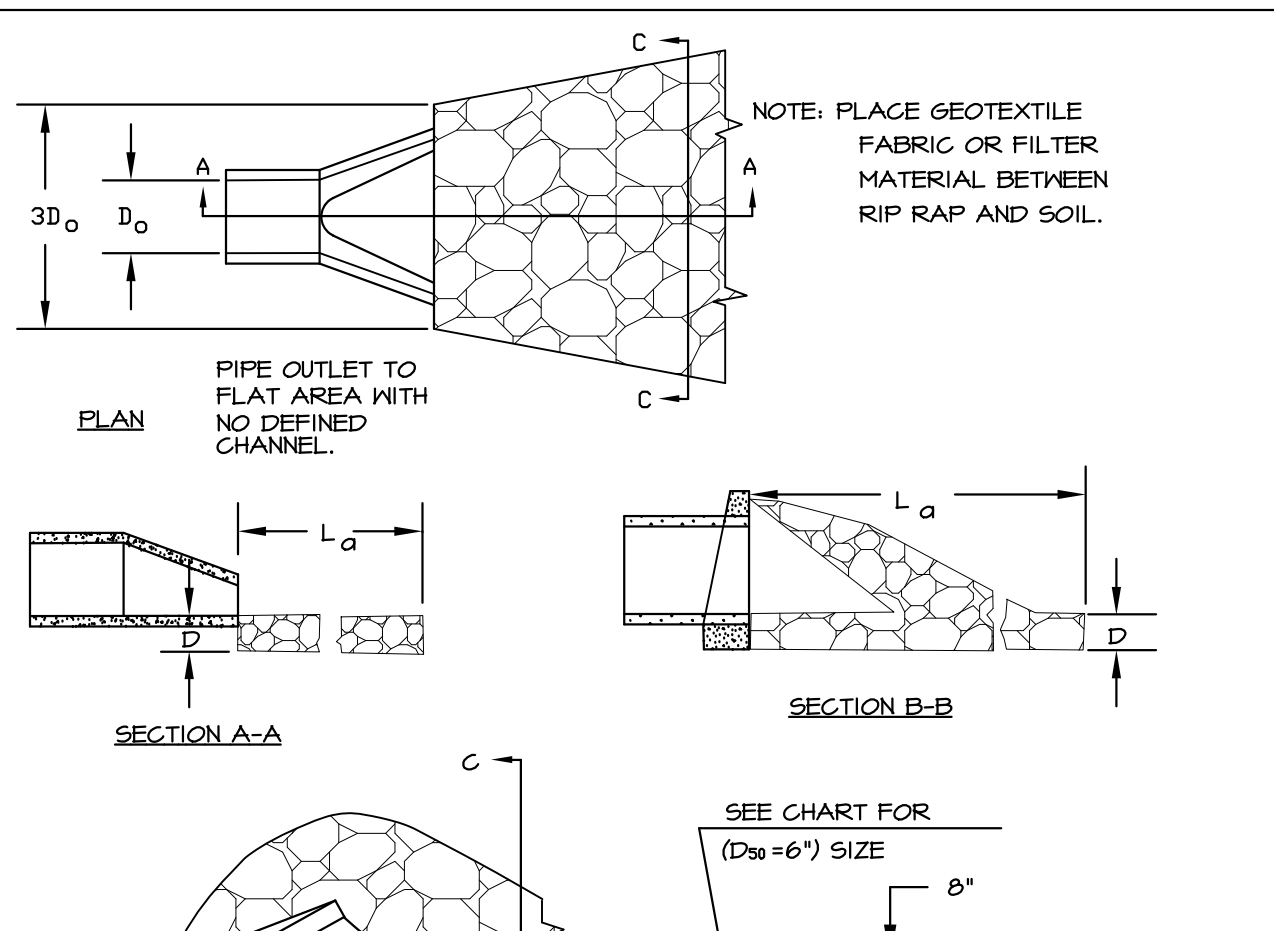
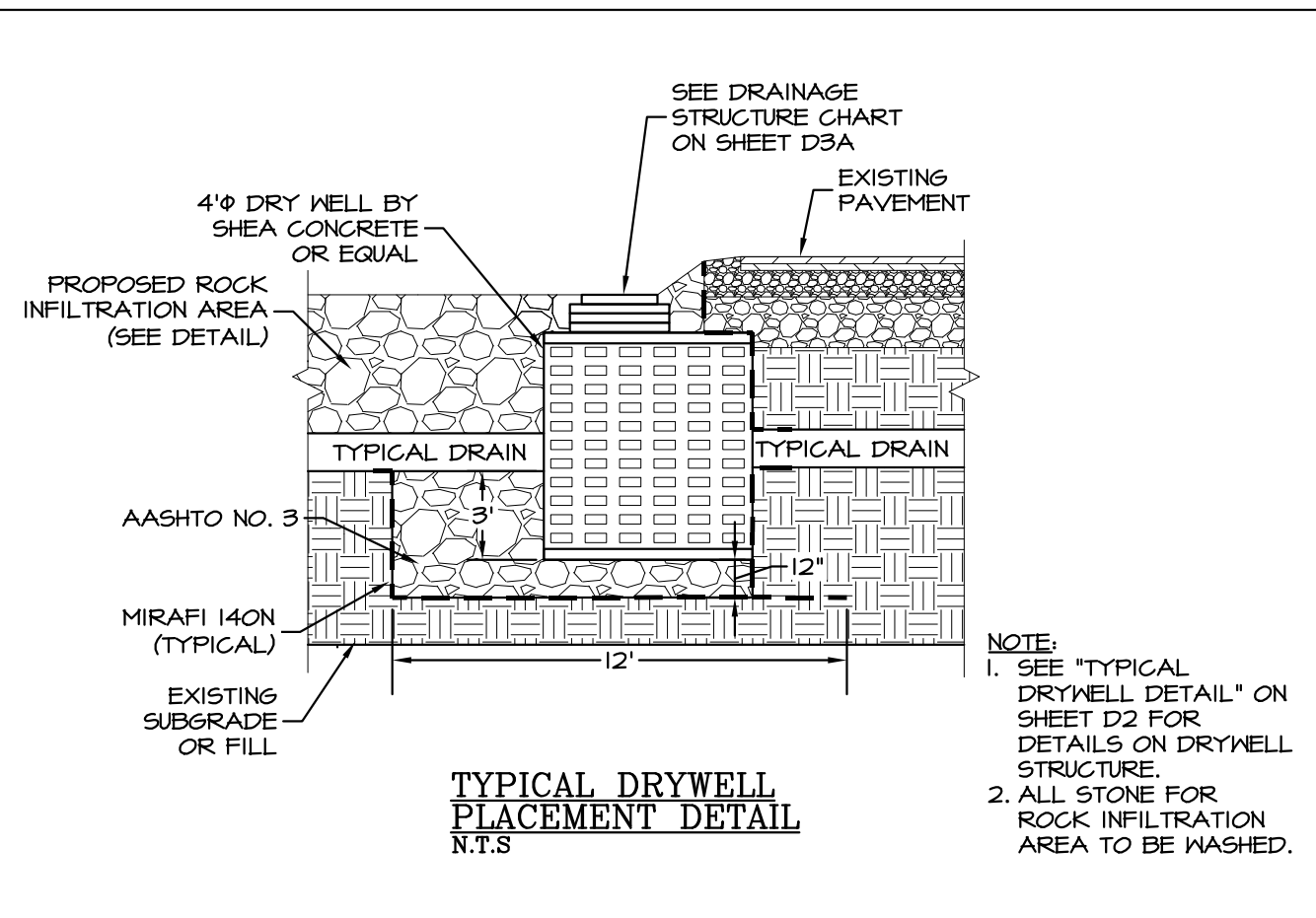
2	APR 20, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
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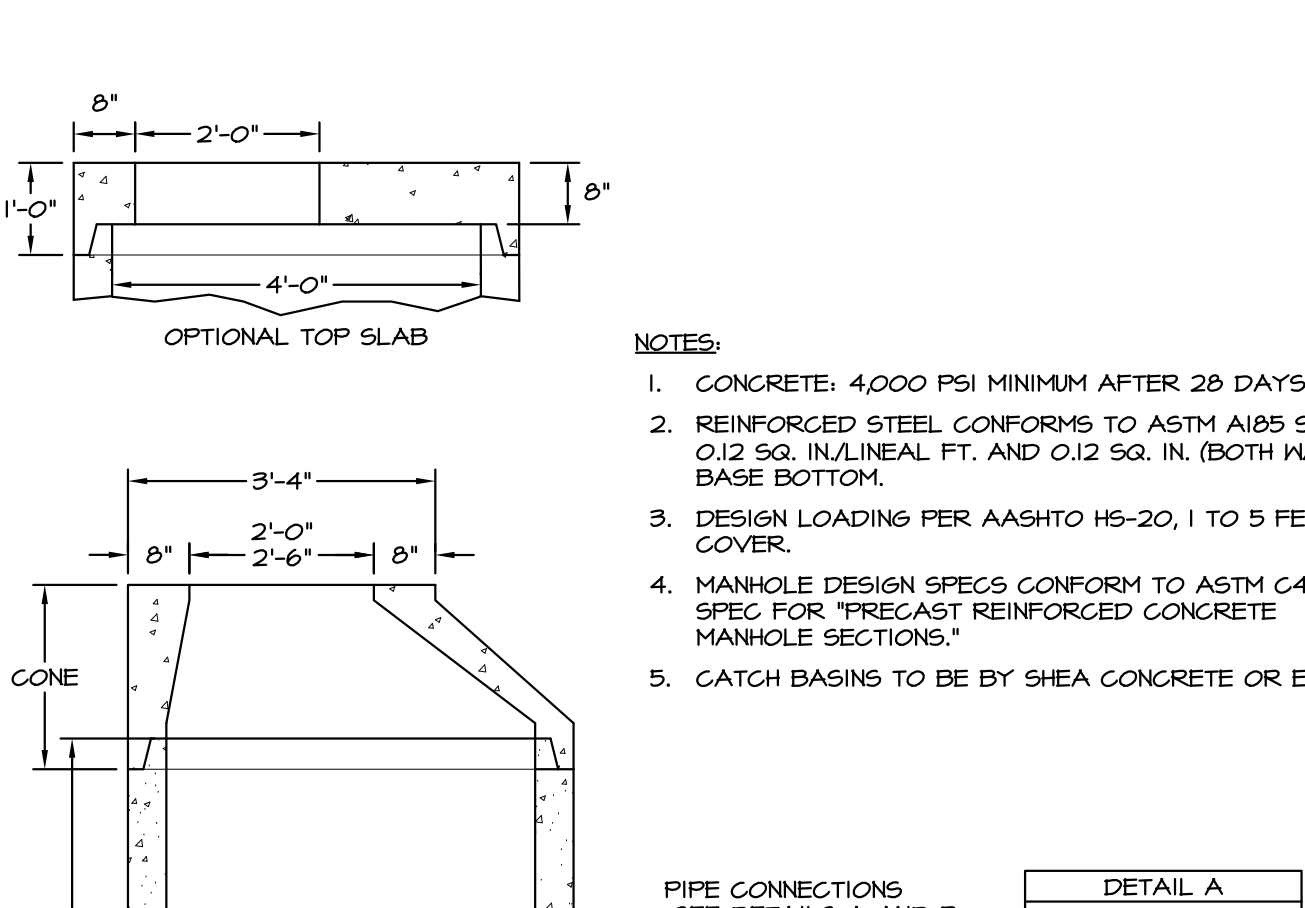
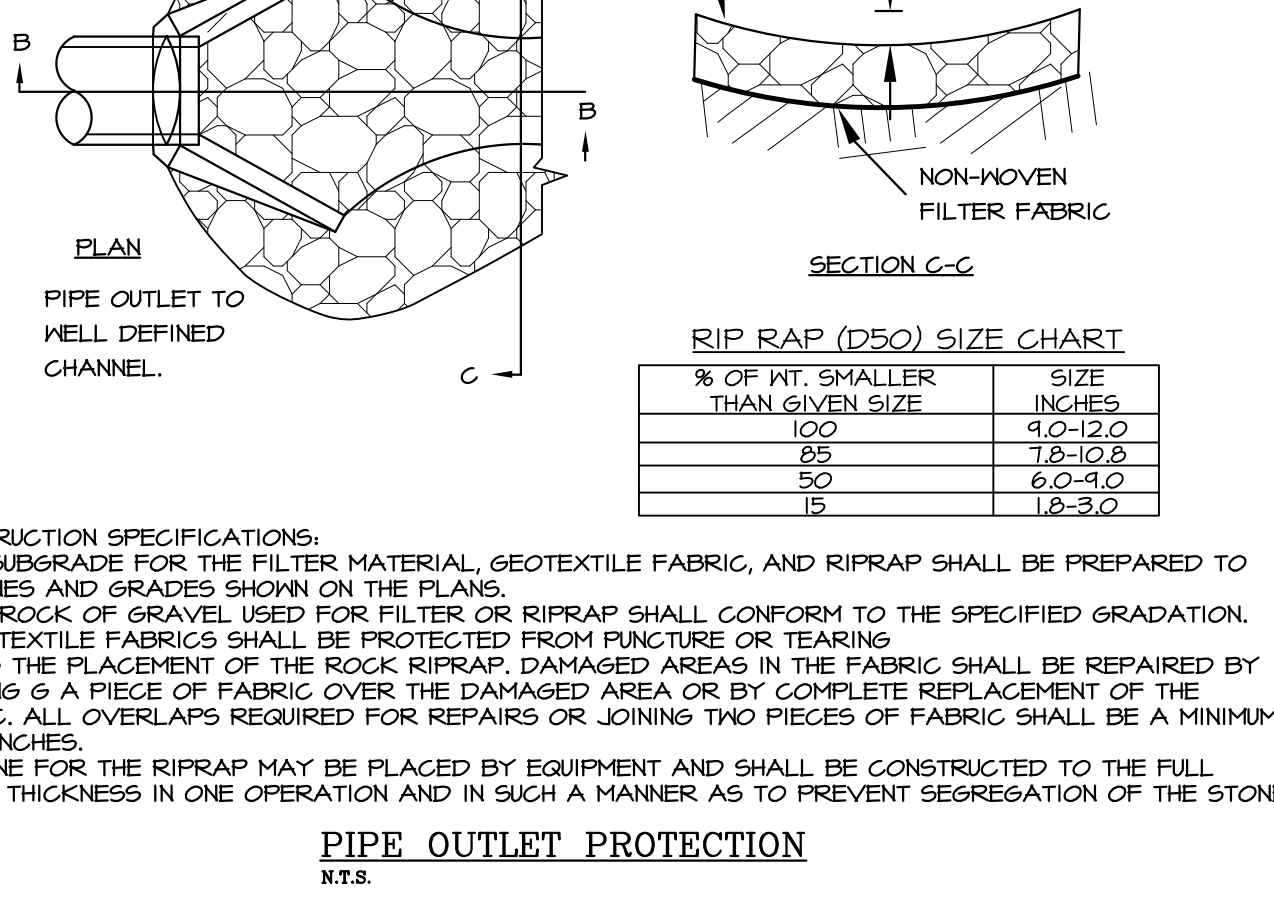
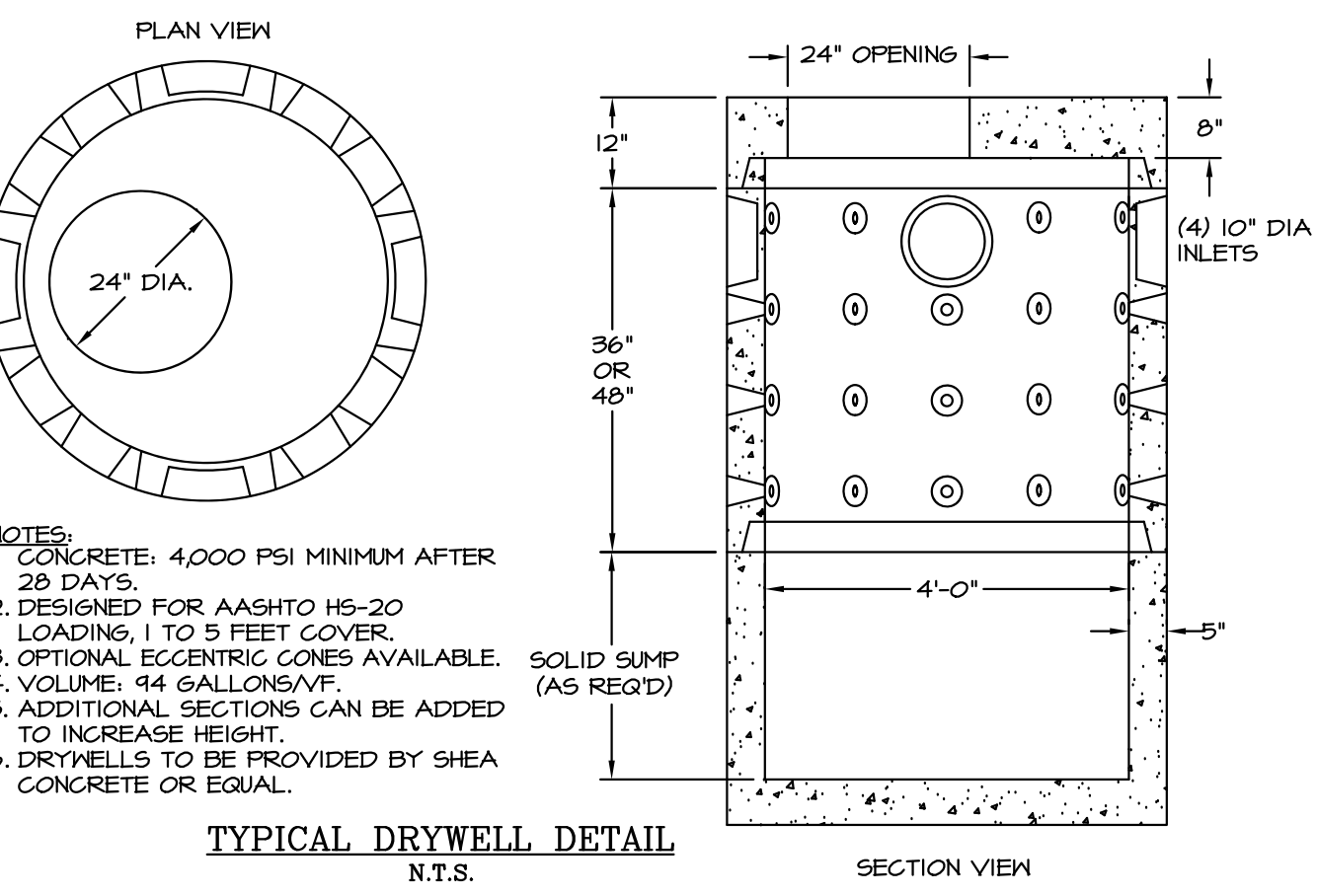
TITLE:
NOTES & EROSION CONTROL DETAILS FOR EXETER COUNTRY CLUB 58 JADY HILL AVENUE (SITE) EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	AS SHOWN	D1

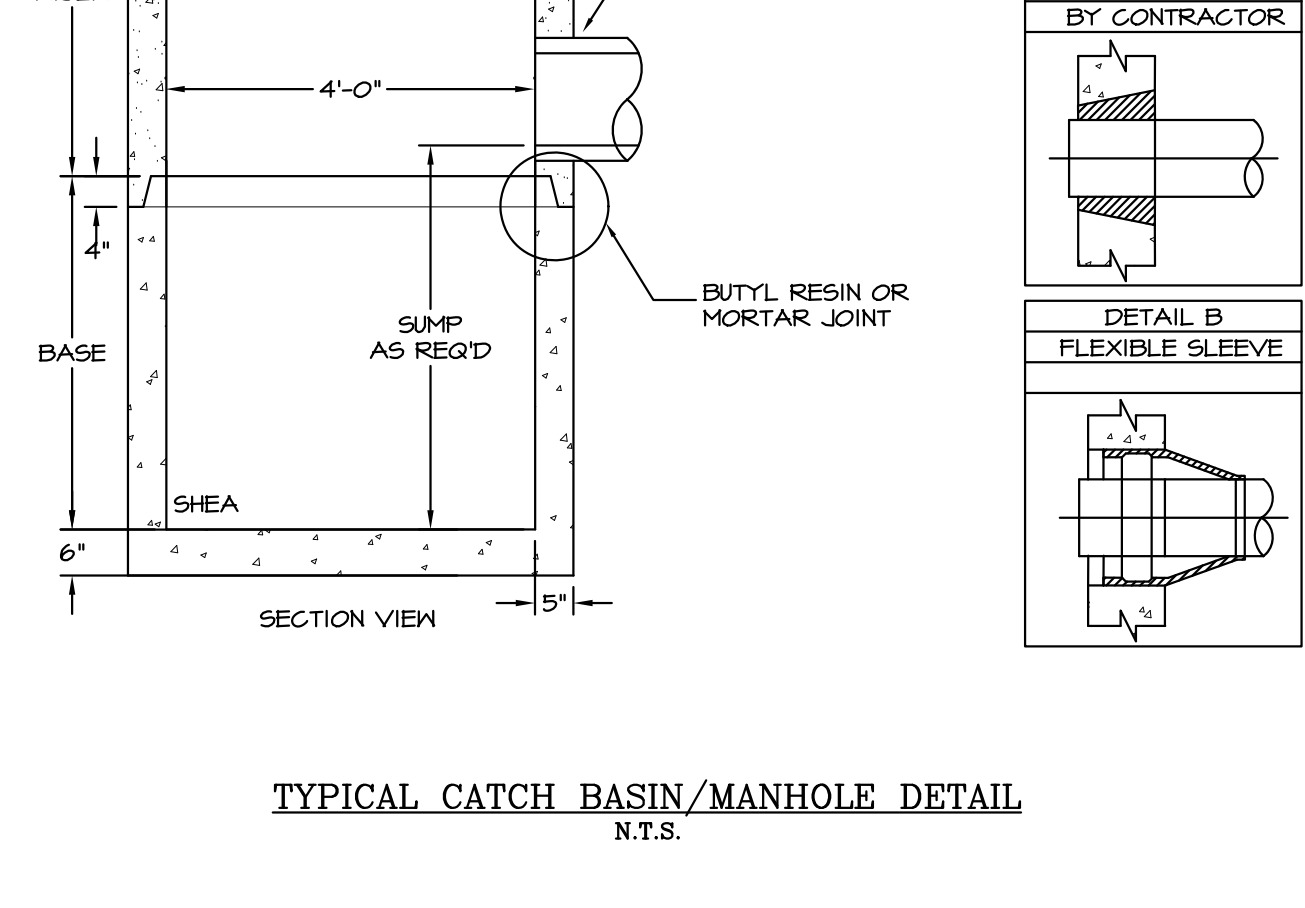
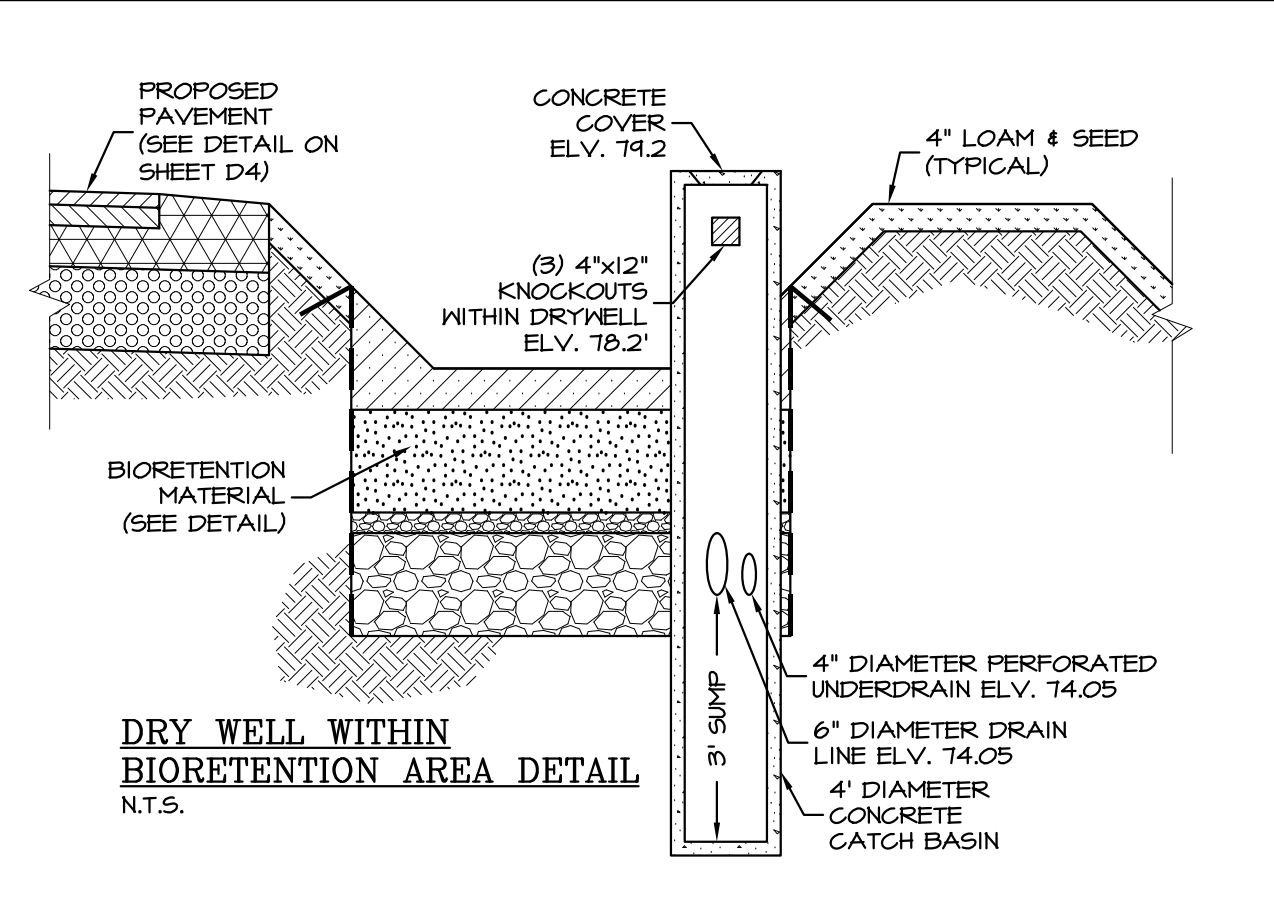
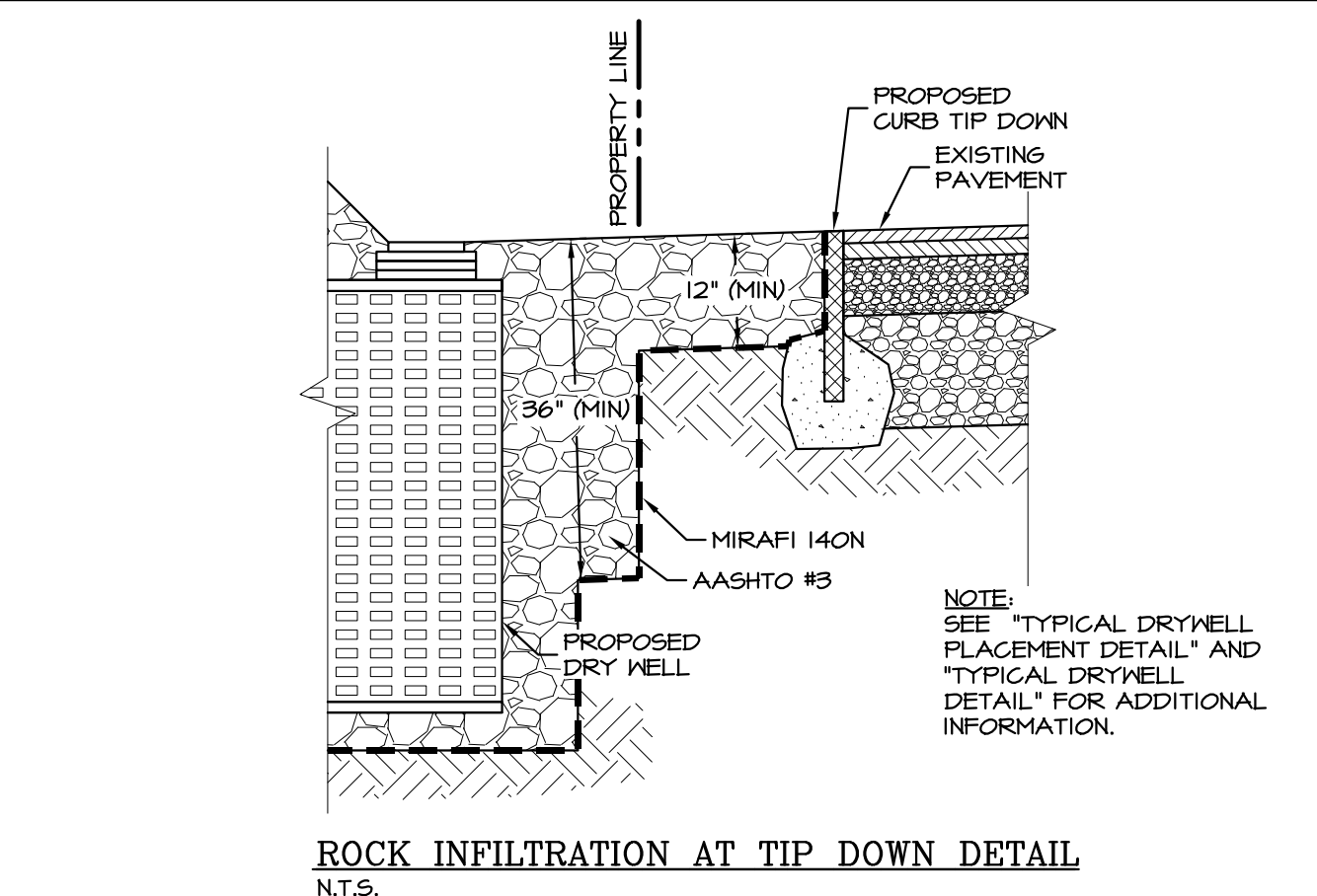


GRADATION FOR AASHTO #3

NOMINAL SIZE SQUARE OPENINGS	AMOUNTS FINER THAN EACH LABORATORY SIEVE (SQUARE OPENINGS), PERCENTAGE BY WEIGHT
100 MM	
4 IN.	
90 MM	
3 1/2 IN.	
75 MM	
3 IN.	
63 MM	100
2 1/2 IN.	
50 MM	90-100
2 IN.	
37.5 MM	35-10
1 1/2 IN.	
25 MM	0-15
1 IN.	
19 MM	
3/4 IN.	
12.5 MM	0-5
1/2 IN.	
4.75 MM	
3/8 IN.	
2.36 MM	
No. 20	
1.18 MM	
No. 16	
600 μm	
No. 30	
300 μm	
No. 50	
150 μm	
No. 100	
75 μm	
No. 200	



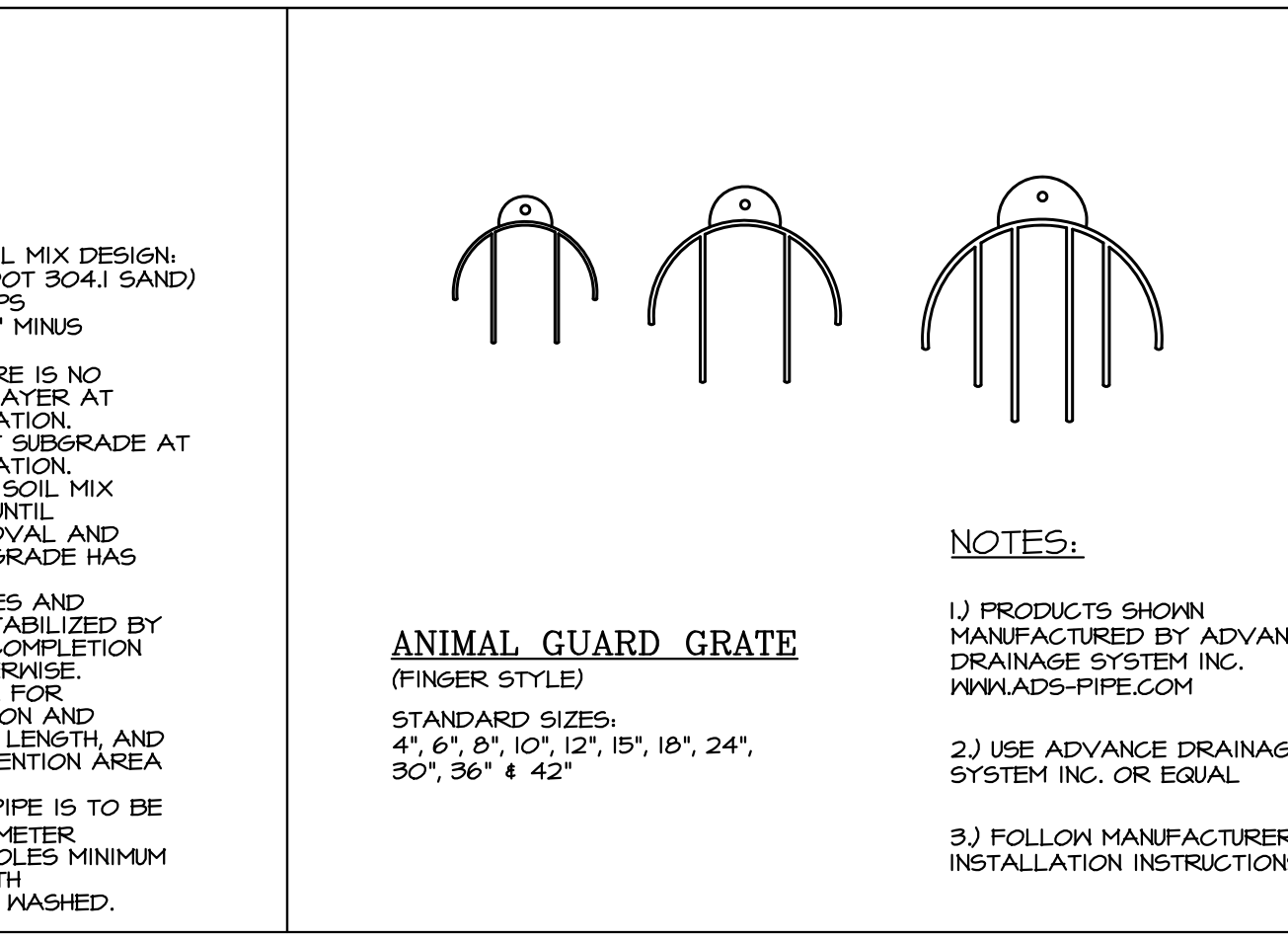
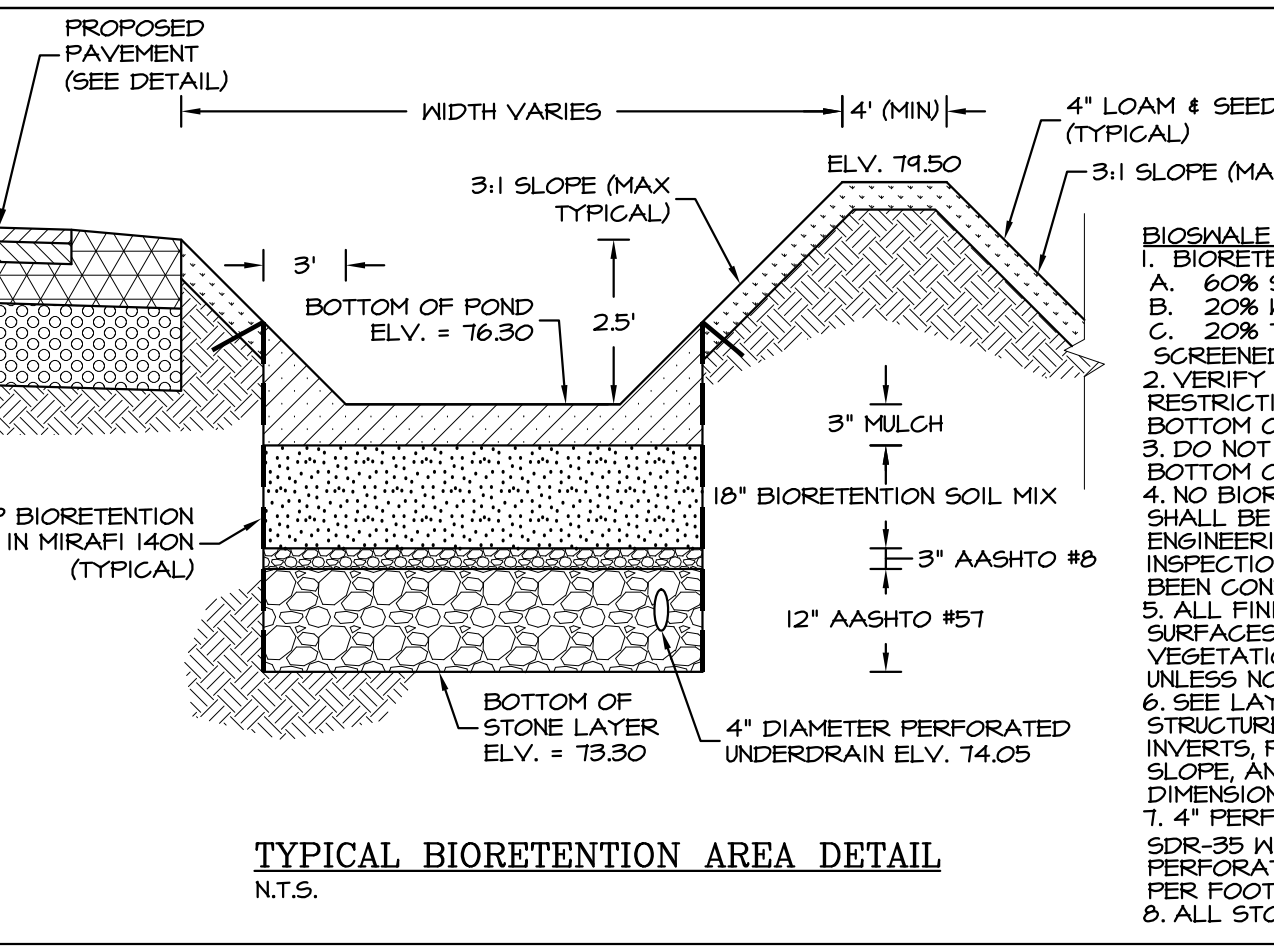
- NOTES:**
- CONCRETE: 4,000 PSI MINIMUM AFTER 28 DAYS.
 - REINFORCED STEEL CONFORMS TO ASTM A185 SPEC. 0.12 SQ. IN./LINEAL FT. AND 0.12 SQ. IN. (BOTH WAYS) BASE BOTTOM.
 - DESIGN LOADING PER AASHTO H5-20, 1 TO 5 FEET COVER.
 - MANHOLE DESIGN SPECS CONFORM TO ASTM C478 SPEC FOR "PRECAST REINFORCED CONCRETE MANHOLE SECTIONS."
 - CATCH BASINS TO BE BY SHEA CONCRETE OR EQUAL.



FLARED END SECTIONS

SIZE	PRODUCT CODE
10" (250mm)	1015NP
12" (300mm) / 15" (375mm)	1215NP
18" (450mm)	1810NP
24" (600mm)	2410NP
30" (750mm)	3015NP
36" (900mm)	3615NP

FLARED END SECTION



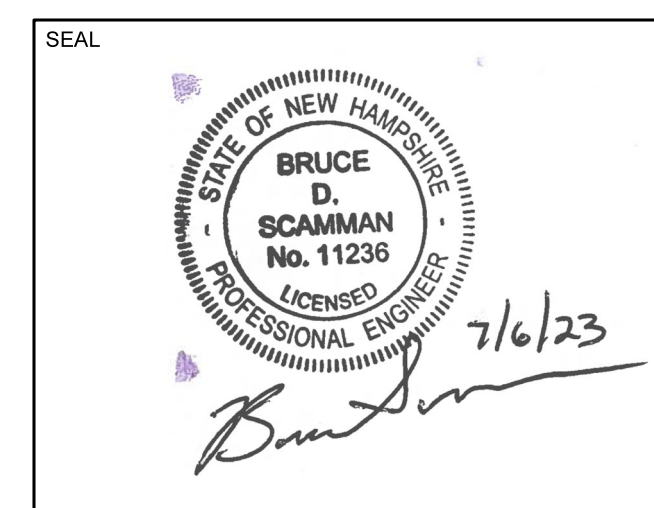
4	JULY 6, 2023	FOR APPROVAL	
3	MAY 23, 2023	FOR APPROVAL	
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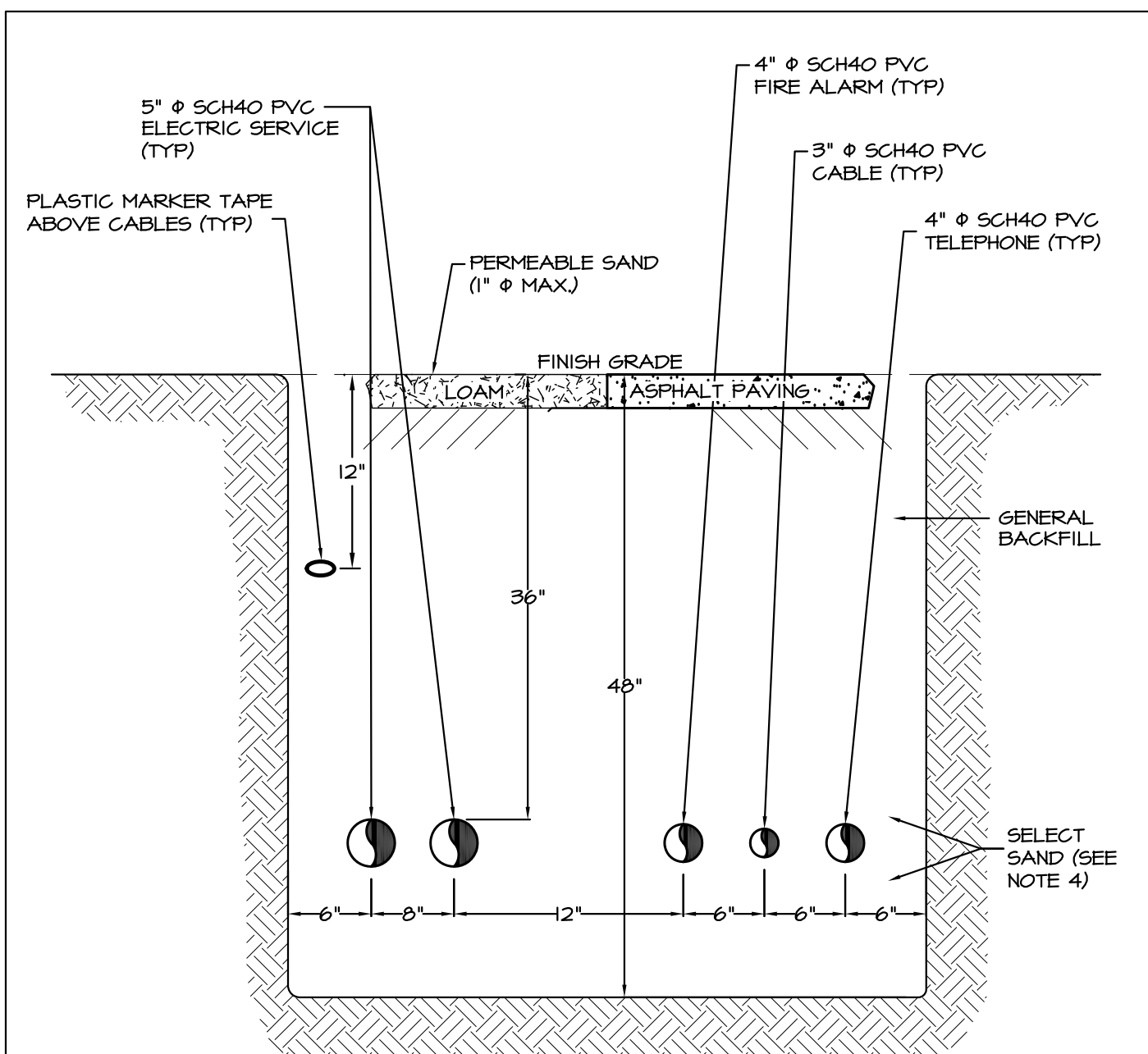


CIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

TITLE:
DRAINAGE DETAILS
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	AS SHOWN	D2



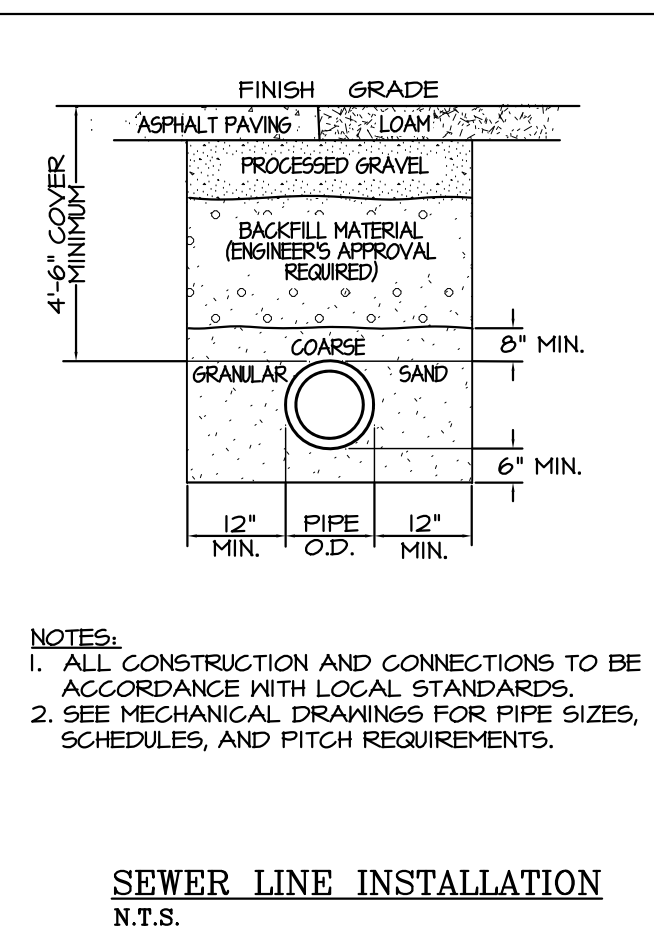


NOTES:

VERIFY NUMBER OF CONDUIT RUNS AND TYPES OF CONDUITS REQUIRED WITH ELECTRICAL AND MECHANICAL DESIGNERS BEFORE INSTALLATION

- ALL UTILITIES SHALL BE REVIEWED AND APPROVED BY APPROPRIATE UTILITY COMPANY.
- SERVICE BOX CONNECTIONS SHALL BE "FLUSH MOUNT" TO GREATEST EXTENT POSSIBLE AND LOCATED AT PROPERTY LINE CORNERS.
- PIPE SIZES ARE MINIMUM SIZES TO BE INSTALLED.
- BACKFILL SHALL BE SELECTED SAND, 100% SHALL PASS THROUGH 1/4" SCREEN, UP TO 1% MAY BE ROUNDED PEBBLES UP TO 3/8" IN SIZE.
- TRENCH WIDTH IS TO BE 12" MINIMUM, DEPENDING ON NUMBER OF UTILITIES IN TRENCH, UNLESS CABLE IS FLOWED IN.
- UTILITIES ARE TO BE LOCATED IN ROAD SHOULDERS AND ROWS AS DETERMINED BY PLANS. ALL WORK TO BE COORDINATED WITH UTILITY COMPANIES.
- THERE MAY BE MORE OR LESS SERVICES TO BE INSTALLED IN TRENCH VERIFY WITH UTILITIES PLAN.
- VERIFY & REFER TO PROJECT ELECTRICAL DRAWINGS AND DETAILS FOR SPECIFICS.

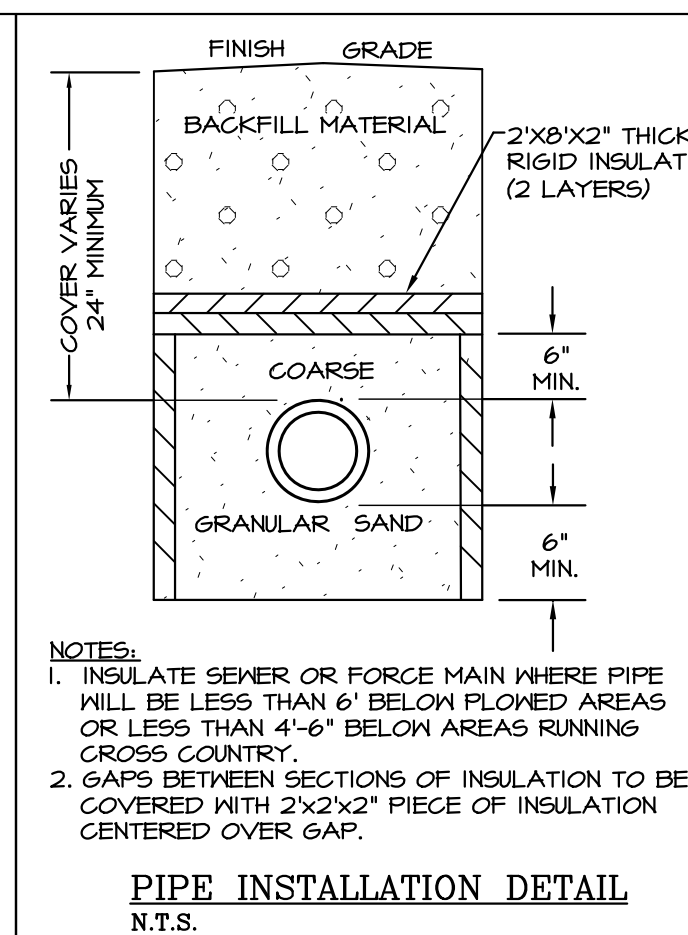
TYPICAL UTILITY TRENCH DETAIL
N.T.S.



NOTES:

- ALL CONSTRUCTION AND CONNECTIONS TO BE IN ACCORDANCE WITH LOCAL STANDARDS.
- SEE MECHANICAL DRAWINGS FOR PIPE SIZES, SCHEDULES, AND FITCH REQUIREMENTS.

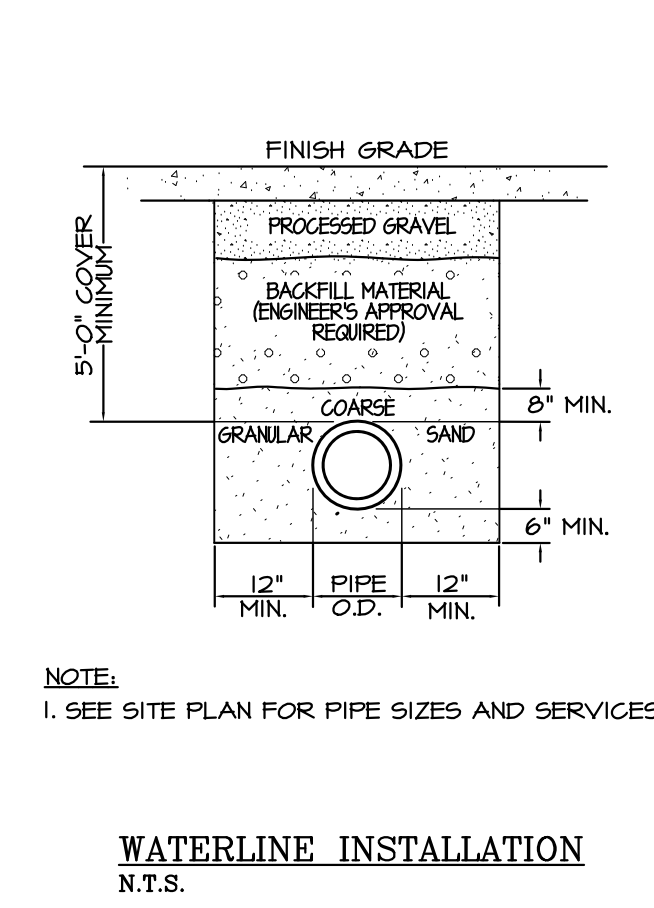
SEWER LINE INSTALLATION
N.T.S.



NOTES:

- INSULATE SEWER OR FORCE MAIN WHERE PIPE WILL BE LESS THAN 6' BELOW FLOWED AREAS OR LESS THAN 4'-6" BELOW AREAS RUNNING CROSS COUNTRY.
- GAPS BETWEEN SECTIONS OF INSULATION TO BE COVERED WITH 2"x2"x2" PIECE OF INSULATION CENTERED OVER GAP.

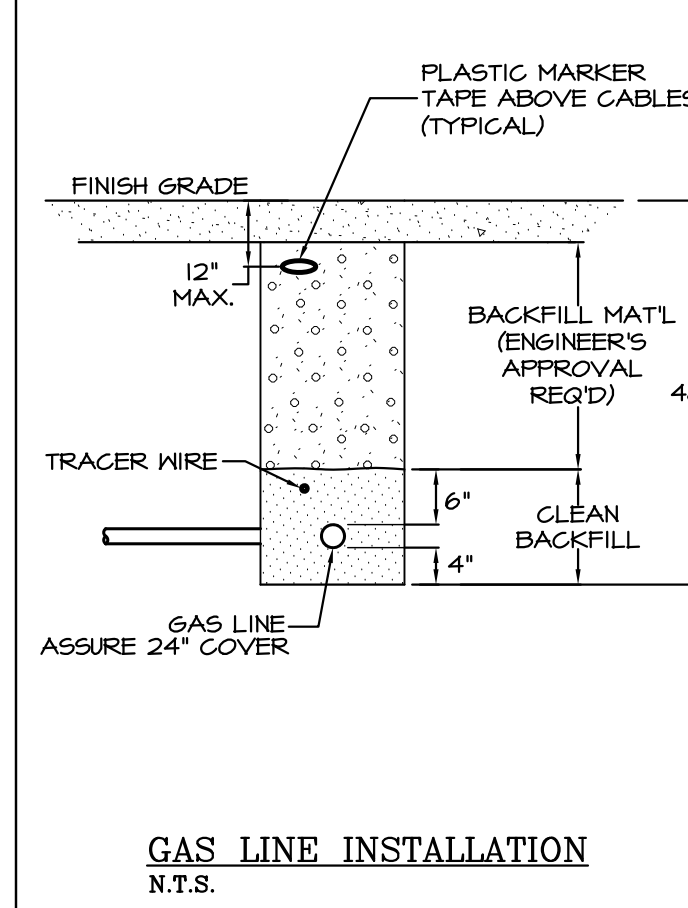
PIPE INSTALLATION DETAIL
N.T.S.



NOTE:

- SEE SITE PLAN FOR PIPE SIZES AND SERVICES.

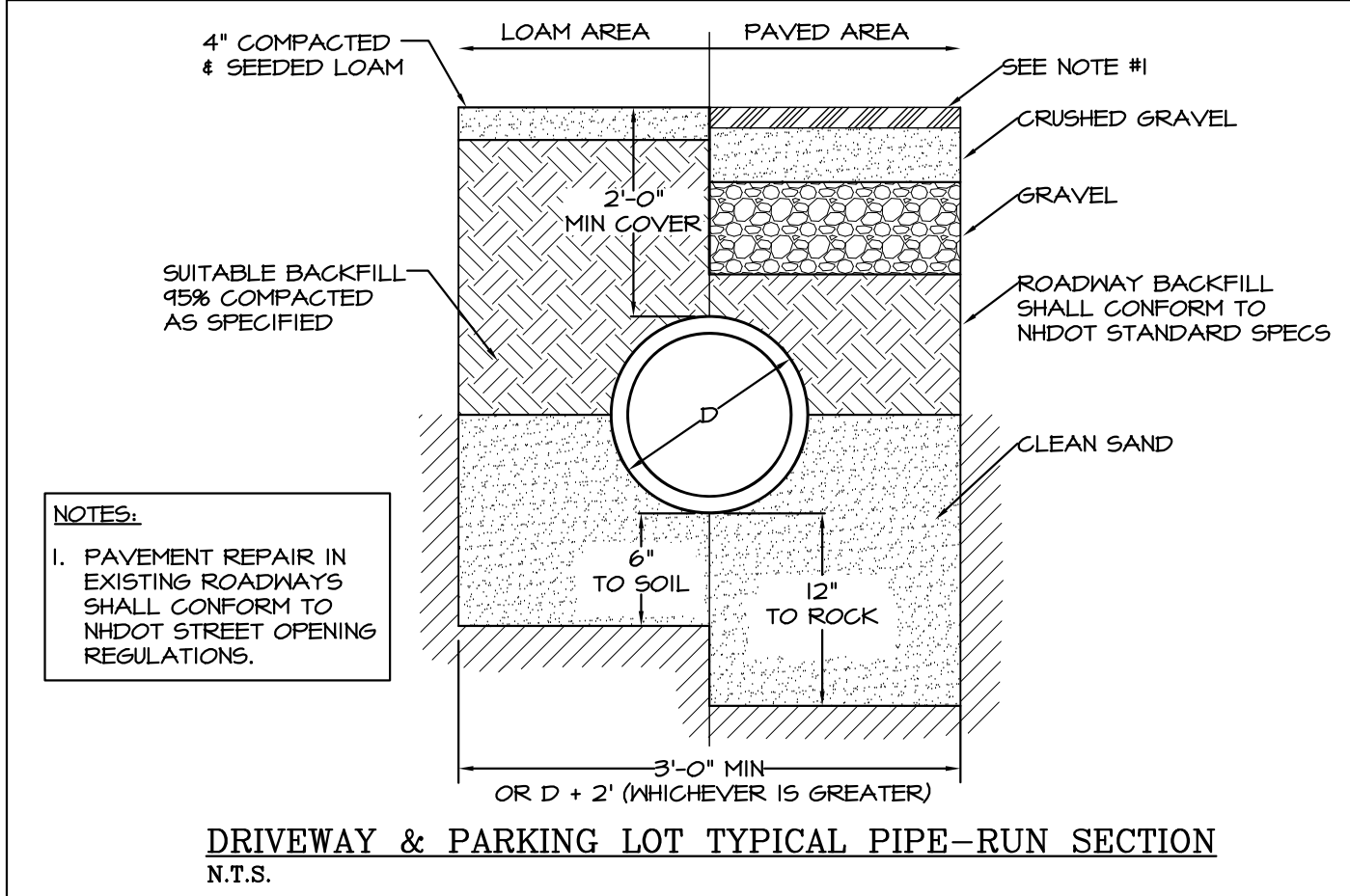
WATERLINE INSTALLATION
N.T.S.



NOTE:

- SEE SITE PLAN FOR PIPE SIZES AND SERVICES.

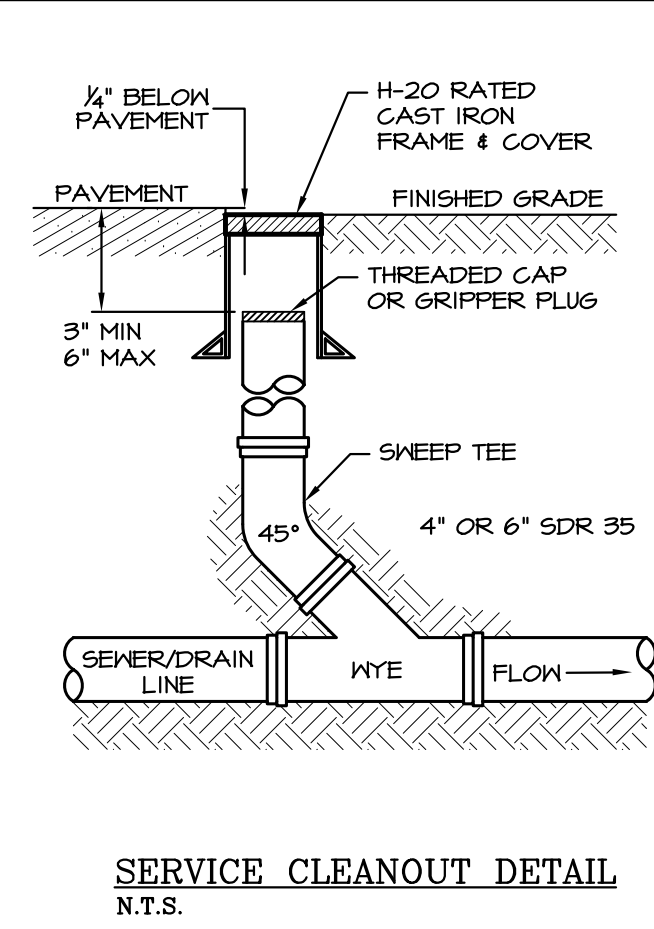
GAS LINE INSTALLATION
N.T.S.



NOTES:

- PAVEMENT REPAIR IN EXISTING ROADWAYS SHALL CONFORM TO NHDOT STREET OPENING REGULATIONS.

DRIVEWAY & PARKING LOT TYPICAL PIPE-RUN SECTION
N.T.S.



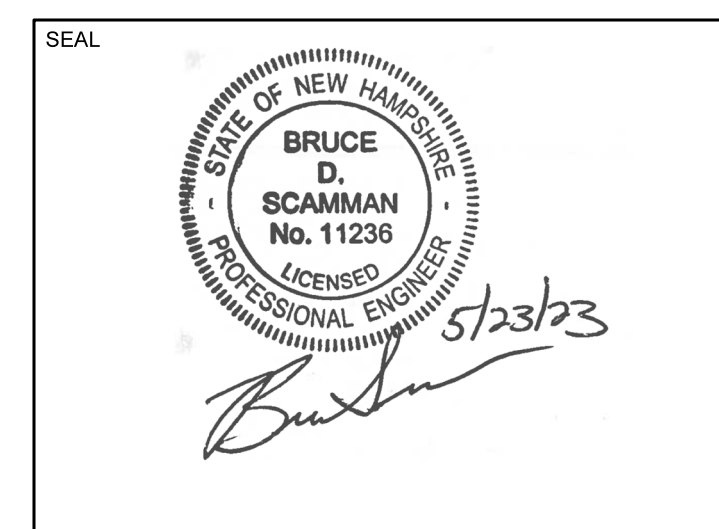
SERVICE CLEANOUT DETAIL
N.T.S.

3	MAY 23, 2023	FOR APPROVAL	
2	APR 20, 2023	FOR APPROVAL	
1	JAN 24, 2022	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK:
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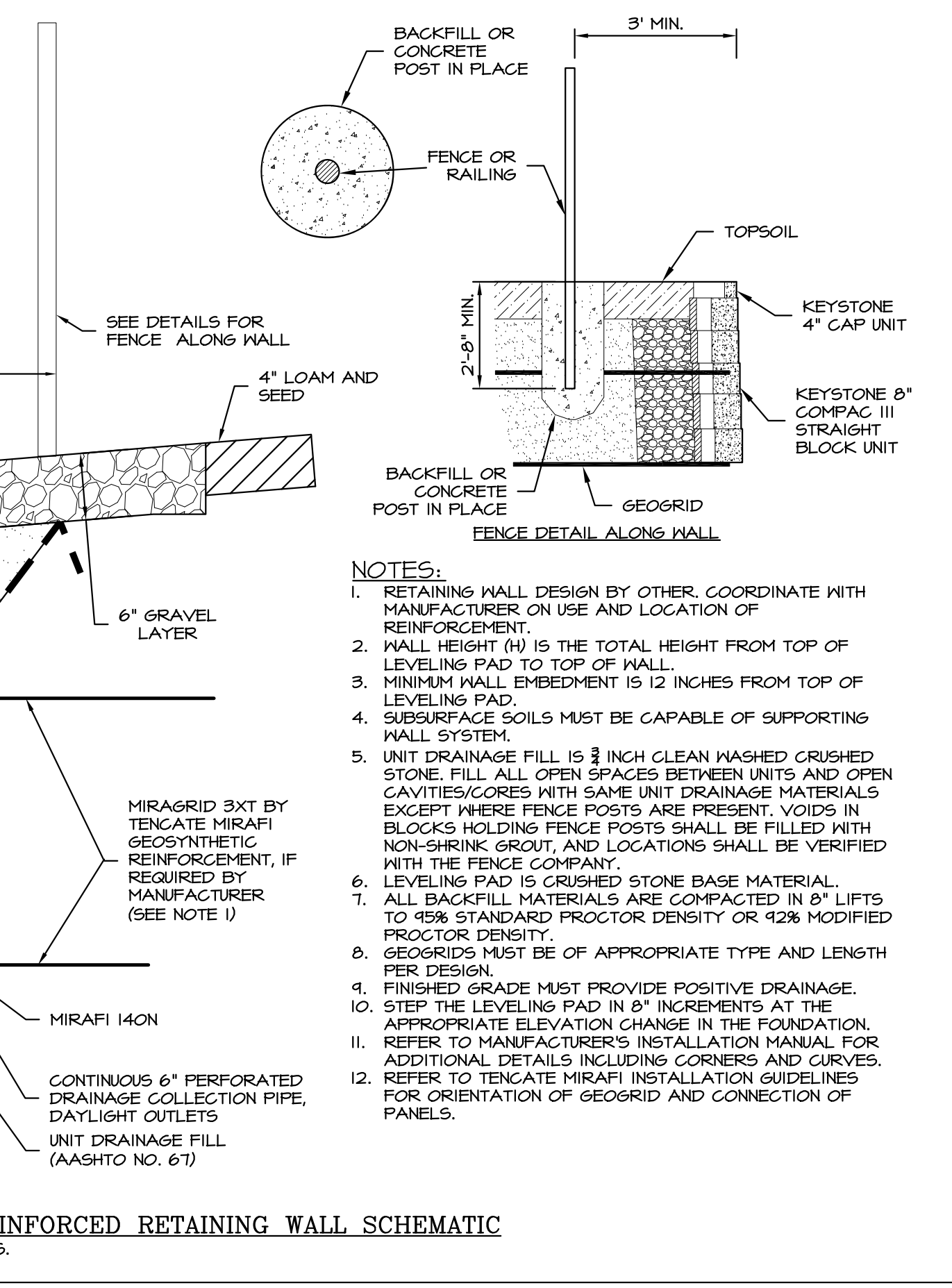
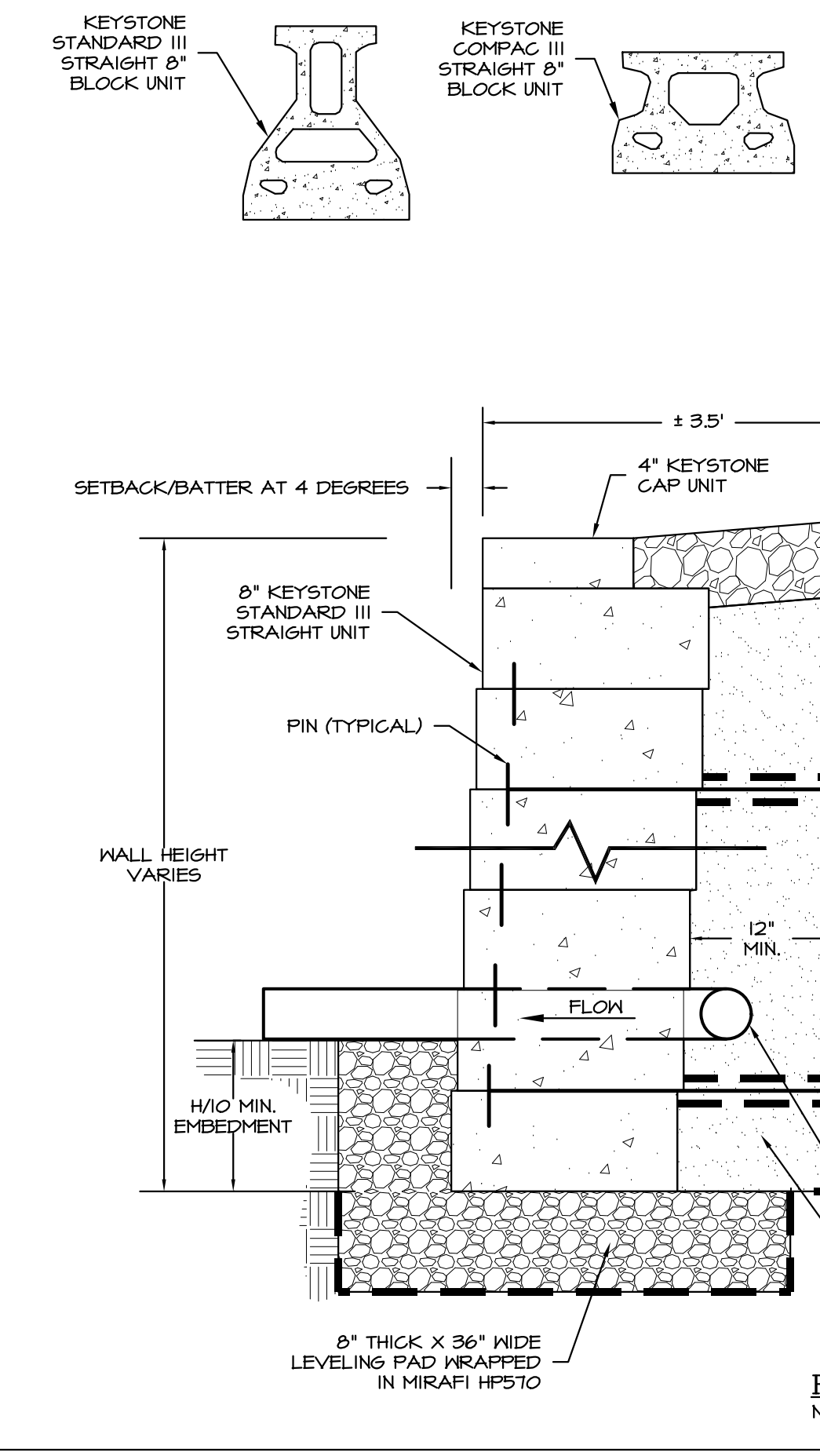
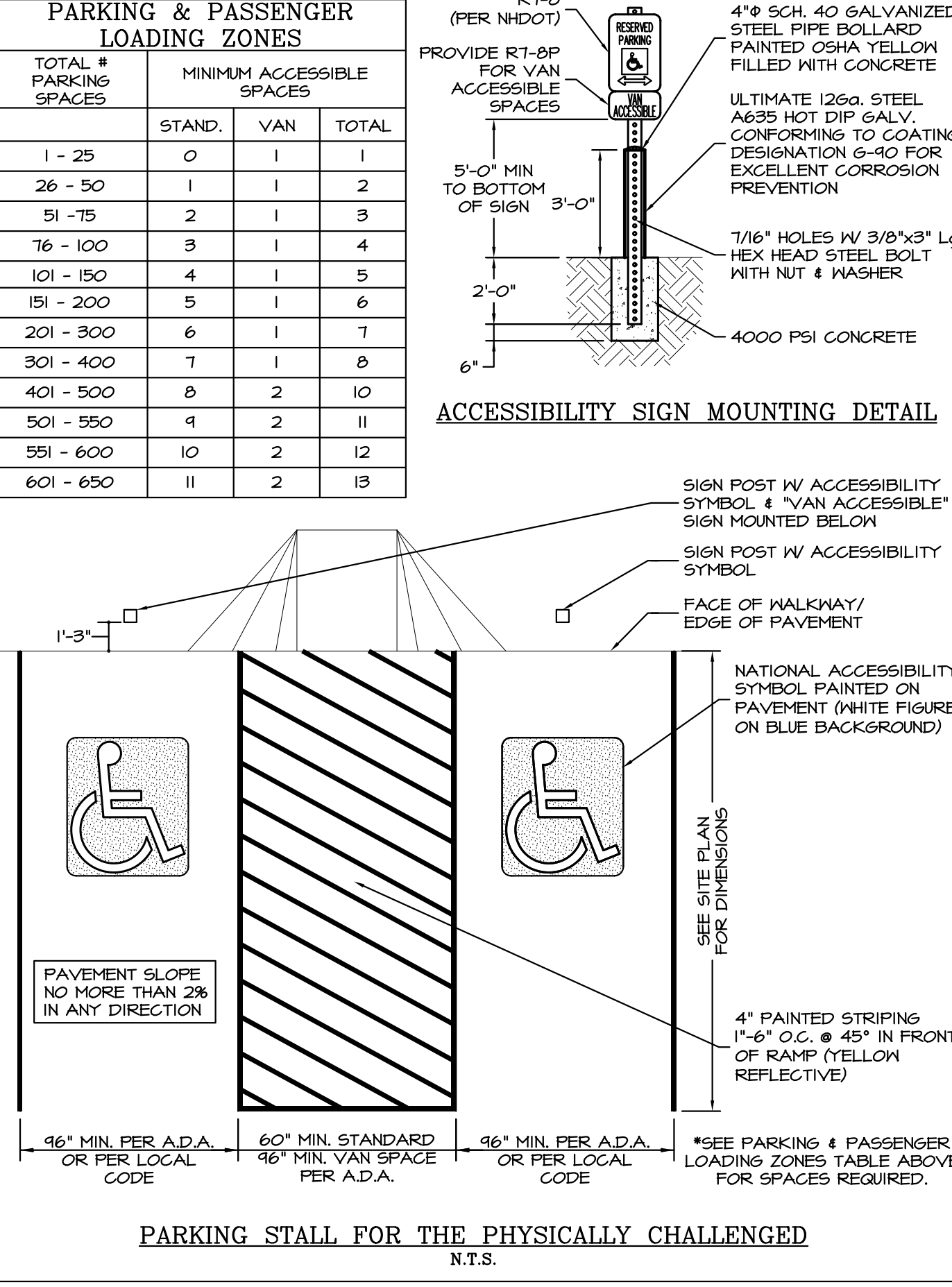
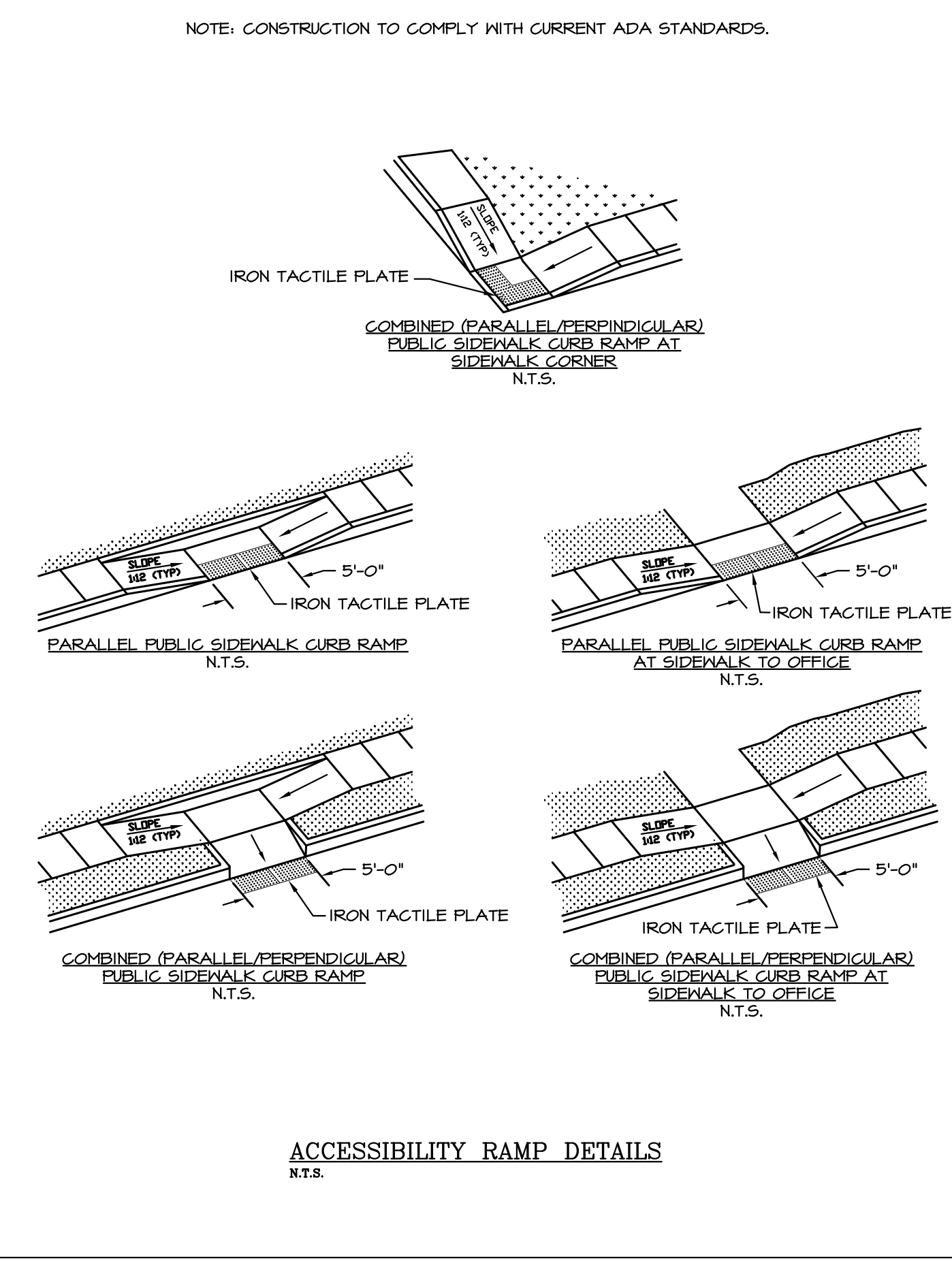
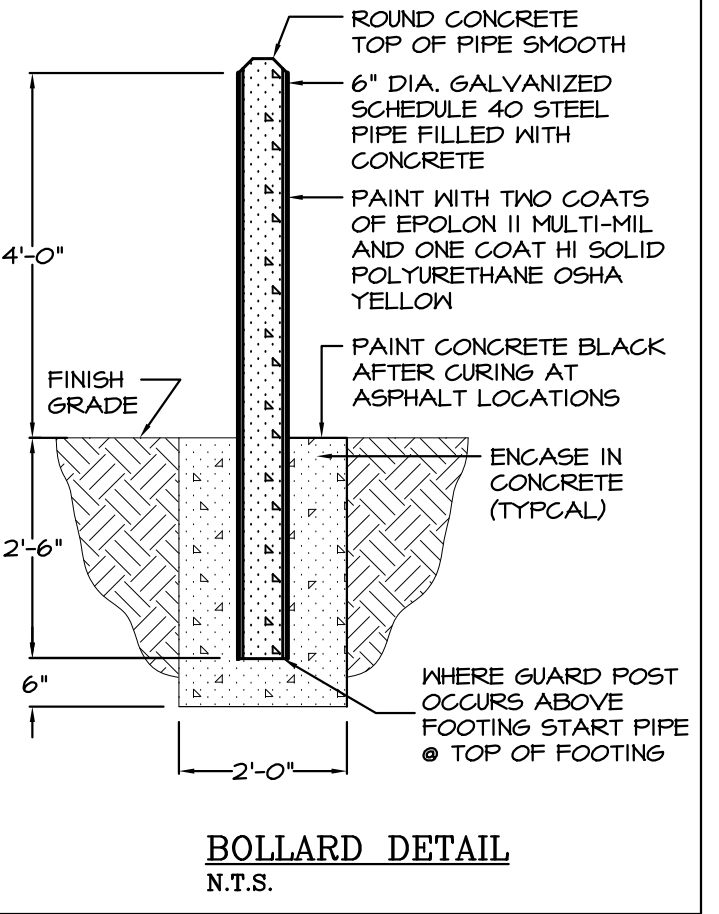
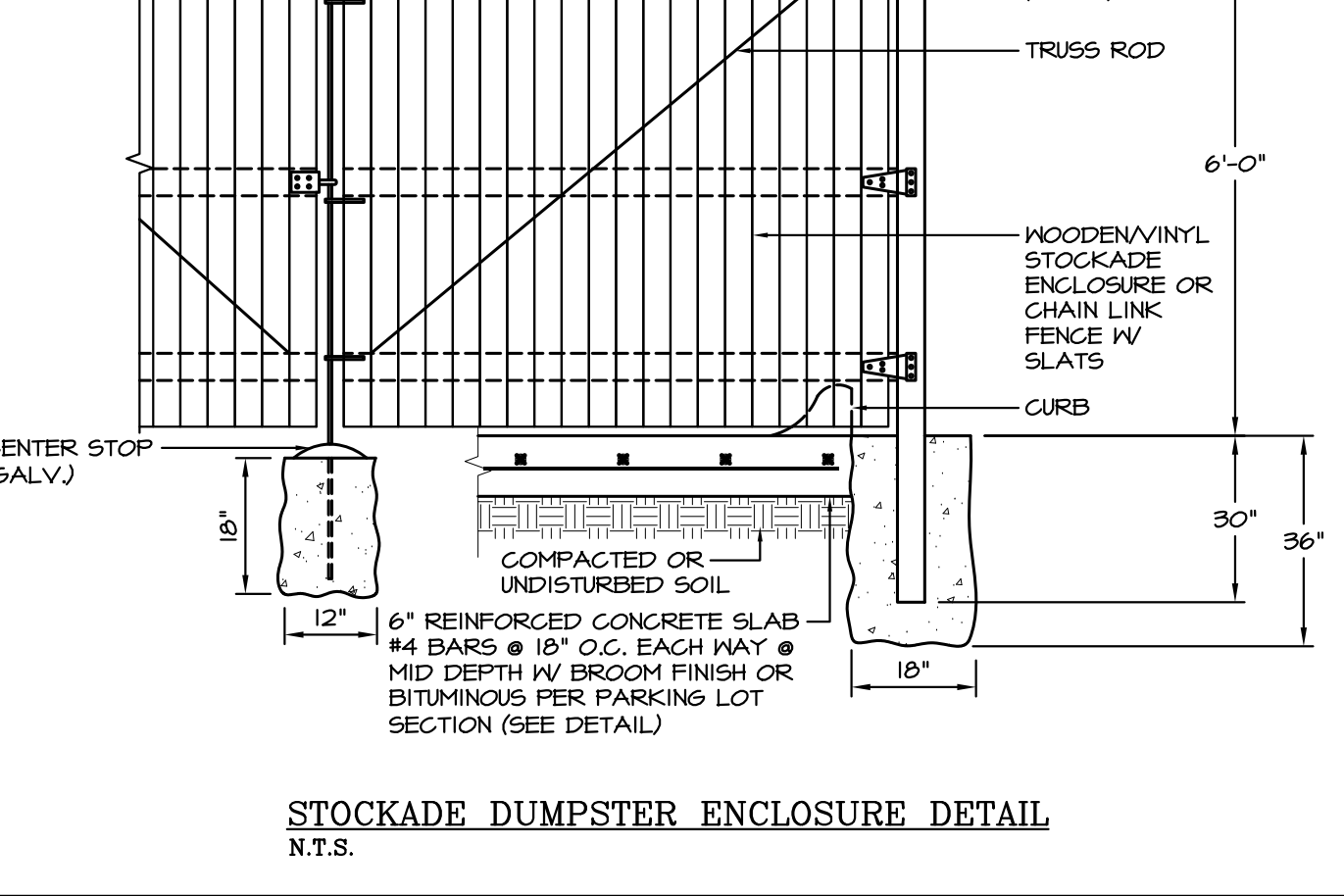
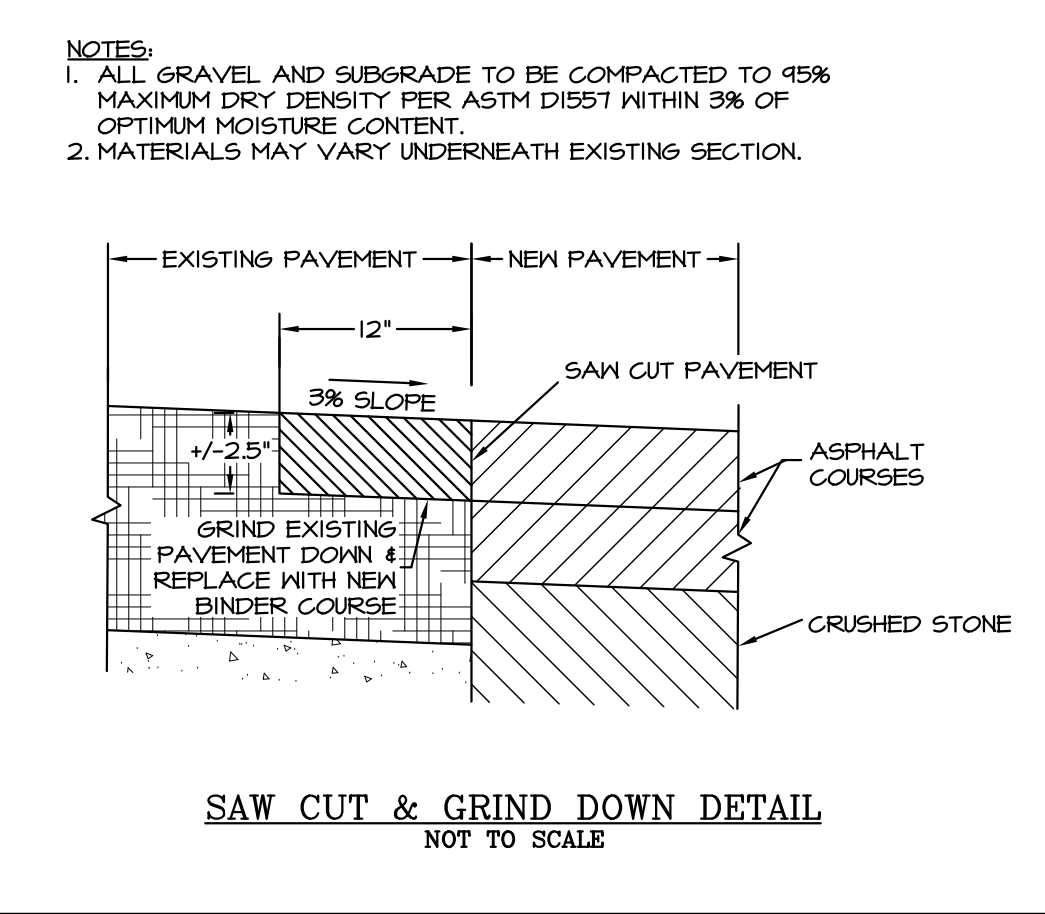
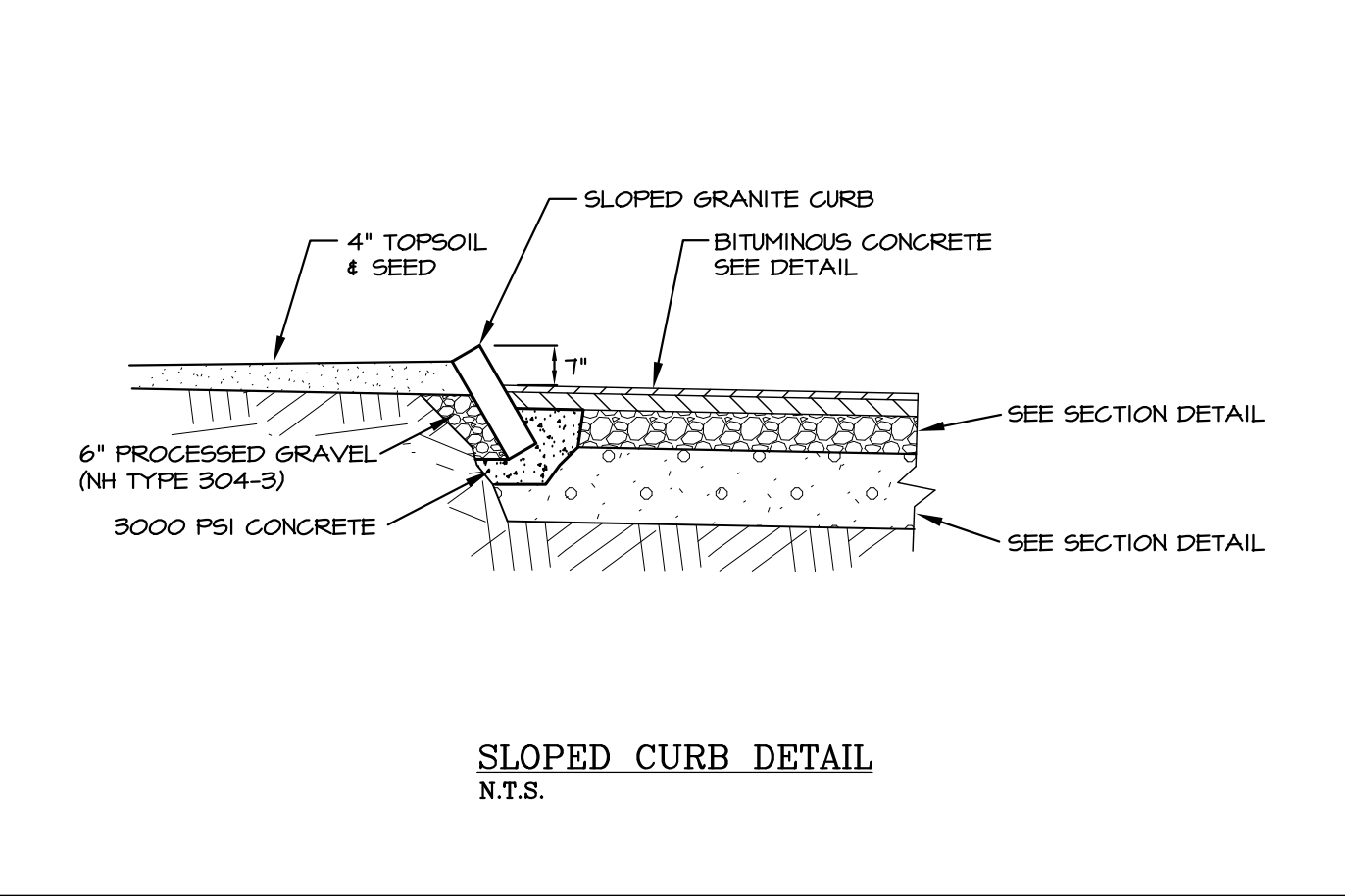
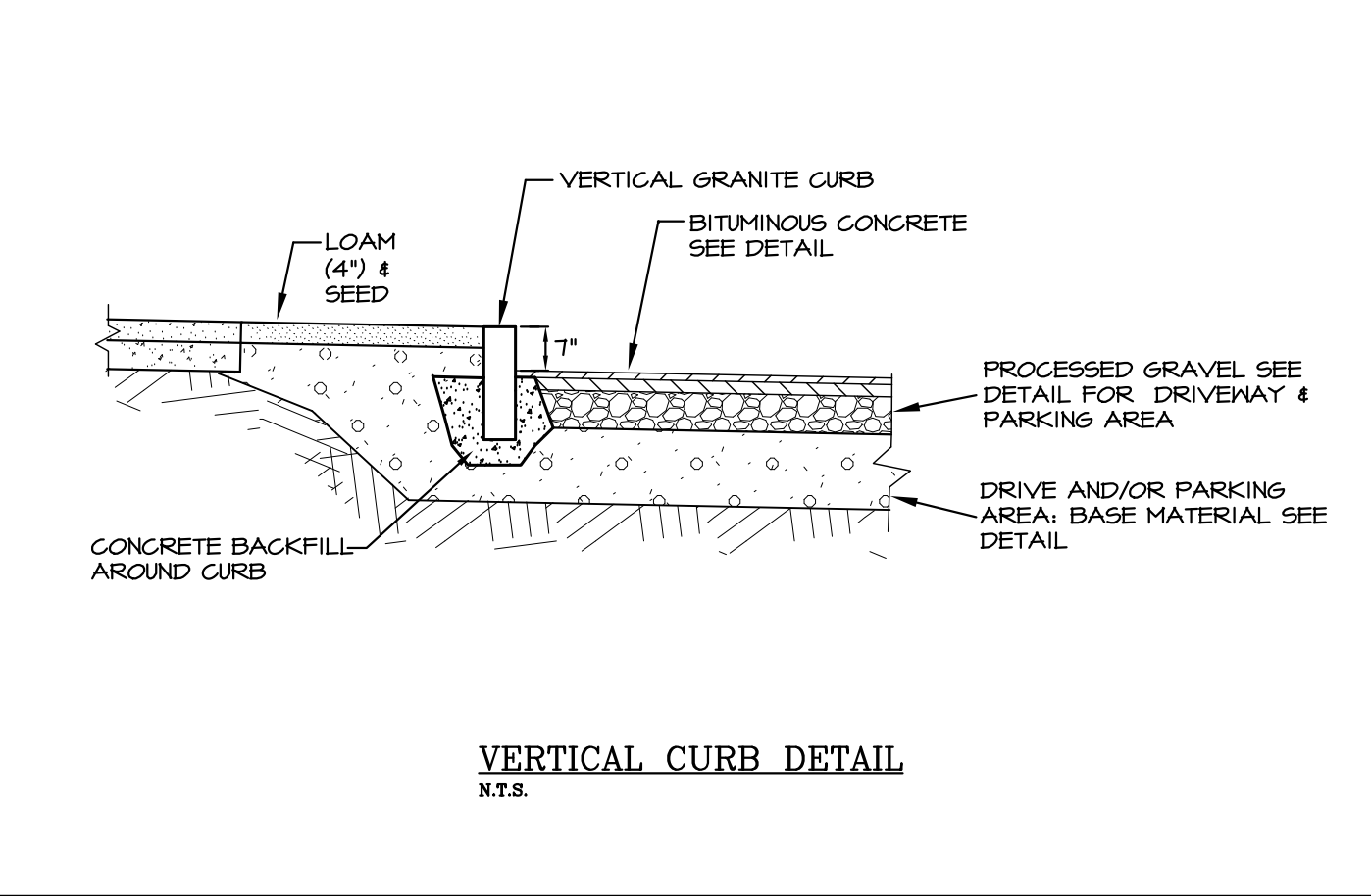
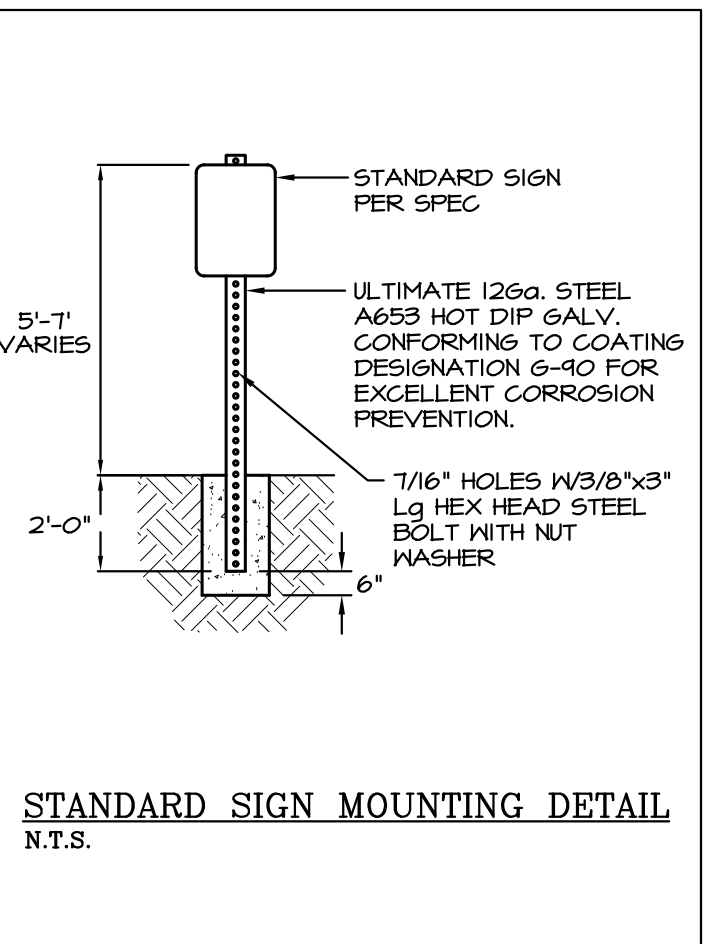
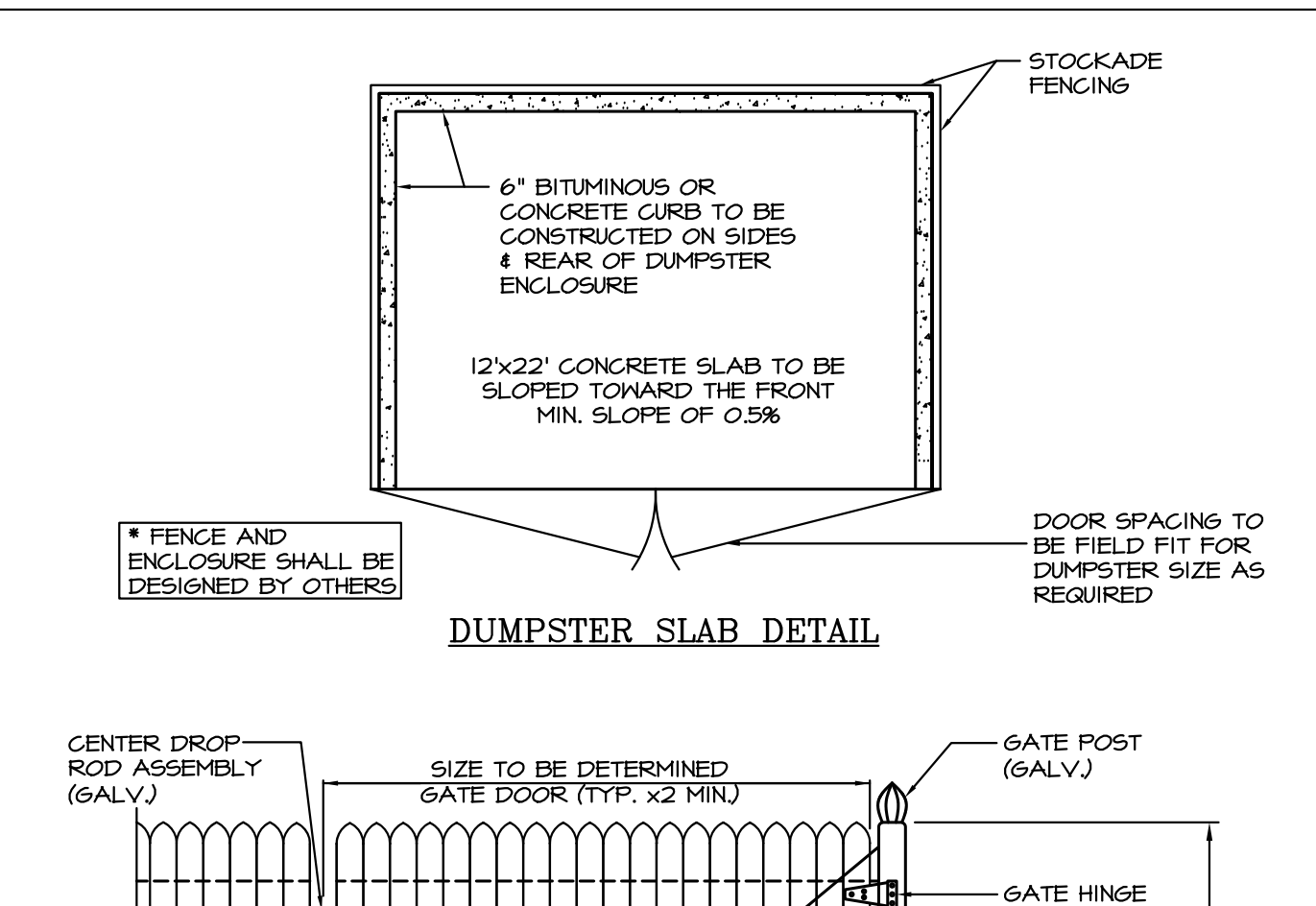
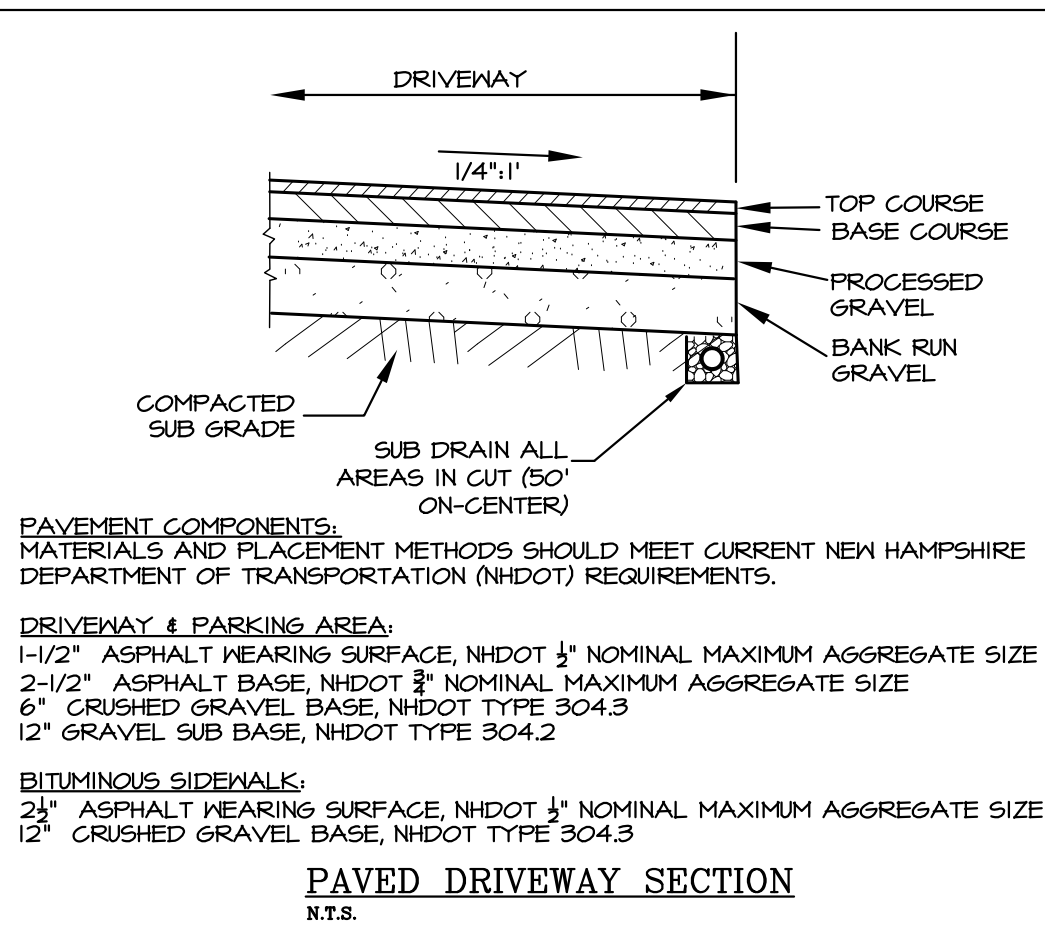
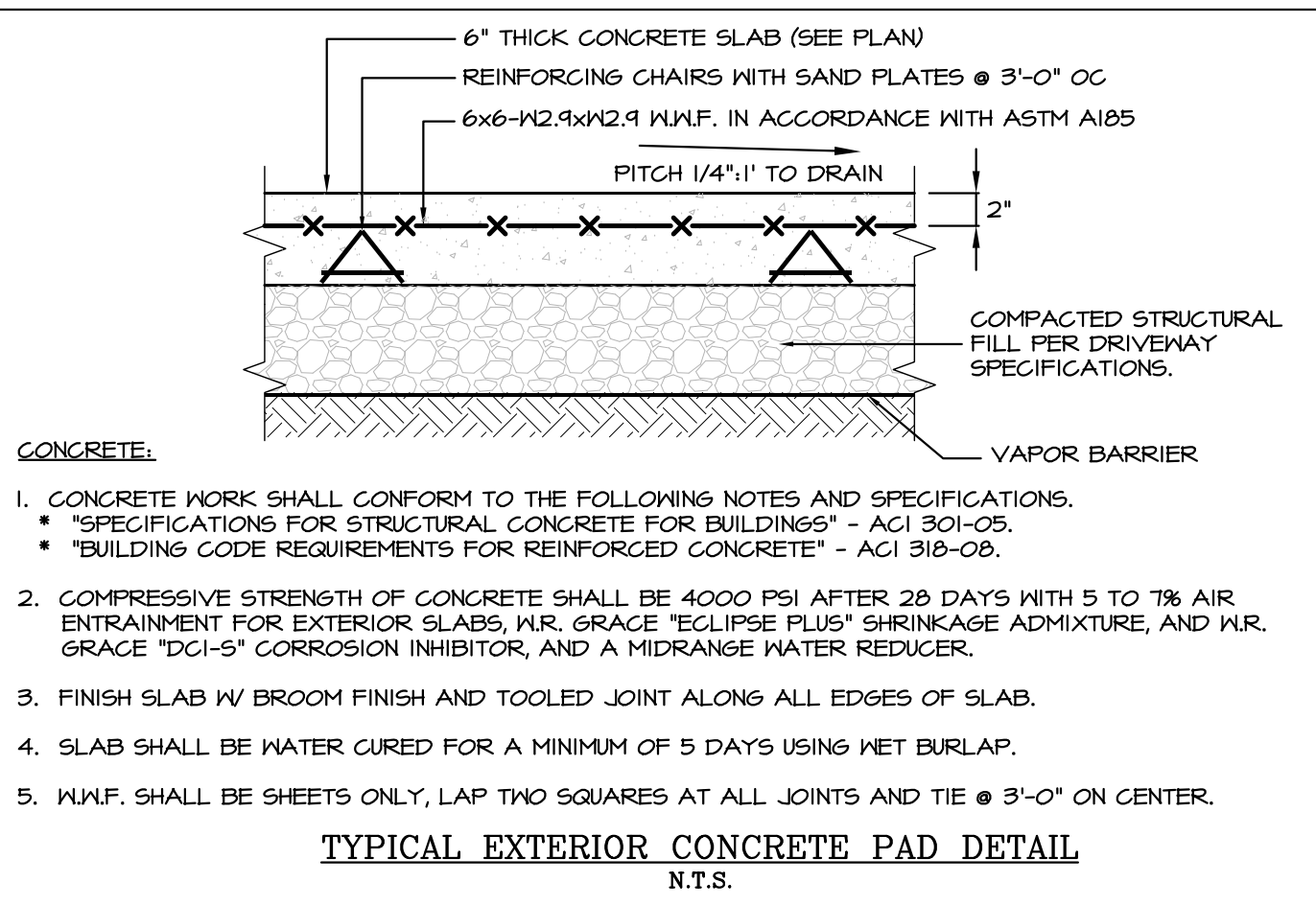
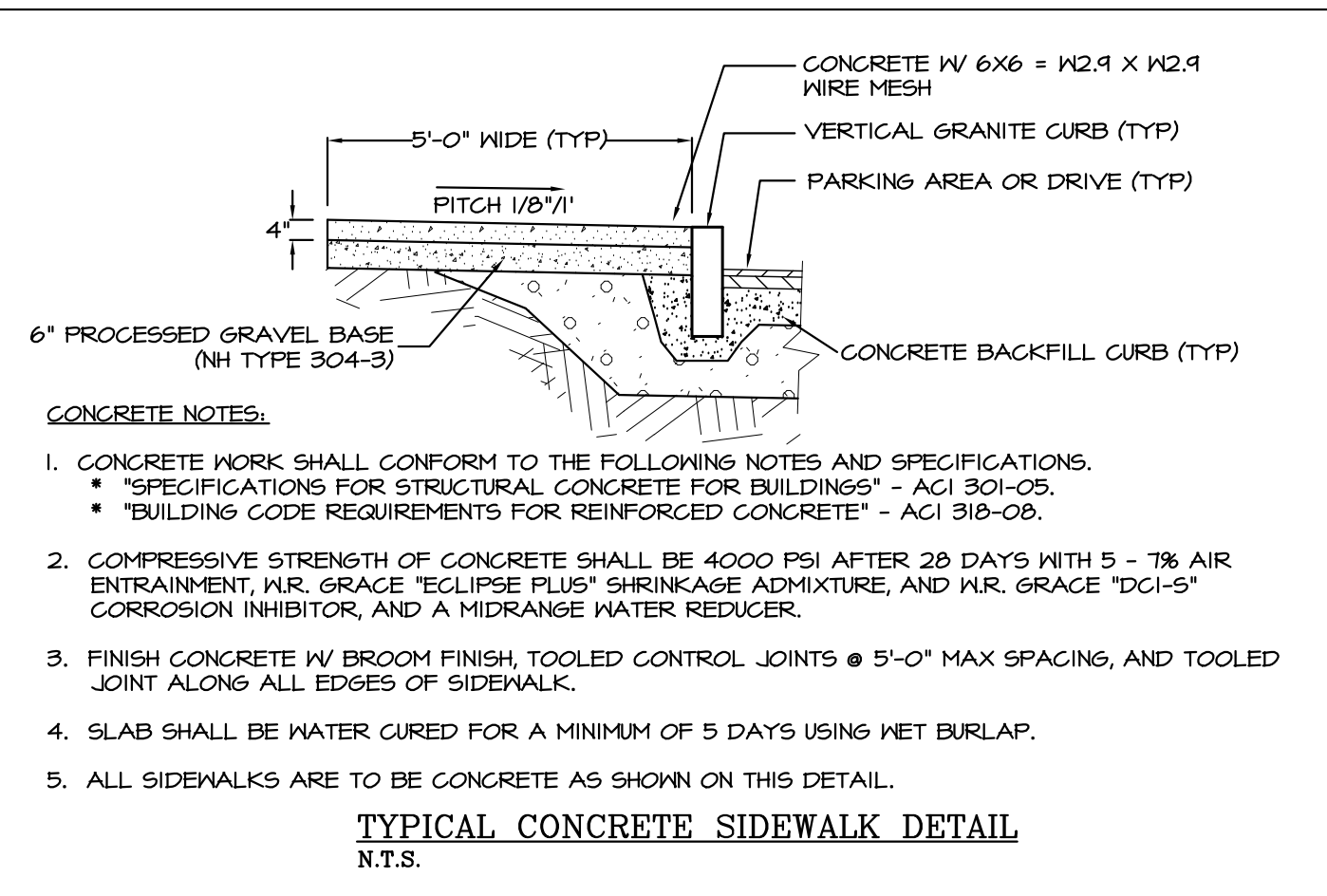
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833



TITLE:

UTILITY DETAILS
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	AS SHOWN	D3



SEAL

BRUCE D. SCAMMAN
No. 11236
LICENSED PROFESSIONAL ENGINEER

5/23/23

2	MAY 23, 2023	FOR APPROVAL	
1	MAY 20, 2023	FOR APPROVAL	

ISS. DATE: DESCRIPTION OF ISSUE: CHK.

DRAWN: JJM DESIGN: JJM

CHECKED: BDS CHECKED: BDS

EMANUEL ENGINEERING
civil & structural consultants, land planners
118 PORTSMOUTH AVENUE, A202
STRATHAM, NH 03885
P: 603-772-4400 F: 603-772-4487
WWW.EMANUELENGINEERING.COM

CLIENT:

BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

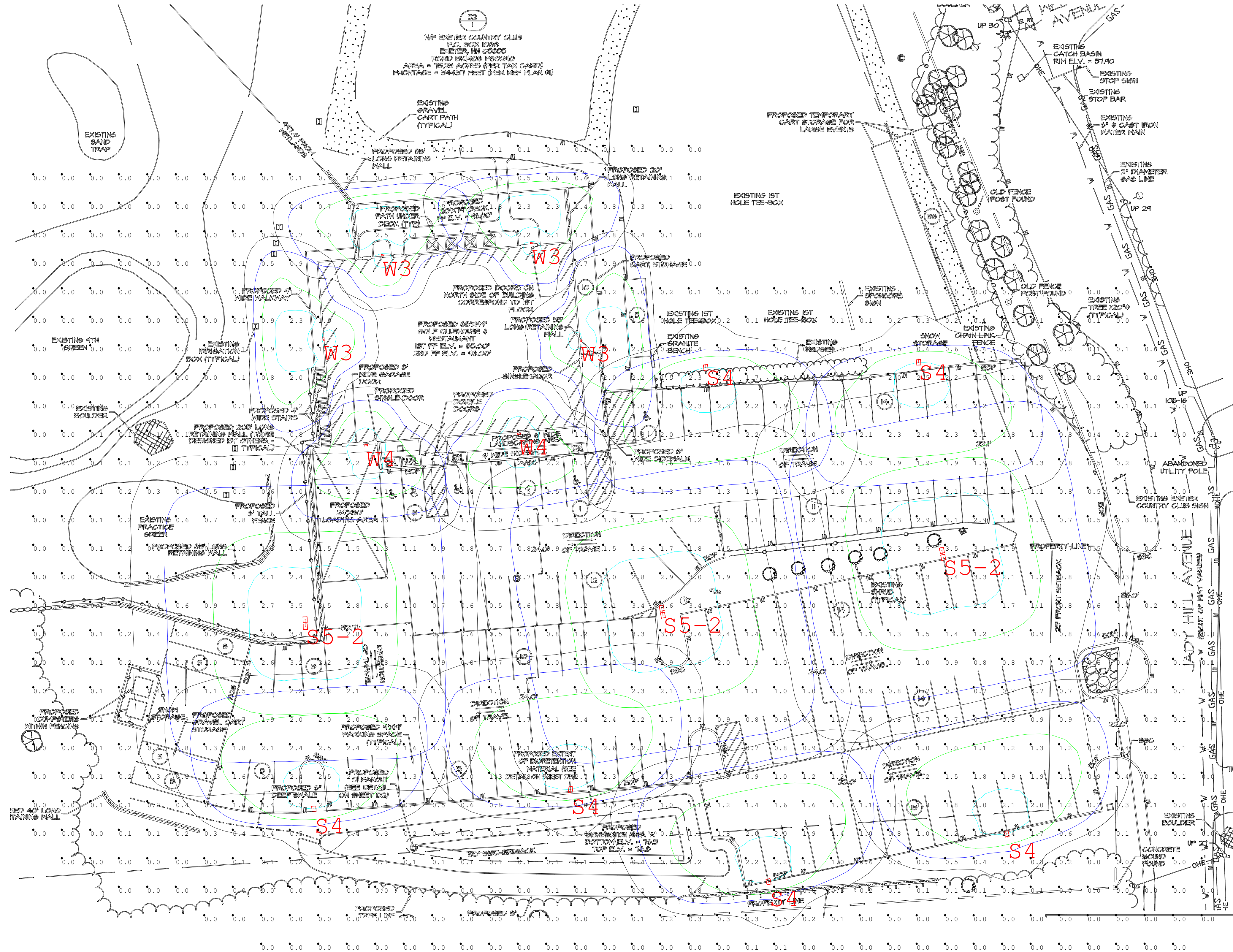
TITLE:

SITE DETAILS
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT: 21-157 SCALE: AS SHOWN SHEET: D4

StatArea_1
 PARKING LOT AREA
 Illuminance (Fc)
 Average = 1.54
 Maximum = 3.6
 Minimum = 0.5
 Avg/Min Ratio = 3.08
 Max/Min Ratio = 7.20

Luminaire Schedule					
Symbol	Qty	Label	Arrangement	Description	[MANUFAC]
	6	S4	Single	MRM-LED-07L-SIL-FT-UNV-DIM-30-70CRI-IH-CXX / 4SQ-B3-S11G-20-S-CXX-GA-4BC (20' AFG)	LSI INDUSTRIES, INC.
	3	S5-2	Back-Back	MRM-LED-07L-SIL-5W-UNV-DIM-30-70CRI-CXX / 4SQ-B3-S11G-20-D180-CXX-GA-4BC (20' AFG)	LSI INDUSTRIES, INC.
	4	W3	Single	XWS-LED-03L-SIL-3-UNV-DIM-30-70CRI-CXX / WALL MTD 12' AFG	LSI INDUSTRIES, INC.
	2	W4	Single	XWS-LED-03L-SIL-FT-UNV-DIM-30-70CRI-CXX / WALL MTD 12' AFG	LSI INDUSTRIES, INC.

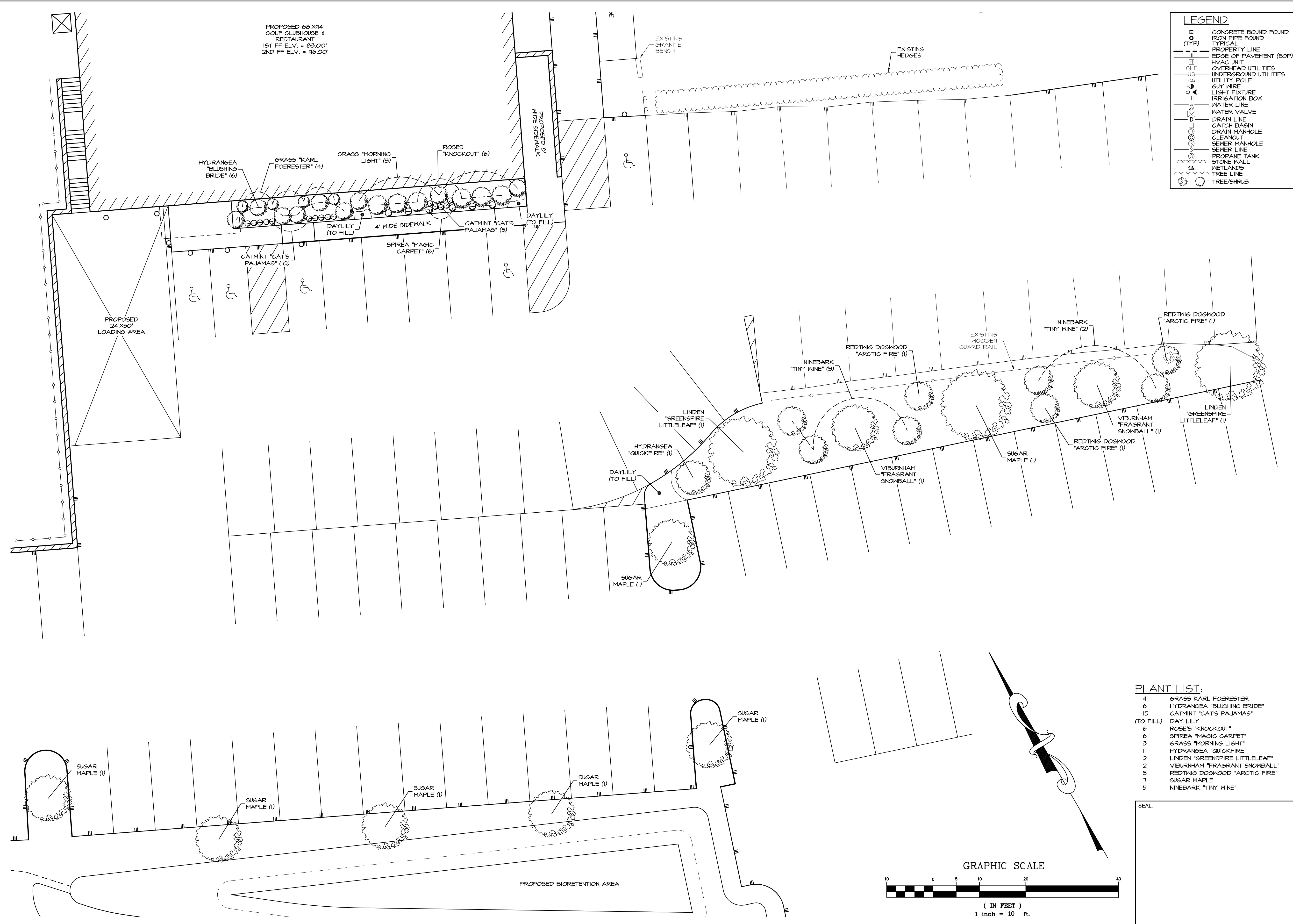


#	Date	Comments

#	Date	Comments

Drawn By:
 Checked By:
 Date: 4/13/2023
 Scale:

EXETER COUNTRY CLUB
58 JADY HILL AVE
EXETER NH 03833



PROPOSED 68'x144'
GOLF CLUBHOUSE &
RESTAURANT
1ST FF ELV. = 83.00'
2ND FF ELV. = 96.00'

LEGEND	
□	CONCRETE BOUND FOUND
○	IRON PIPE FOUND
○ (TYP)	TYPICAL
---	PROPERTY LINE
---	EDGE OF PAVEMENT (EOP)
---	H/VAC UNIT
---	OVERHEAD UTILITIES
---	UNDERGROUND UTILITIES
---	UTILITY POLE
---	GUY WIRE
---	LIGHT FIXTURE
---	IRRIGATION BOX
---	WATER LINE
---	WATER VALVE
---	DRAIN LINE
---	CATCH BASIN
---	DRAIN MANHOLE
---	CLEANOUT
---	SEWER MANHOLE
---	SEWER LINE
---	PROPANE TANK
---	STONE WALL
---	WETLANDS
---	TREE LINE
---	TREE/SHRUB

- NOTES:**
- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1038
EXETER, NH 03833
RCRD BK1406 P602940
 - THE INTENT OF THIS PLAN IS TO SHOW THE PROPOSED LANDSCAPING ON SITE ASSOCIATED WITH THE PROPOSED SITE IMPROVEMENTS.
 - PARCEL IS ZONED R-2 SINGLE FAMILY PER THE 2019 ZONING MAP OF EXETER, NEW HAMPSHIRE.
 - A PORTION OF THE PARCEL IS WITHIN IN FLOOD HAZARD ZONES "AE" (EL B) AND "X", REFERENCE FLOOD INSURANCE RATE MAPS 33015C0402E & 33015C0406E, DATED MAY 17, 2005.
 - FIELDWORK COMPLETED BY JAMES VERRA AND ASSOCIATES, INC. IN SPRING 2022. NH GRID; NAVD 1988.
 - WETLANDS DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. IN SPRING 2022. THE DELINEATION WAS LIMITED TO THE AREAS OF PROPOSED WORK DEPICTED ON THESE PLANS. THE LANDOWNER IS RESPONSIBLE FOR COMPLYING WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL WETLANDS REGULATIONS, INCLUDING ANY PERMITTING AND SETBACK REQUIREMENTS REQUIRED UNDER THESE REGULATIONS.
 - PROPERTY TO BE SERVICED TOWN WATER AND SEWER.
 - ALL CONSTRUCTION SHOULD COMPLY WITH FEDERAL, STATE, AND LOCAL STANDARDS AND REGULATIONS.
 - THIS PLAN WAS PREPARED WITH ON-SITE FIELD SURVEY AND EXISTING PLANS. THE CONTRACTOR SHOULD NOTIFY EMANUEL ENGINEERING, INC. DURING CONSTRUCTION IF ANY DISCREPANCY TO THE PLAN IS FOUND ON SITE.
 - BEFORE ANY EXCAVATION, DIG SAFE AND ALL UTILITY COMPANIES SHOULD BE CONTACTED 72 HOURS BEFORE COMMENCING BY THE CONTRACTOR. CALL DIG SAFE @ 811 OR 1-888-DIG-SAFE.
 - ALL UTILITIES SHALL BE LOCATED UNDERGROUND EXCEPT AS NOTED ON PLAN APPROVED BY THE PLANNING BOARD.
 - EMANUEL ENGINEERING, INC. (EEI) DID NOT DESIGN THE LANDSCAPING. ALL LANDSCAPING WAS DESIGNED BY THORN AND THISTLE GARDENS. THE LANDSCAPING DESIGN WAS PROVIDED TO EEI BY THE CLIENT.

- REFERENCE PLANS:**
- "PLAN OF LAND IN EXETER NH, EXETER COUNTRY CLUB" BY GREAT BAY ENGINEERING, INC., DATED DECEMBER 1988; SCALE: 1"=80'; RCRD D-18431.
 - "PLAN OF LAND IN EXETER, NH SHOWING SITE IMPROVEMENTS AT 58 JADY HILL AVE (EXETER COUNTRY CLUB)" BY MILLENNIUM ENGINEERING, INC., DATED JUNE 4, 2021; SCALE: 1"=40'; NOT RECORDED.
 - "TOWN OF EXETER, NEW HAMPSHIRE WEBSTER AVENUE PUMP STATION & FORCE MAIN UPGRADES EXETER, NEW HAMPSHIRE" (SHEET C-5) BY WRIGHT-PEIRCE (UNDATED); SCALE: 1"=20'; NOT RECORDED.

3	JUNE 16, 2023	FOR APPROVAL	
2	MAY 23, 2023	FOR APPROVAL	
1	APR 20, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN: JJM	DESIGN: JJM		
CHECKED: BDS	CHECKED: BDS		

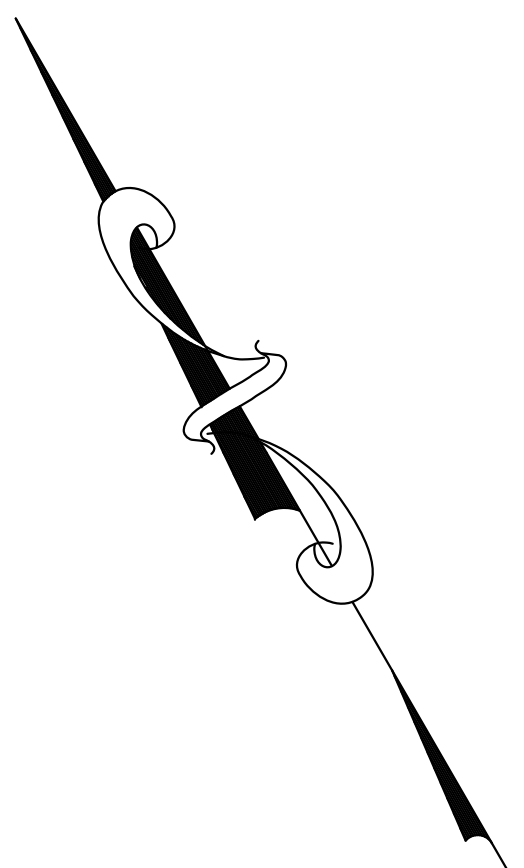
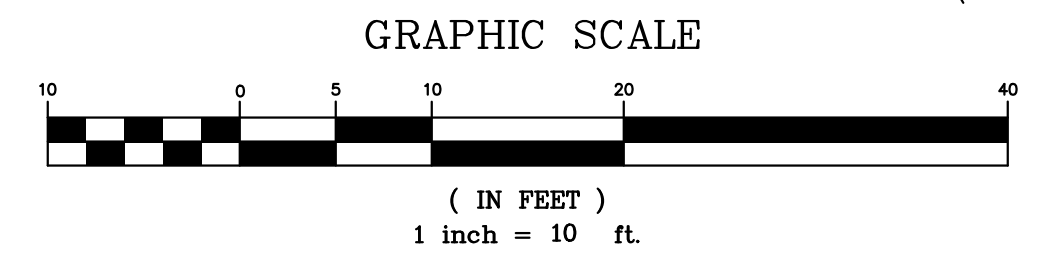


CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

TITLE:
LANDSCAPING PLAN
FOR
EXETER COUNTRY CLUB
58 JADY HILL AVENUE (SITE)
EXETER, NH 03833

PROJECT: 21-157 SCALE: 1"=10' SHEET: LA1

- PLANT LIST:**
- 4 GRASS KARL FOERESTER
 - 6 HYDRANGEA "BLUSHING BRIDE"
 - 15 CATMINT "CAT'S PAJAMAS"
 - (TO FILL) DAY LILY
 - 6 ROSE'S "KNOCKOUT"
 - 6 SPIREA "MAGIC CARPET"
 - 3 GRASS "MORNING LIGHT"
 - 1 HYDRANGEA "QUICKFIRE"
 - 2 LINDEN "GREENSPIRE LITTLELEAF"
 - 2 VIBURNHAM "FRAGRANT SNOWBALL"
 - 3 REDTWIG DOGWOOD "ARCTIC FIRE"
 - 7 SUGAR MAPLE
 - 5 NINEBARK "TINY WINE"



STORMWATER ANALYSIS AREA WORKSHEET

EMANUEL ENGINEERING INC.

JOB: 21-157 Blind Tiger - Exeter Country Club

DATE: 7/7/2023

ENGINEER: JJM

POST DEVELOPMENT DRAINAGE AREAS:

SOIL TYPE	SOIL		SUBCAT PS25A Area (SF)	SUBCAT PS25B Area (SF)	SUBCAT PS26 Area (SF)	SUBCAT PS27A Area (SF)	SUBCAT PS27B Area (SF)	SUBCAT PS27C Area (SF)	SUBCAT PS28 Area (SF)	SUBCAT PS29 Area (SF)	SUBCAT PS30 Area (SF)	SUBCAT PS31 Area (SF)	SUBCAT PS32 Area (SF)	TOTAL AREA (SF)
	GROUP	CN#												
Grass	B	61	1,430	400	32,900	13,160	65	2,060	475	1,455	6,820	8,395	8,840	76,000
Woods	B	55	0	0	0	1,365	0	0	0	0	22,470	74,625	0	98,460
Gravel	B	85	0	0	1,265	1,775	0	0	0	0	0	0	1,800	4,840
Building	B	98	4,960	3,255	0	0	0	0	0	0	0	0	0	8,215
Pavement	B	98	270	0	525	820	10,125	10,115	6,895	2,580	0	0	15,160	46,490
Total Area (SF)			6,660	3,655	34,690	17,120	10,190	12,175	7,370	4,035	29,290	83,020	25,800	234,005
Area (Acres)			0.15	0.08	0.80	0.39	0.23	0.28	0.17	0.09	0.67	1.91	0.59	5.37
Total Impervious (SF)			5,230	3,255	1,790	2,595	10,125	10,115	6,895	2,580	0	0	16,960	59,545
Impervious (Acres)			0.12	0.07	0.04	0.06	0.23	0.23	0.16	0.06	0.00	0.00	0.39	1.37

STORMWATER/DRAINAGE SUMMARY

EMANUEL ENGINEERING, INC.

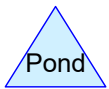
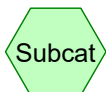
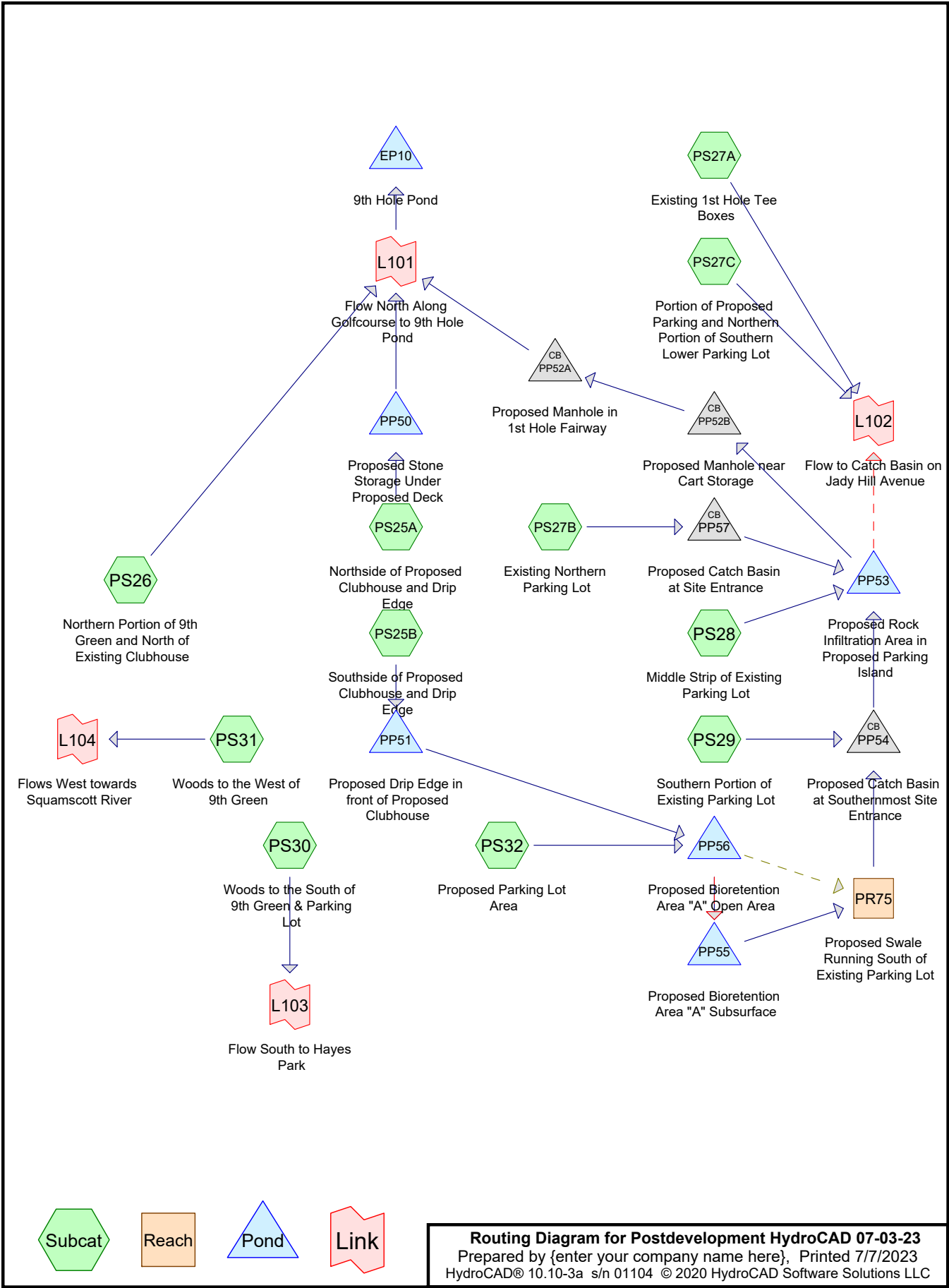
JOB: 21-157 Blind Tiger - Exeter Country Club
 DATE: 7/7/2023
 ENGINEER: JJM

PEAK FLOWS FROM HYDROCAD

Subcatchment Area	Storm Quality 1"		2-Year Storm 3.22"		10-Year Storm 4.91"		25-Year Storm 6.24"		50-Year Storm 7.49"	
	Pre (CFS)	Post (CFS)	Pre (CFS)	Post (CFS)	Pre (CFS)	Post (CFS)	Pre (CFS)	Post (CFS)	Pre (CFS)	Post (CFS)
POINTS OF DISCHARGE										
LINK L101	0.00	0.34	0.33	1.65	1.09	2.96	1.77	3.98	2.41	5.35
LINK L102	0.13	0.13	3.13	0.97	5.18	1.69	6.74	2.28	8.11	2.81
LINK L103	0.00	0.00	0.07	0.05	0.73	0.53	1.39	1.01	2.04	1.48
LINK L104	0.00	0.00	0.14	0.14	1.32	1.32	2.52	2.52	3.73	3.73
FLOW TOTALS (CFS)	0.13	0.47	3.67	2.81	8.32	6.50	12.42	9.79	16.29	13.37
Net Increase/(Decrease) (CFS)		0.34		(0.86)		(1.82)		(2.63)		(2.92)

VOLUMES FROM HYDROCAD

Subcatchment Area	Storm Quality 1"		2-Year Storm 3.22"		10-Year Storm 4.91"		25-Year Storm 6.24"		50-Year Storm 7.49"	
	Pre (AF)	Post (AF)	Pre (AF)	Post (AF)	Pre (AF)	Post (AF)	Pre (AF)	Post (AF)	Pre (AF)	Post (AF)
POINTS OF DISCHARGE										
LINK L101	0.000	0.024	0.040	0.213	0.109	0.422	0.177	0.601	0.247	0.778
LINK L102	0.018	0.009	0.222	0.075	0.422	0.145	0.589	0.206	0.749	0.266
LINK L103	0.000	0.000	0.022	0.016	0.077	0.056	0.134	0.098	0.196	0.142
LINK L104	0.000	0.000	0.045	0.045	0.158	0.158	0.276	0.276	0.404	0.404
VOLUME TOTALS (CFS)	0.018	0.033	0.329	0.349	0.766	0.781	1.176	1.181	1.596	1.590
Net Increase/(Decrease) (CFS)		0.015		0.020		0.015		0.005		(0.006)



Routing Diagram for Postdevelopment HydroCAD 07-03-23
 Prepared by {enter your company name here}, Printed 7/7/2023
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Postdevelopment HydroCAD 07-03-23

Prepared by {enter your company name here}

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Page 2

Project Notes

Copied 9 events from NH-Exeter 08-23-22 24-hr S1 storm

Postdevelopment HydroCAD 07-03-23

Prepared by {enter your company name here}

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Page 3

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-inch	NH-Exeter 08-23-22 24-hr S1	1-yr	Default	24.00	1	1.00	2
2	1-yr	NH-Exeter 08-23-22 24-hr S1	1-yr	Default	24.00	1	2.68	2
3	2-yr	NH-Exeter 08-23-22 24-hr S1	2-yr	Default	24.00	1	3.22	2
4	5-yr	NH-Exeter 08-23-22 24-hr S1	5-yr	Default	24.00	1	4.09	2
5	10-yr	NH-Exeter 08-23-22 24-hr S1	10-yr	Default	24.00	1	4.91	2
6	25-yr	NH-Exeter 08-23-22 24-hr S1	25-yr	Default	24.00	1	6.24	2
7	50-yr	NH-Exeter 08-23-22 24-hr S1	50-yr	Default	24.00	1	7.49	2
8	100-yr	NH-Exeter 08-23-22 24-hr S1	100-yr	Default	24.00	1	9.00	2
9	200-yr	NH-Exeter 08-23-22 24-hr S1	200-yr	Default	24.00	1	10.81	2
10	500-yr	NH-Exeter 08-23-22 24-hr S1	500-yr	Default	24.00	1	13.77	2

Postdevelopment HydroCAD 07-03-23

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Page 4

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.745	61	>75% Grass cover, Good, HSG B (PS25A, PS25B, PS26, PS27A, PS27B, PS27C, PS28, PS29, PS30, PS31, PS32)
0.111	85	Gravel roads, HSG B (PS26, PS27A, PS32)
1.067	98	Paved parking, HSG B (PS25A, PS26, PS27A, PS27B, PS27C, PS28, PS29, PS32)
0.189	98	Roofs, HSG B (PS25A, PS25B)
2.260	55	Woods, Good, HSG B (PS27A, PS30, PS31)
5.372	68	TOTAL AREA

Postdevelopment HydroCAD 07-03-23

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Page 5

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
5.372	HSG B	PS25A, PS25B, PS26, PS27A, PS27B, PS27C, PS28, PS29, PS30, PS31, PS32
0.000	HSG C	
0.000	HSG D	
0.000	Other	
5.372		TOTAL AREA

Postdevelopment HydroCAD 07-03-23

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	1.745	0.000	0.000	0.000	1.745	>75% Grass cover, Good	PS25A, PS25B, PS26, PS27A, PS27B, PS27C, PS28, PS29, PS30, PS31, PS32
0.000	0.111	0.000	0.000	0.000	0.111	Gravel roads	PS26, PS27A, PS32
0.000	1.067	0.000	0.000	0.000	1.067	Paved parking	PS25A, PS26, PS27A, PS27B, PS27C, PS28, PS29, PS32
0.000	0.189	0.000	0.000	0.000	0.189	Roofs	PS25A, PS25B
0.000	2.260	0.000	0.000	0.000	2.260	Woods, Good	PS27A, PS30, PS31
0.000	5.372	0.000	0.000	0.000	5.372	TOTAL AREA	

Postdevelopment HydroCAD 07-03-23

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Page 7

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	PP51	80.00	77.00	145.0	0.0207	0.010	8.0	0.0	0.0
2	PP52A	58.40	46.10	265.0	0.0464	0.010	12.0	0.0	0.0
3	PP52B	60.25	58.50	420.0	0.0042	0.012	12.0	0.0	0.0
4	PP53	61.00	60.35	245.0	0.0027	0.012	12.0	0.0	0.0
5	PP54	63.00	61.10	50.0	0.0380	0.010	12.0	0.0	0.0
6	PP55	74.05	73.90	75.0	0.0020	0.010	8.0	0.0	0.0
7	PP57	61.30	61.10	40.0	0.0050	0.012	12.0	0.0	0.0

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of	Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=3.79" Tc=5.0 min CN=90 Runoff=0.58 cfs 0.048 af
Subcatchment PS25B: Southside of	Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=4.22" Tc=5.0 min CN=94 Runoff=0.34 cfs 0.030 af
Subcatchment PS26: Northern Portion of	Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=1.38" Flow Length=235' Tc=7.1 min CN=62 Runoff=0.90 cfs 0.092 af
Subcatchment PS27A: Existing 1st Hole Tee	Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=1.59" Tc=5.0 min CN=65 Runoff=0.60 cfs 0.052 af
Subcatchment PS27B: Existing Northern	Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=4.67" Tc=5.0 min CN=98 Runoff=0.99 cfs 0.091 af
Subcatchment PS27C: Portion of	Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=4.00" Tc=5.0 min CN=92 Runoff=1.10 cfs 0.093 af
Subcatchment PS28: Middle Strip of	Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=4.44" Tc=5.0 min CN=96 Runoff=0.70 cfs 0.063 af
Subcatchment PS29: Southern Portion of	Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=3.28" Tc=5.0 min CN=85 Runoff=0.31 cfs 0.025 af
Subcatchment PS30: Woods to the South of	Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=1.00" Flow Length=233' Tc=5.0 min CN=56 Runoff=0.53 cfs 0.056 af
Subcatchment PS31: Woods to the West of	Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=1.00" Flow Length=345' Tc=7.4 min CN=56 Runoff=1.32 cfs 0.158 af
Subcatchment PS32: Proposed Parking	Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=3.19" Tc=5.0 min CN=84 Runoff=1.93 cfs 0.157 af
Reach PR75: Proposed Swale Running	Avg. Flow Depth=0.14' Max Vel=3.40 fps Inflow=0.20 cfs 0.152 af n=0.022 L=90.0' S=0.0944 '/' Capacity=37.88 cfs Outflow=0.20 cfs 0.152 af
Pond EP10: 9th Hole Pond	Peak Elev=38.79' Storage=18,366 cf Inflow=2.96 cfs 0.422 af Outflow=0.00 cfs 0.000 af
Pond PP50: Proposed Stone Storage Under	Peak Elev=78.30' Storage=1,119 cf Inflow=0.58 cfs 0.048 af Discarded=0.02 cfs 0.048 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.048 af
Pond PP51: Proposed Drip Edge in front of	Peak Elev=80.32' Storage=51 cf Inflow=0.34 cfs 0.030 af Discarded=0.00 cfs 0.005 af Primary=0.31 cfs 0.024 af Outflow=0.32 cfs 0.030 af
Pond PP52A: Proposed Manhole in 1st Hole Fairway	Peak Elev=59.21' Inflow=2.10 cfs 0.330 af 12.0" Round Culvert n=0.010 L=265.0' S=0.0464 '/' Outflow=2.10 cfs 0.330 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=61.20' Inflow=2.10 cfs 0.330 af
12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=2.10 cfs 0.330 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=62.12' Storage=55 cf Inflow=2.11 cfs 0.331 af
Discarded=0.00 cfs 0.002 af Primary=2.10 cfs 0.330 af Secondary=0.00 cfs 0.000 af Outflow=2.10 cfs 0.331 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=63.54' Inflow=0.42 cfs 0.178 af
Outflow=0.42 cfs 0.178 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=74.37' Storage=637 cf Inflow=0.21 cfs 0.182 af
Discarded=0.01 cfs 0.029 af Primary=0.20 cfs 0.152 af Outflow=0.21 cfs 0.182 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=78.11' Storage=2,667 cf Inflow=2.23 cfs 0.182 af
Primary=0.21 cfs 0.182 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=0.21 cfs 0.182 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=62.46' Inflow=0.99 cfs 0.091 af
Outflow=0.99 cfs 0.091 af

Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=2.96 cfs 0.422 af
Primary=2.96 cfs 0.422 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=1.69 cfs 0.145 af
Primary=1.69 cfs 0.145 af

Link L103: Flow South to Hayes Park Inflow=0.53 cfs 0.056 af
Primary=0.53 cfs 0.056 af

Link L104: Flows West towards Squamscott River Inflow=1.32 cfs 0.158 af
Primary=1.32 cfs 0.158 af

Total Runoff Area = 5.372 ac Runoff Volume = 0.865 af Average Runoff Depth = 1.93"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac

Summary for Subcatchment PS25A: Northside of Proposed Clubhouse and Drip Edge

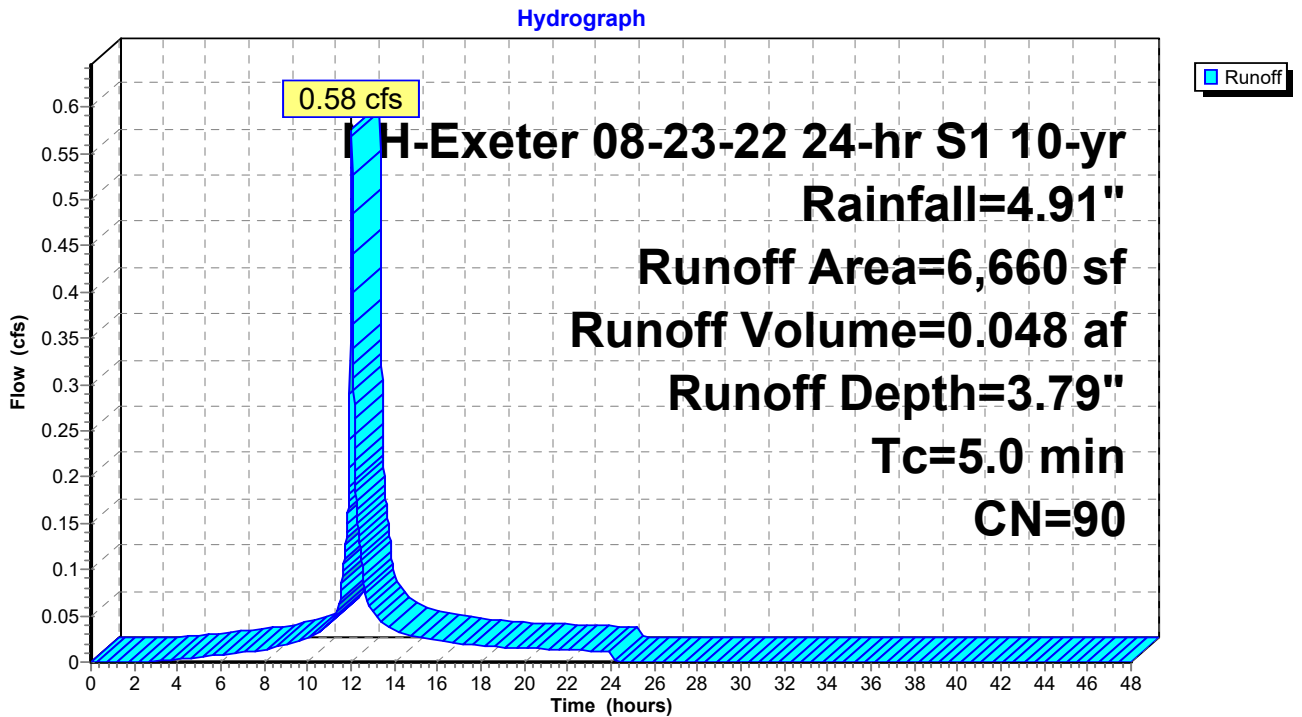
Runoff = 0.58 cfs @ 12.03 hrs, Volume= 0.048 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
1,430	61	>75% Grass cover, Good, HSG B
4,960	98	Roofs, HSG B
270	98	Paved parking, HSG B
6,660	90	Weighted Average
1,430		21.47% Pervious Area
5,230		78.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS25A: Northside of Proposed Clubhouse and Drip Edge



Summary for Subcatchment PS25B: Southside of Proposed Clubhouse and Drip Edge

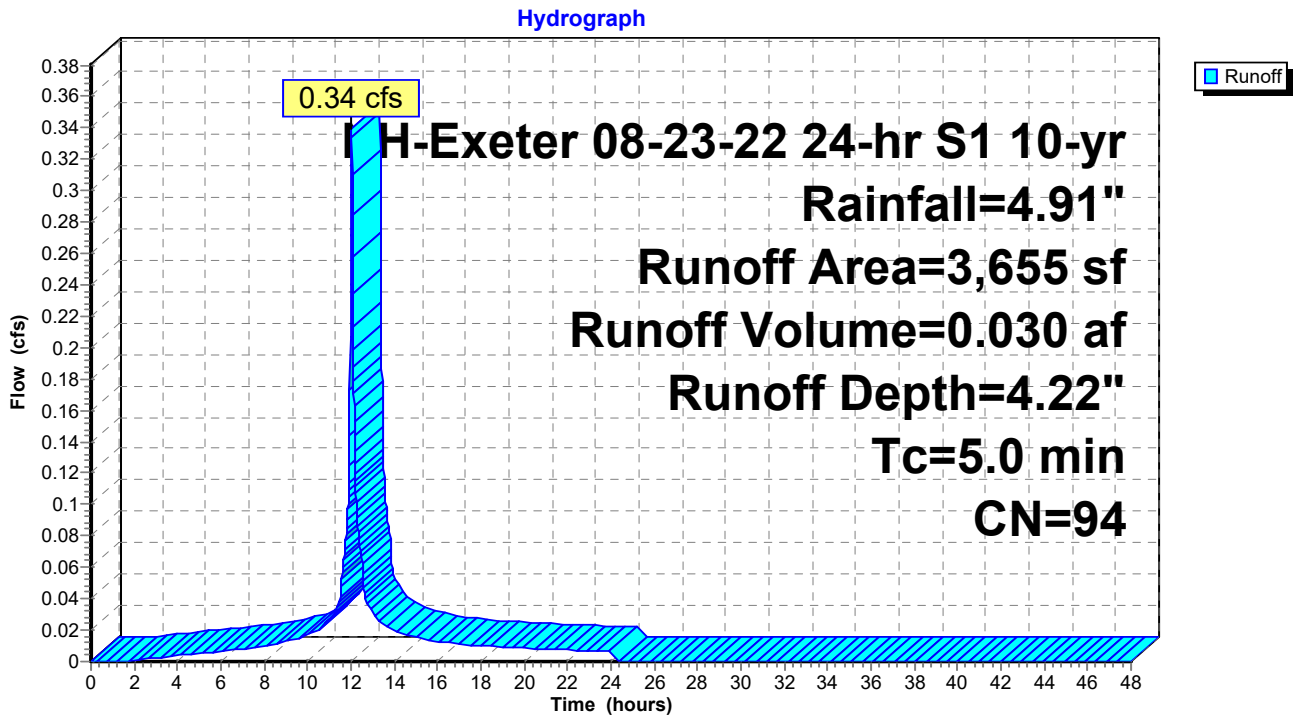
Runoff = 0.34 cfs @ 12.03 hrs, Volume= 0.030 af, Depth= 4.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
400	61	>75% Grass cover, Good, HSG B
3,255	98	Roofs, HSG B
3,655	94	Weighted Average
400		10.94% Pervious Area
3,255		89.06% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS25B: Southside of Proposed Clubhouse and Drip Edge



Summary for Subcatchment PS26: Northern Portion of 9th Green and North of Existing Clubhouse

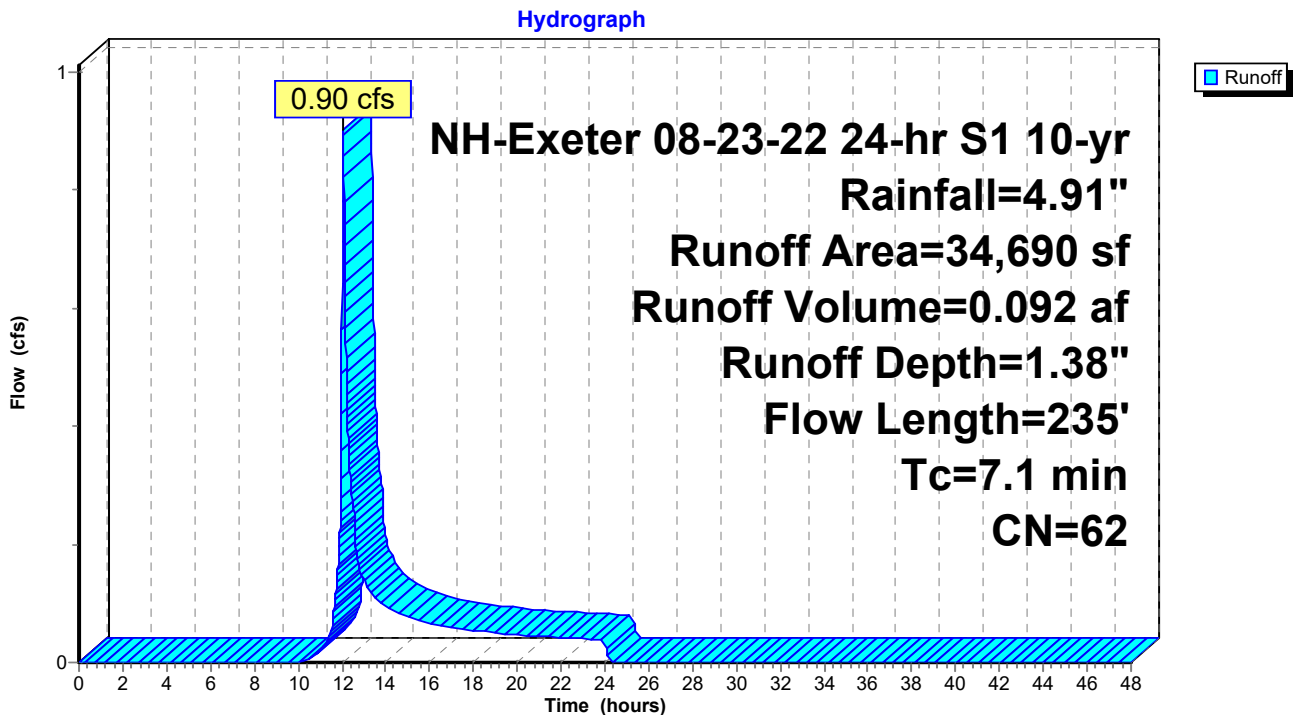
Runoff = 0.90 cfs @ 12.06 hrs, Volume= 0.092 af, Depth= 1.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
32,900	61	>75% Grass cover, Good, HSG B
1,265	85	Gravel roads, HSG B
525	98	Paved parking, HSG B
34,690	62	Weighted Average
34,165		98.49% Pervious Area
525		1.51% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	50	0.0200	0.15		Sheet Flow, Grass Grass: Short n= 0.150 P2= 3.22"
0.6	55	0.0550	1.64		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
0.9	130	0.1100	2.32		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
7.1	235	Total			

Subcatchment PS26: Northern Portion of 9th Green and North of Existing Clubhouse



Summary for Subcatchment PS27A: Existing 1st Hole Tee Boxes

Runoff = 0.60 cfs @ 12.03 hrs, Volume= 0.052 af, Depth= 1.59"

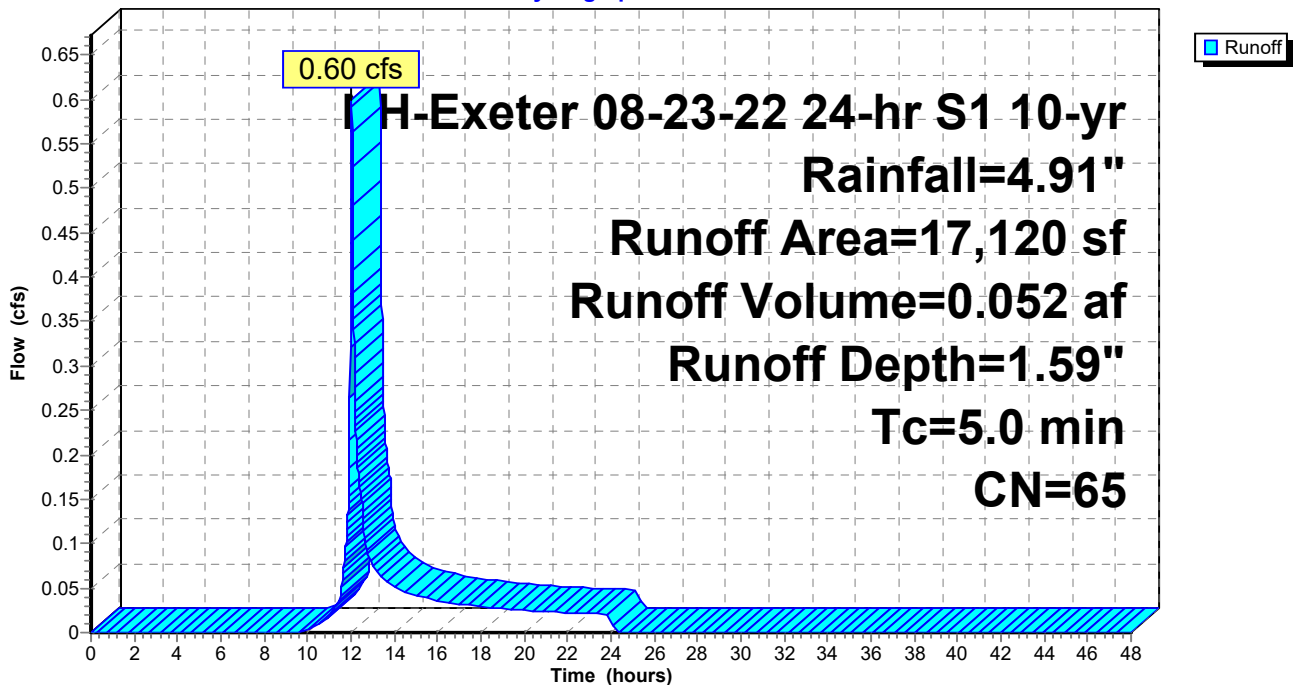
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
13,160	61	>75% Grass cover, Good, HSG B
1,365	55	Woods, Good, HSG B
1,775	85	Gravel roads, HSG B
820	98	Paved parking, HSG B
17,120	65	Weighted Average
16,300		95.21% Pervious Area
820		4.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS27A: Existing 1st Hole Tee Boxes

Hydrograph



Summary for Subcatchment PS27B: Existing Northern Parking Lot

Runoff = 0.99 cfs @ 12.03 hrs, Volume= 0.091 af, Depth= 4.67"

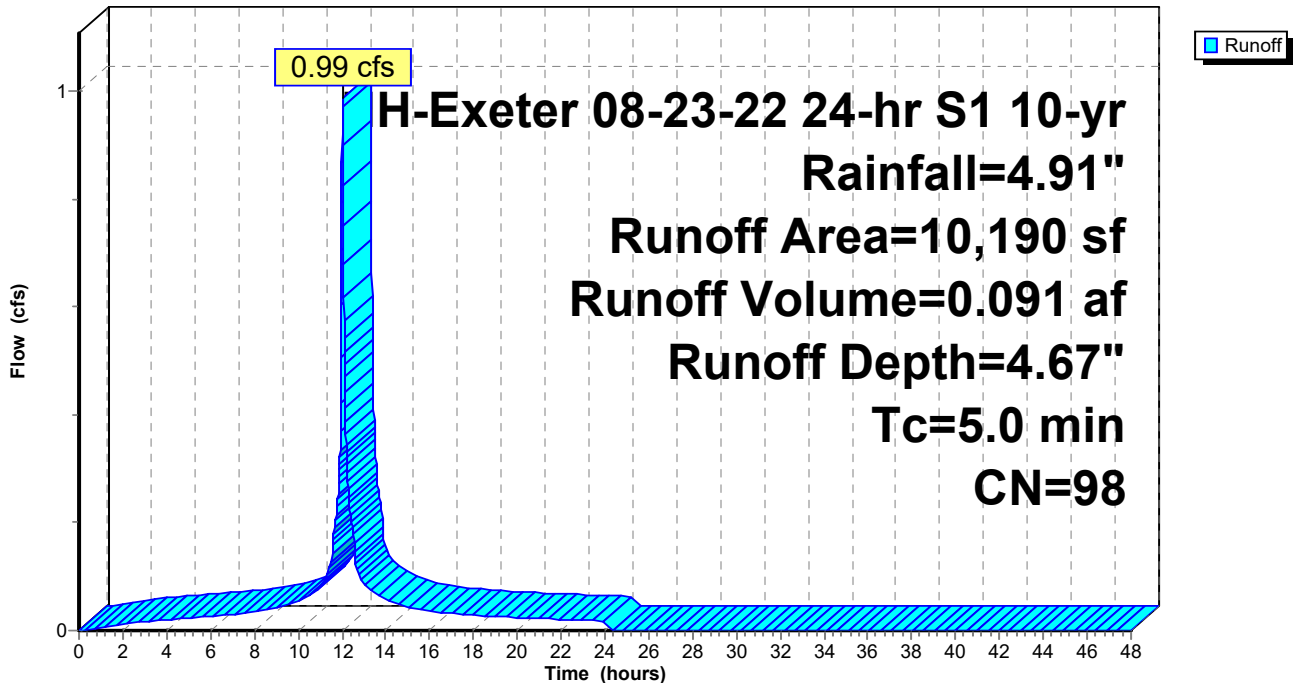
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
65	61	>75% Grass cover, Good, HSG B
10,125	98	Paved parking, HSG B
10,190	98	Weighted Average
65		0.64% Pervious Area
10,125		99.36% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS27B: Existing Northern Parking Lot

Hydrograph



Summary for Subcatchment PS27C: Portion of Proposed Parking and Northern Portion of Southern Lower Pa

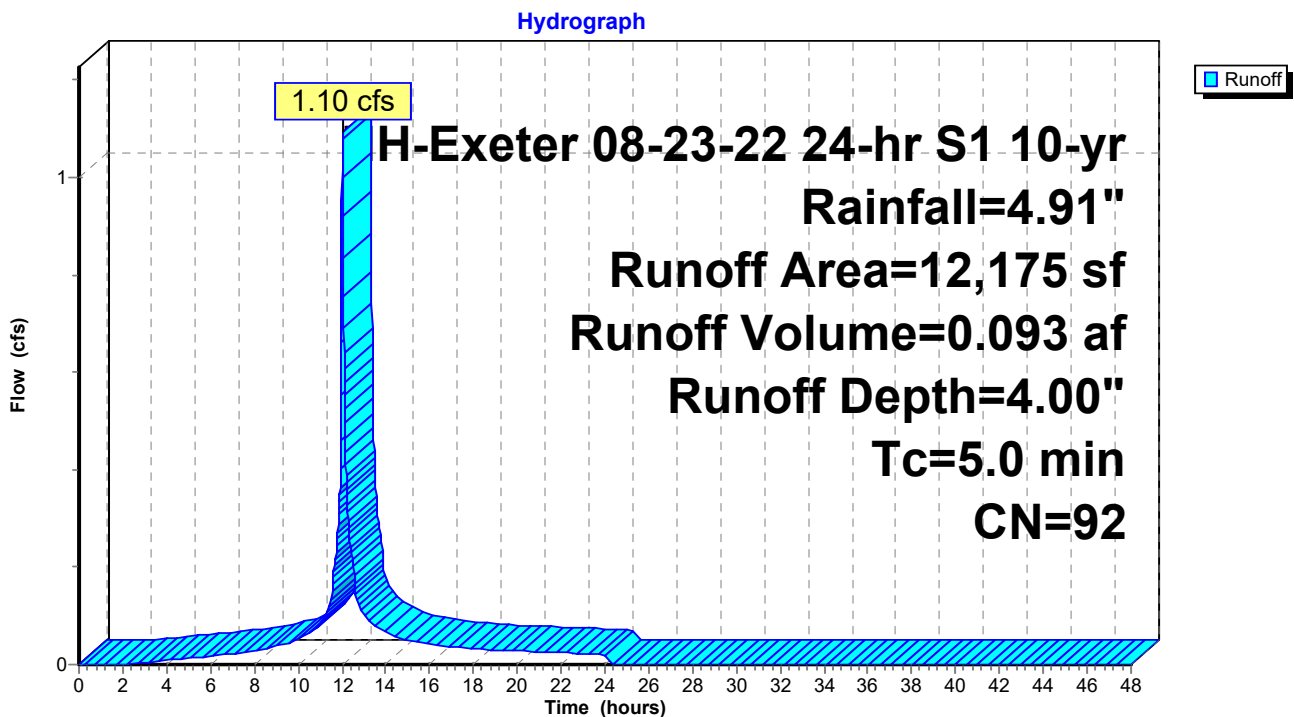
Runoff = 1.10 cfs @ 12.03 hrs, Volume= 0.093 af, Depth= 4.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
2,060	61	>75% Grass cover, Good, HSG B
10,115	98	Paved parking, HSG B
12,175	92	Weighted Average
2,060		16.92% Pervious Area
10,115		83.08% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS27C: Portion of Proposed Parking and Northern Portion of Southern Lower Parking L



Summary for Subcatchment PS28: Middle Strip of Existing Parking Lot

Runoff = 0.70 cfs @ 12.03 hrs, Volume= 0.063 af, Depth= 4.44"

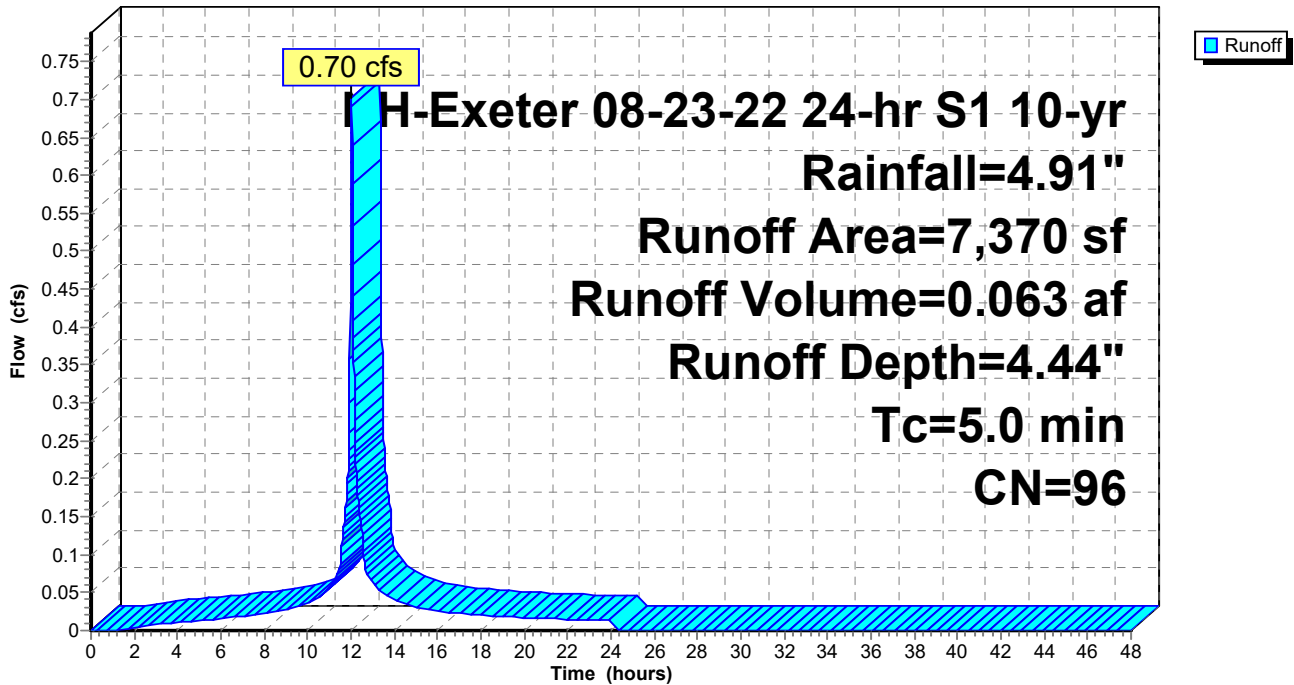
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
475	61	>75% Grass cover, Good, HSG B
6,895	98	Paved parking, HSG B
7,370	96	Weighted Average
475		6.45% Pervious Area
6,895		93.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS28: Middle Strip of Existing Parking Lot

Hydrograph



Summary for Subcatchment PS29: Southern Portion of Existing Parking Lot

Runoff = 0.31 cfs @ 12.03 hrs, Volume= 0.025 af, Depth= 3.28"

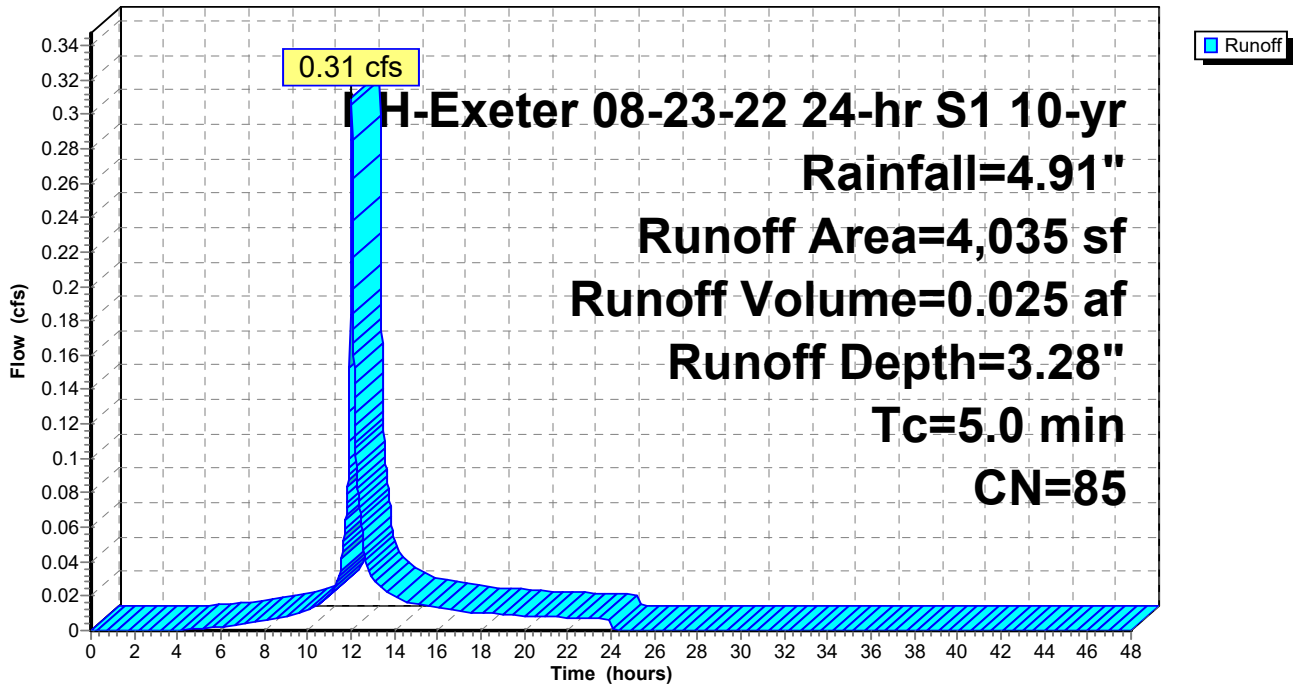
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
1,455	61	>75% Grass cover, Good, HSG B
2,580	98	Paved parking, HSG B
4,035	85	Weighted Average
1,455		36.06% Pervious Area
2,580		63.94% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS29: Southern Portion of Existing Parking Lot

Hydrograph



Summary for Subcatchment PS30: Woods to the South of 9th Green & Parking Lot

Runoff = 0.53 cfs @ 12.04 hrs, Volume= 0.056 af, Depth= 1.00"

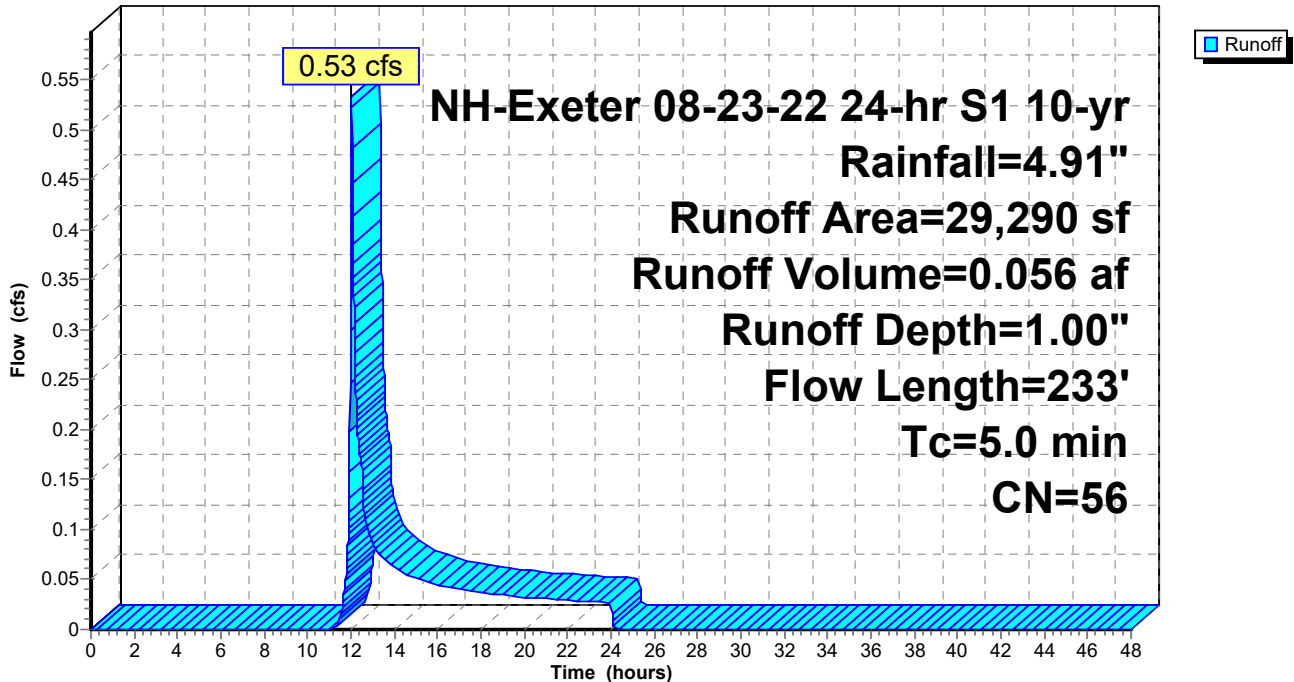
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
6,820	61	>75% Grass cover, Good, HSG B
22,470	55	Woods, Good, HSG B
29,290	56	Weighted Average
29,290		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.6	50	0.0600	0.23		Sheet Flow, Grass Grass: Short n= 0.150 P2= 3.22"
0.1	15	0.1200	2.42		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
0.2	10	0.0200	0.71		Shallow Concentrated Flow, Gravel Woodland Kv= 5.0 fps
0.1	8	0.1200	2.42		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
1.0	150	0.2500	2.50		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
5.0	233	Total			

Subcatchment PS30: Woods to the South of 9th Green & Parking Lot

Hydrograph



Summary for Subcatchment PS31: Woods to the West of 9th Green

Runoff = 1.32 cfs @ 12.06 hrs, Volume= 0.158 af, Depth= 1.00"

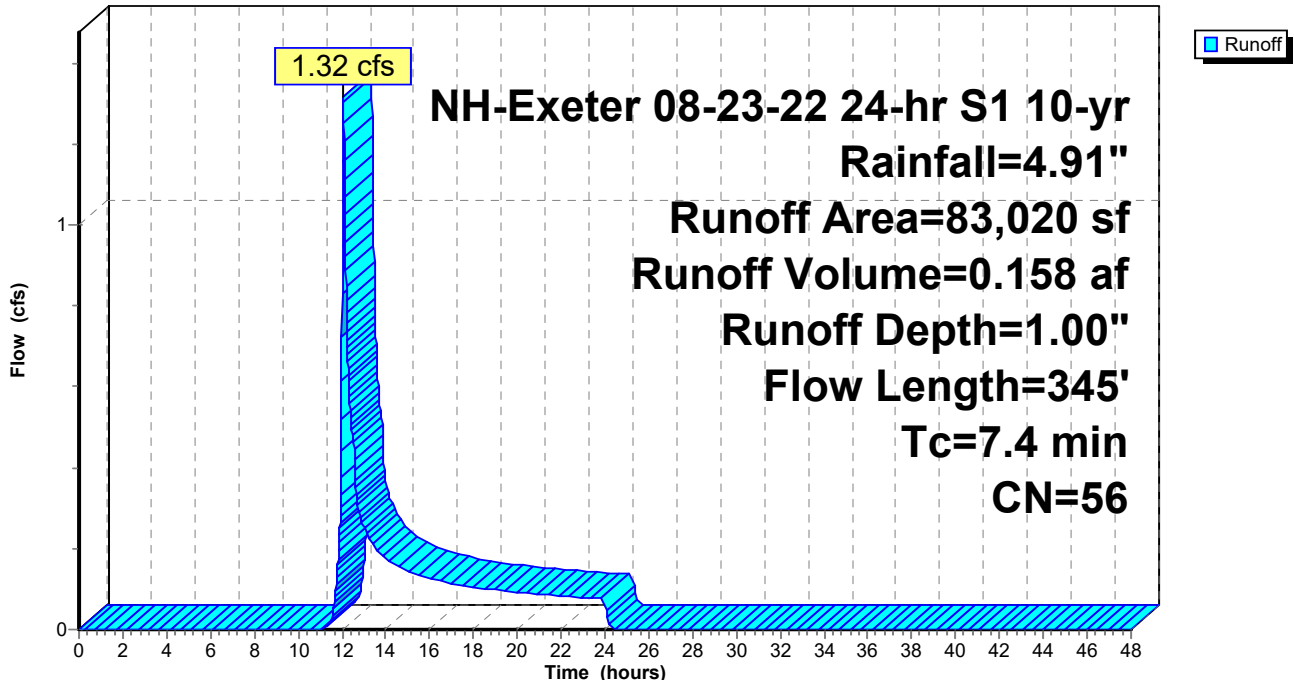
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
8,395	61	>75% Grass cover, Good, HSG B
74,625	55	Woods, Good, HSG B
83,020	56	Weighted Average
83,020		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.3	50	0.0400	0.20		Sheet Flow, Grass Grass: Short n= 0.150 P2= 3.22"
0.3	22	0.0400	1.40		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
0.1	11	0.0200	2.28		Shallow Concentrated Flow, Gravel Unpaved Kv= 16.1 fps
0.1	12	0.0400	1.40		Shallow Concentrated Flow, Grass Short Grass Pasture Kv= 7.0 fps
2.6	250	0.1000	1.58		Shallow Concentrated Flow, Woods Woodland Kv= 5.0 fps
7.4	345	Total			

Subcatchment PS31: Woods to the West of 9th Green

Hydrograph



Summary for Subcatchment PS32: Proposed Parking Lot Area

Runoff = 1.93 cfs @ 12.03 hrs, Volume= 0.157 af, Depth= 3.19"

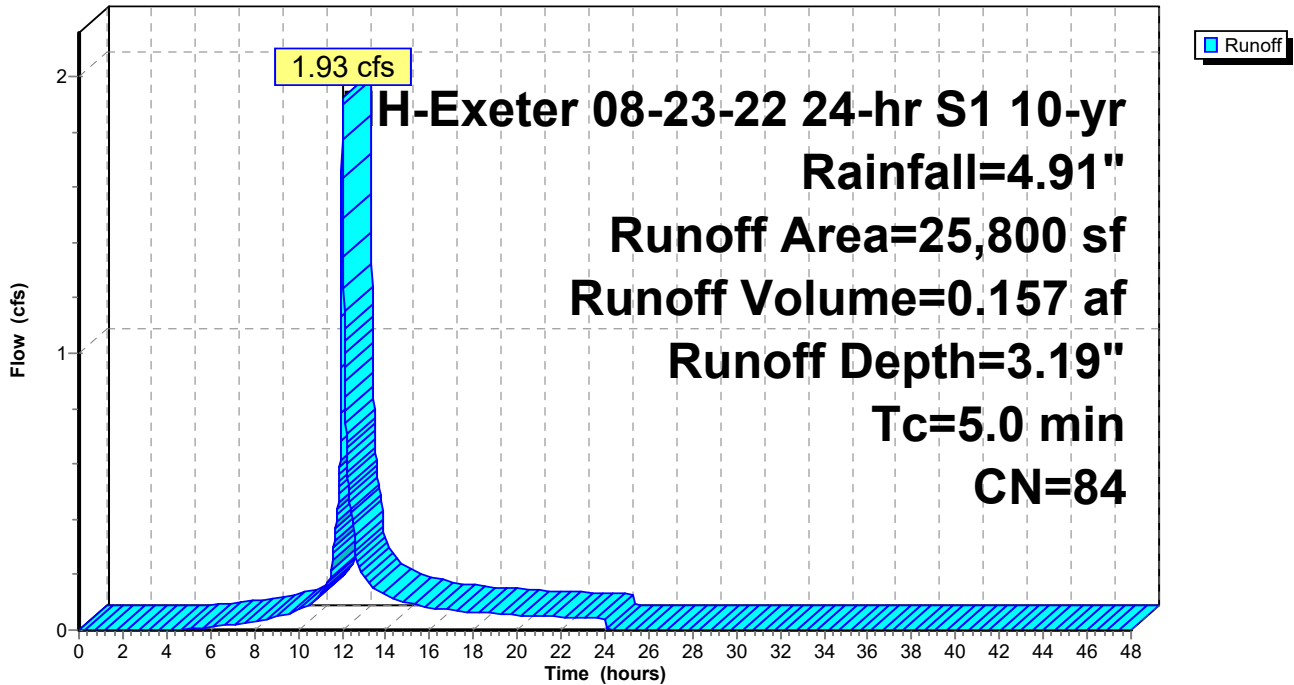
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs
 NH-Exeter 08-23-22 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description
8,840	61	>75% Grass cover, Good, HSG B
1,800	85	Gravel roads, HSG B
15,160	98	Paved parking, HSG B
25,800	84	Weighted Average
10,640		41.24% Pervious Area
15,160		58.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Minimum Tc

Subcatchment PS32: Proposed Parking Lot Area

Hydrograph



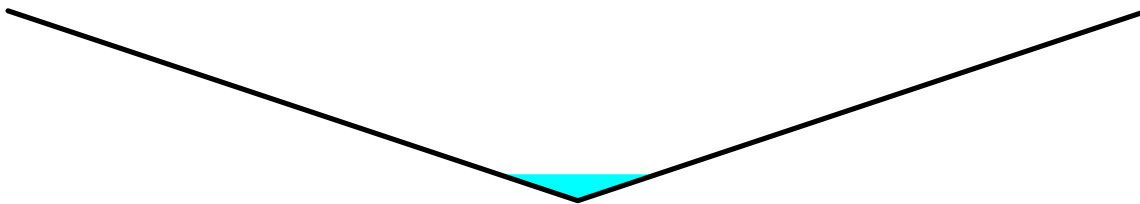
Summary for Reach PR75: Proposed Swale Running South of Existing Parking Lot

Inflow Area = 0.676 ac, 62.52% Impervious, Inflow Depth = 2.70" for 10-yr event
 Inflow = 0.20 cfs @ 13.23 hrs, Volume= 0.152 af
 Outflow = 0.20 cfs @ 13.24 hrs, Volume= 0.152 af, Atten= 0%, Lag= 0.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Max. Velocity= 3.40 fps, Min. Travel Time= 0.4 min
 Avg. Velocity = 2.84 fps, Avg. Travel Time= 0.5 min

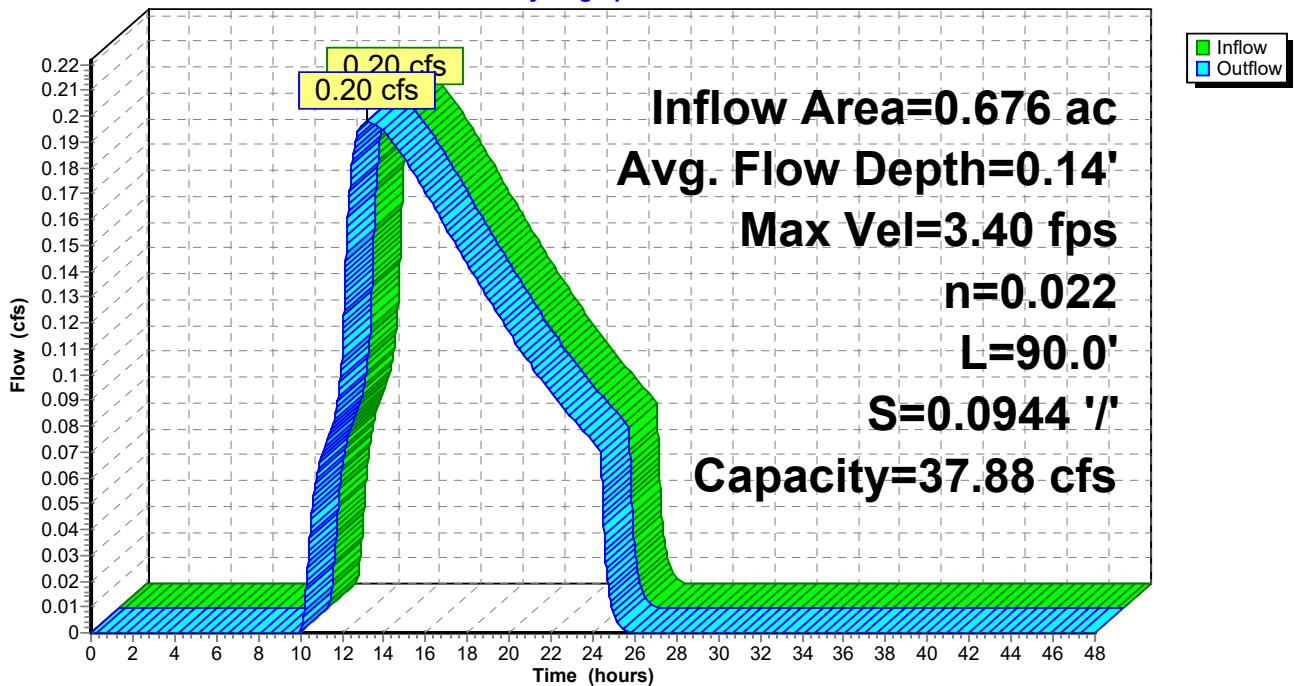
Peak Storage= 5 cf @ 13.24 hrs
 Average Depth at Peak Storage= 0.14' , Surface Width= 0.84'
 Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 37.88 cfs

0.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
 Side Slope Z-value= 3.0 '/' Top Width= 6.00'
 Length= 90.0' Slope= 0.0944 '/'
 Inlet Invert= 74.00', Outlet Invert= 65.50'



Reach PR75: Proposed Swale Running South of Existing Parking Lot

Hydrograph



Summary for Pond EP10: 9th Hole Pond

Inflow Area = 2.121 ac, 47.37% Impervious, Inflow Depth = 2.39" for 10-yr event
 Inflow = 2.96 cfs @ 12.04 hrs, Volume= 0.422 af
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 38.79' @ 25.91 hrs Surf.Area= 12,336 sf Storage= 18,366 cf
 Flood Elev= 44.00' Surf.Area= 28,470 sf Storage= 127,940 cf

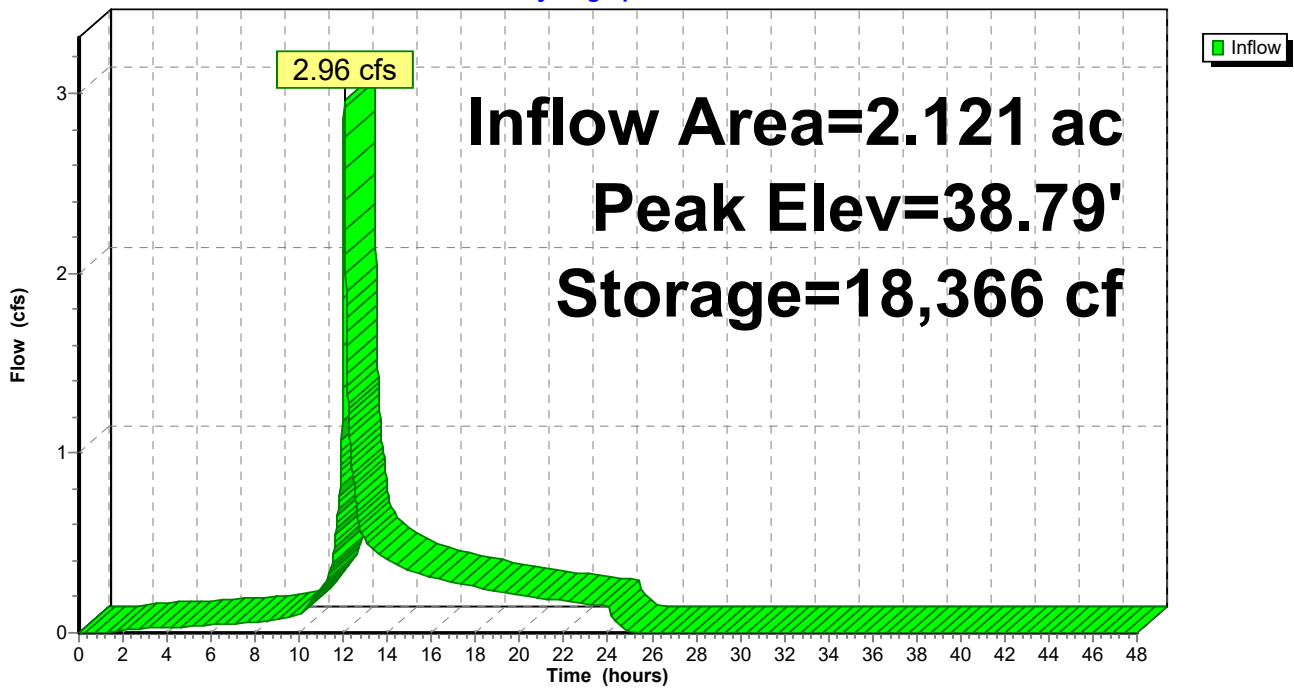
Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	36.00'	127,940 cf	Pond by 9th Hole Tee Boxes (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
36.00	850	0	0
38.00	9,050	9,900	9,900
40.00	17,350	26,400	36,300
44.00	28,470	91,640	127,940

Pond EP10: 9th Hole Pond

Hydrograph



Summary for Pond PP50: Proposed Stone Storage Under Proposed Deck

Inflow Area = 0.153 ac, 78.53% Impervious, Inflow Depth = 3.79" for 10-yr event
 Inflow = 0.58 cfs @ 12.03 hrs, Volume= 0.048 af
 Outflow = 0.02 cfs @ 18.81 hrs, Volume= 0.048 af, Atten= 97%, Lag= 407.0 min
 Discarded = 0.02 cfs @ 18.81 hrs, Volume= 0.048 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 78.30' @ 18.81 hrs Surf.Area= 2,150 sf Storage= 1,119 cf
 Flood Elev= 80.00' Surf.Area= 2,150 sf Storage= 2,580 cf

Plug-Flow detention time= 657.0 min calculated for 0.048 af (100% of inflow)
 Center-of-Mass det. time= 656.9 min (1,463.7 - 806.8)

Volume	Invert	Avail.Storage	Storage Description
#1	77.00'	2,580 cf	Stone Storage (Conic) Listed below (Recalc) 6,450 cf Overall x 40.0% Voids

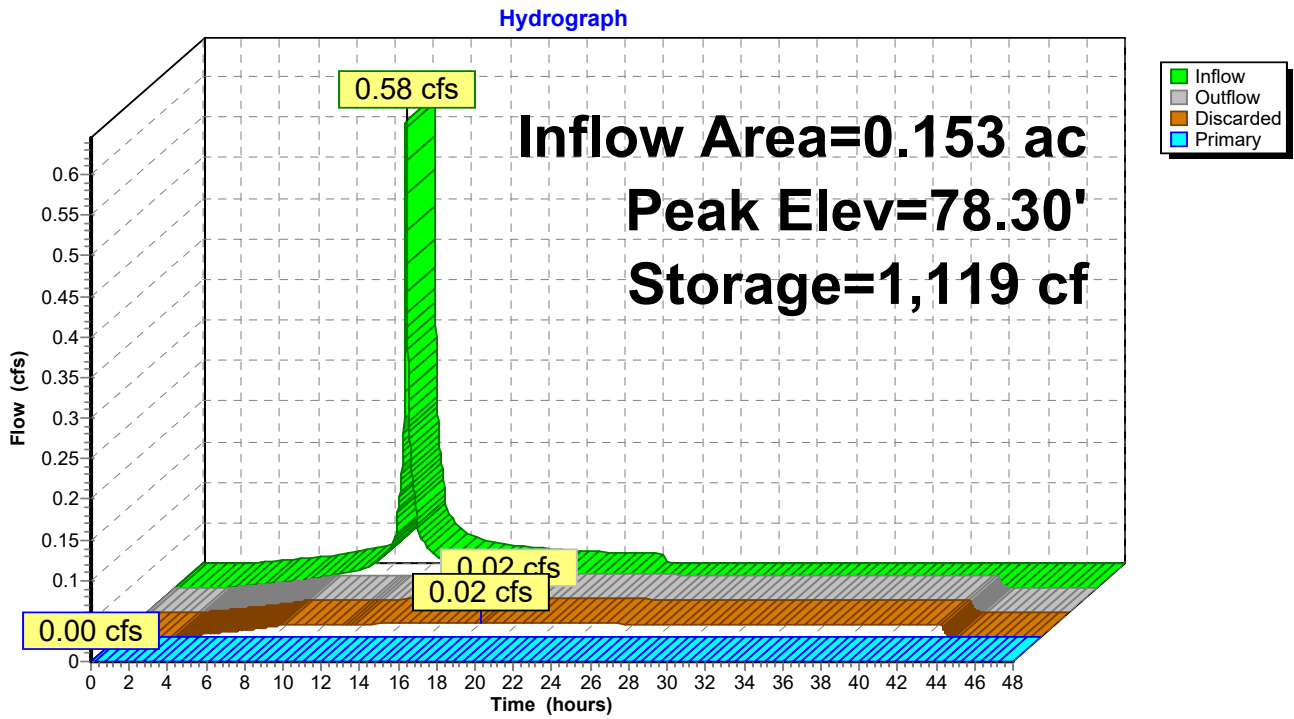
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
77.00	2,150	0	0	2,150
80.00	2,150	6,450	6,450	2,643

Device	Routing	Invert	Outlet Devices
#1	Primary	79.90'	15.0' long x 135.0' breadth Overflow Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	77.00'	0.300 in/hr Exfiltration into Groundwater over Wetted area Phase-In= 0.01'

Discarded OutFlow Max=0.02 cfs @ 18.81 hrs HW=78.30' (Free Discharge)
 ↳ **2=Exfiltration into Groundwater** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=77.00' TW=0.00' (Dynamic Tailwater)
 ↳ **1=Overflow** (Controls 0.00 cfs)

Pond PP50: Proposed Stone Storage Under Proposed Deck



Summary for Pond PP51: Proposed Drip Edge in front of Proposed Clubhouse

Inflow Area = 0.084 ac, 89.06% Impervious, Inflow Depth = 4.22" for 10-yr event
 Inflow = 0.34 cfs @ 12.03 hrs, Volume= 0.030 af
 Outflow = 0.32 cfs @ 12.05 hrs, Volume= 0.030 af, Atten= 7%, Lag= 1.3 min
 Discarded = 0.00 cfs @ 12.05 hrs, Volume= 0.005 af
 Primary = 0.31 cfs @ 12.05 hrs, Volume= 0.024 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 80.32' @ 12.05 hrs Surf.Area= 400 sf Storage= 51 cf
 Flood Elev= 83.00' Surf.Area= 400 sf Storage= 480 cf

Plug-Flow detention time= 8.2 min calculated for 0.029 af (100% of inflow)
 Center-of-Mass det. time= 8.2 min (790.9 - 782.7)

Volume	Invert	Avail.Storage	Storage Description
#1	80.00'	480 cf	Stone Storage (Conic) Listed below (Recalc) 1,200 cf Overall x 40.0% Voids

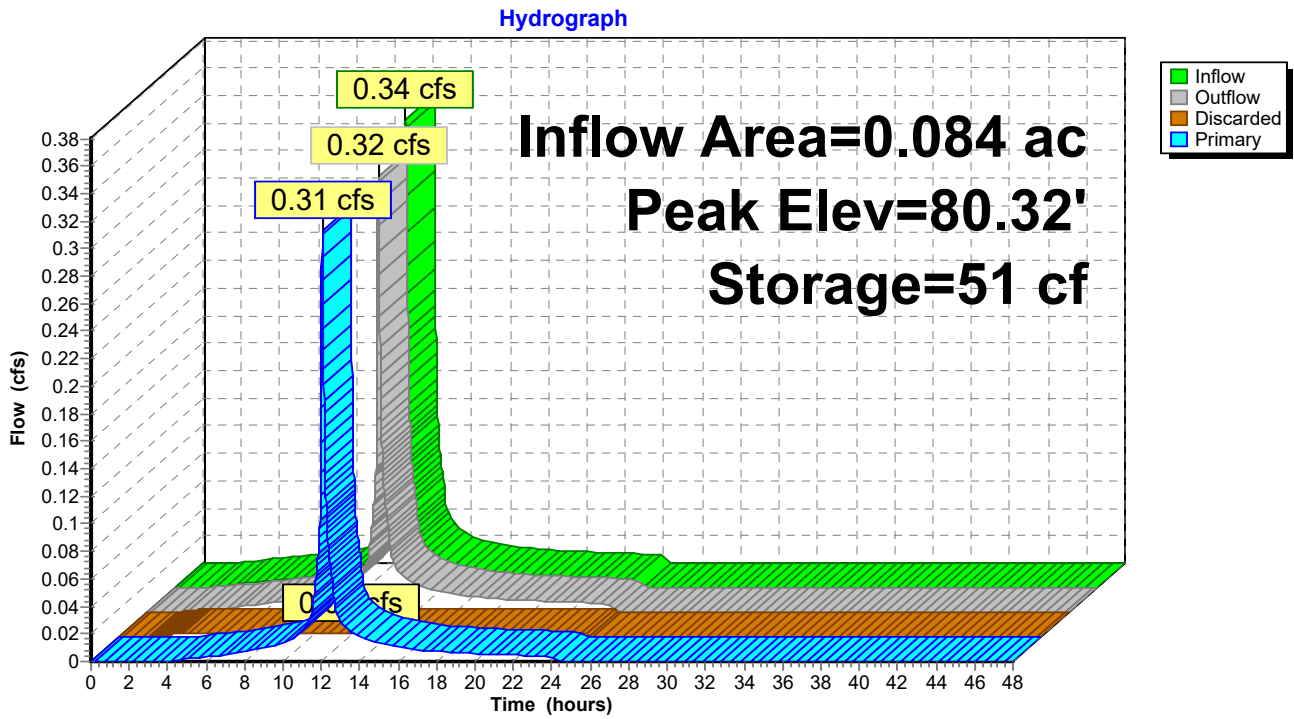
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
80.00	400	0	0	400
83.00	400	1,200	1,200	613

Device	Routing	Invert	Outlet Devices
#1	Primary	80.00'	8.0" Round 8" SDR-35 Pipe L= 145.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 80.00' / 77.00' S= 0.0207 ' / ' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.35 sf
#2	Discarded	80.00'	0.300 in/hr Exfiltration into Groundwater over Wetted area

Discarded OutFlow Max=0.00 cfs @ 12.05 hrs HW=80.32' (Free Discharge)
 ↳ **2=Exfiltration into Groundwater** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.31 cfs @ 12.05 hrs HW=80.32' TW=77.50' (Dynamic Tailwater)
 ↳ **1=8" SDR-35 Pipe** (Inlet Controls 0.31 cfs @ 1.92 fps)

Pond PP51: Proposed Drip Edge in front of Proposed Clubhouse



Summary for Pond PP52A: Proposed Manhole in 1st Hole Fairway

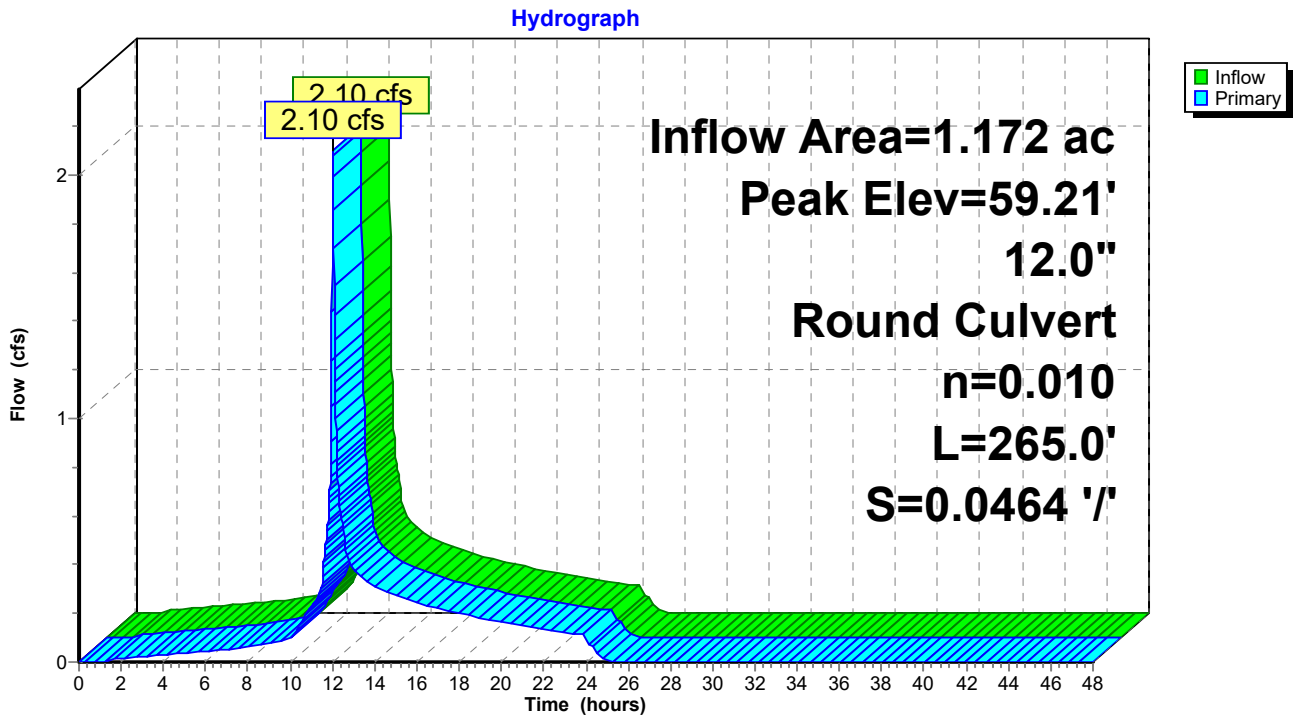
Inflow Area = 1.172 ac, 74.47% Impervious, Inflow Depth = 3.38" for 10-yr event
 Inflow = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af
 Outflow = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af, Atten= 0%, Lag= 0.0 min
 Primary = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 59.21' @ 12.03 hrs
 Flood Elev= 62.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	58.40'	12.0" Round 12" HDPE Pipe L= 265.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 58.40' / 46.10' S= 0.0464 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.10 cfs @ 12.03 hrs HW=59.21' TW=0.00' (Dynamic Tailwater)
 ↳ 1=12" HDPE Pipe (Inlet Controls 2.10 cfs @ 3.07 fps)

Pond PP52A: Proposed Manhole in 1st Hole Fairway



Summary for Pond PP52B: Proposed Manhole near Cart Storage

Inflow Area = 1.172 ac, 74.47% Impervious, Inflow Depth = 3.38" for 10-yr event
 Inflow = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af
 Outflow = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af, Atten= 0%, Lag= 0.0 min
 Primary = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af

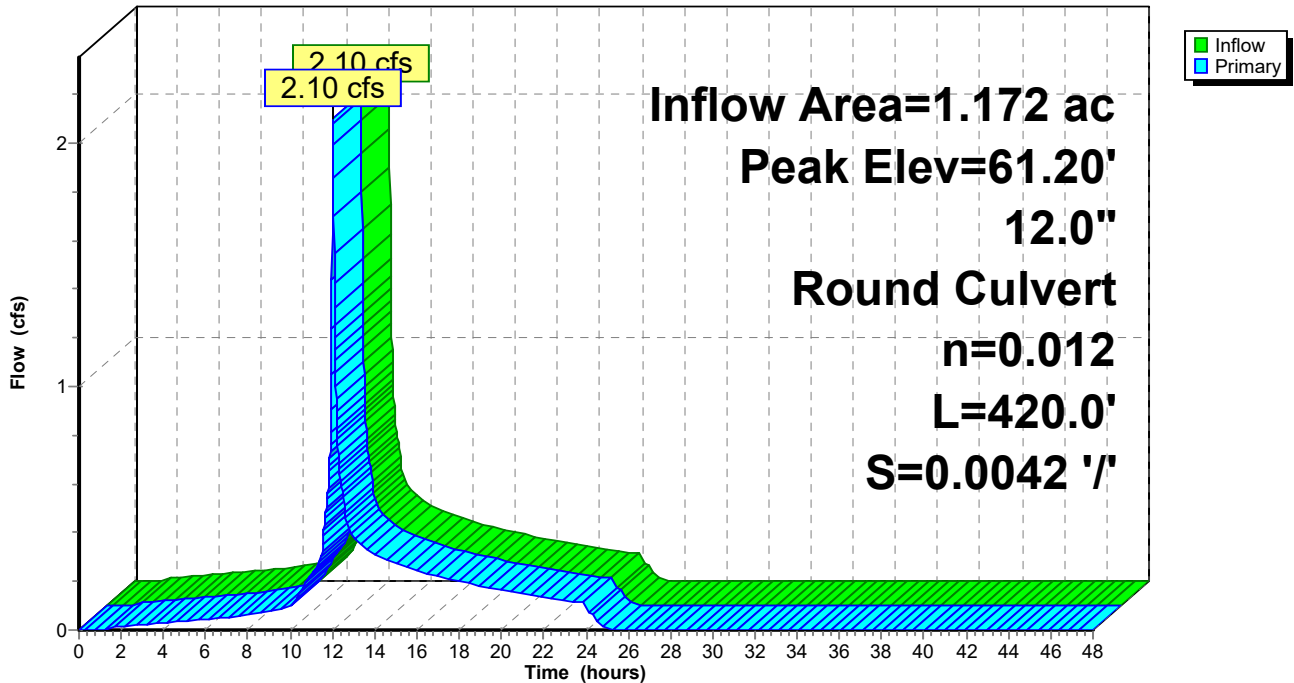
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 61.20' @ 12.03 hrs
 Flood Elev= 63.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	60.25'	12.0" Round 12" HDPE Pipe L= 420.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 60.25' / 58.50' S= 0.0042 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.10 cfs @ 12.03 hrs HW=61.20' TW=59.21' (Dynamic Tailwater)
 ↳ 1=12" HDPE Pipe (Outlet Controls 2.10 cfs @ 3.51 fps)

Pond PP52B: Proposed Manhole near Cart Storage

Hydrograph



Summary for Pond PP53: Proposed Rock Infiltration Area in Proposed Parking Island

Inflow Area = 1.172 ac, 74.47% Impervious, Inflow Depth = 3.39" for 10-yr event
 Inflow = 2.11 cfs @ 12.03 hrs, Volume= 0.331 af
 Outflow = 2.10 cfs @ 12.03 hrs, Volume= 0.331 af, Atten= 0%, Lag= 0.3 min
 Discarded = 0.00 cfs @ 12.03 hrs, Volume= 0.002 af
 Primary = 2.10 cfs @ 12.03 hrs, Volume= 0.330 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 62.12' @ 12.03 hrs Surf.Area= 91 sf Storage= 55 cf
 Flood Elev= 64.00' Surf.Area= 800 sf Storage= 198 cf

Plug-Flow detention time= 2.6 min calculated for 0.331 af (100% of inflow)
 Center-of-Mass det. time= 2.7 min (883.0 - 880.2)

Volume	Invert	Avail.Storage	Storage Description
#1	60.50'	198 cf	Stone Storage (Prismatic) Listed below (Recalc) 495 cf Overall x 40.0% Voids

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
60.50	80	0	0
63.50	100	270	270
64.00	800	225	495

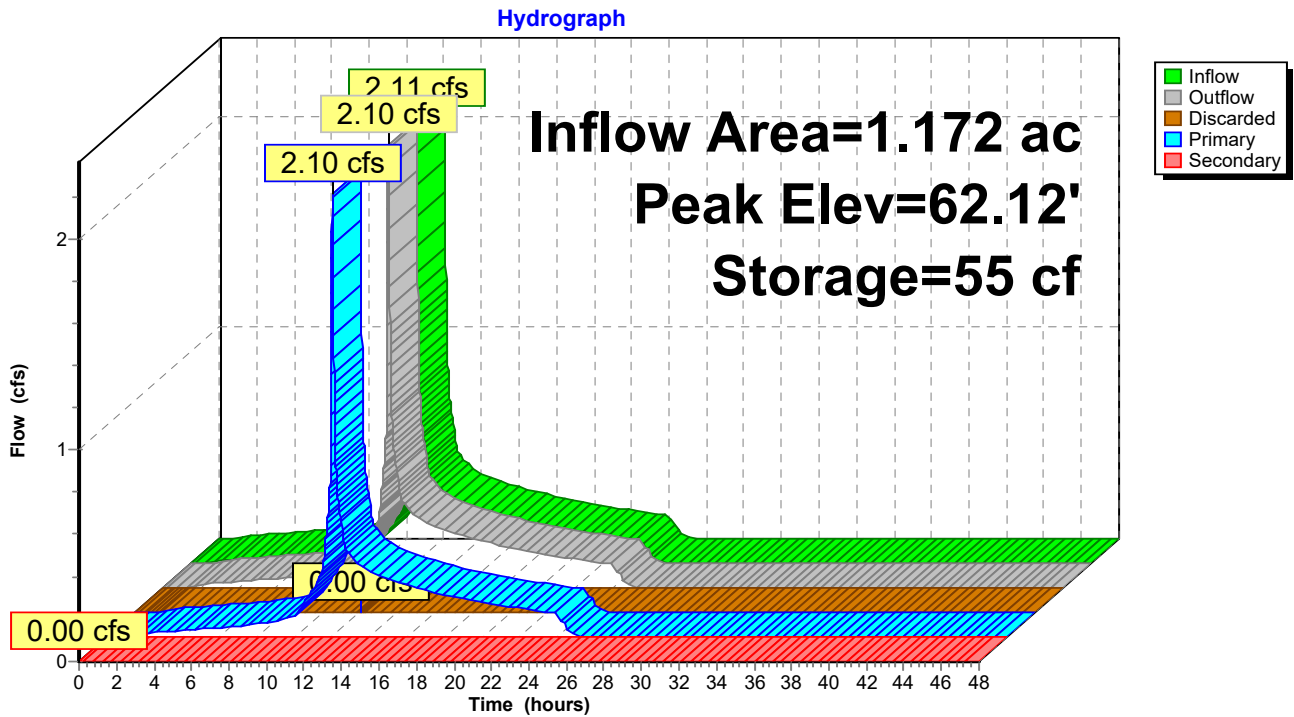
Device	Routing	Invert	Outlet Devices
#1	Primary	61.00'	12.0" Round 12" HDPE Pipe L= 245.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 61.00' / 60.35' S= 0.0027 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Secondary	63.90'	15.0' long x 40.0' breadth Overflow into Road Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#3	Discarded	60.50'	0.300 in/hr Exfiltration into Groundwater over Surface area

Discarded OutFlow Max=0.00 cfs @ 12.03 hrs HW=62.11' (Free Discharge)
 ↗**3=Exfiltration into Groundwater** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=2.10 cfs @ 12.03 hrs HW=62.11' TW=61.20' (Dynamic Tailwater)
 ↗**1=12" HDPE Pipe** (Outlet Controls 2.10 cfs @ 3.00 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=60.50' TW=0.00' (Dynamic Tailwater)
 ↗**2=Overflow into Road** (Controls 0.00 cfs)

Pond PP53: Proposed Rock Infiltration Area in Proposed Parking Island



Summary for Pond PP54: Proposed Catch Basin at Southernmost Site Entrance

Inflow Area = 0.769 ac, 62.69% Impervious, Inflow Depth = 2.77" for 10-yr event
 Inflow = 0.42 cfs @ 12.03 hrs, Volume= 0.178 af
 Outflow = 0.42 cfs @ 12.03 hrs, Volume= 0.178 af, Atten= 0%, Lag= 0.0 min
 Primary = 0.42 cfs @ 12.03 hrs, Volume= 0.178 af

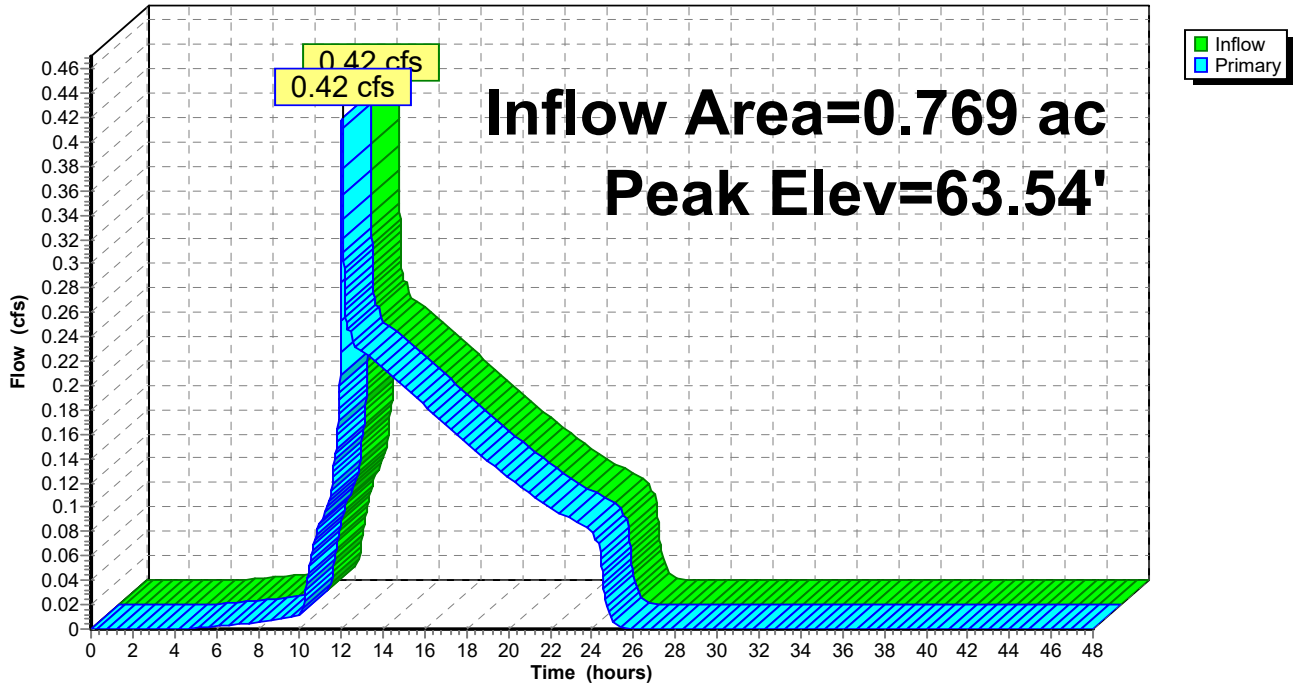
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 63.54' @ 12.03 hrs
 Flood Elev= 65.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	63.00'	12.0" Round 12" HDPE Pipe L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 63.00' / 61.10' S= 0.0380 '/ Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.79 sf
#2	Device 1	63.17'	8.0" Vert. 8" Oriface C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.42 cfs @ 12.03 hrs HW=63.54' TW=62.11' (Dynamic Tailwater)
 1=12" HDPE Pipe (Passes 0.42 cfs of 1.10 cfs potential flow)
 2=8" Oriface (Orifice Controls 0.42 cfs @ 2.08 fps)

Pond PP54: Proposed Catch Basin at Southernmost Site Entrance

Hydrograph



Summary for Pond PP55: Proposed Bioretention Area "A" Subsurface

Inflow Area = 0.676 ac, 62.52% Impervious, Inflow Depth = 3.23" for 10-yr event
 Inflow = 0.21 cfs @ 13.08 hrs, Volume= 0.182 af
 Outflow = 0.21 cfs @ 13.23 hrs, Volume= 0.182 af, Atten= 0%, Lag= 9.3 min
 Discarded = 0.01 cfs @ 13.23 hrs, Volume= 0.029 af
 Primary = 0.20 cfs @ 13.23 hrs, Volume= 0.152 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 74.37' @ 13.23 hrs Surf.Area= 1,490 sf Storage= 637 cf

Plug-Flow detention time= 95.0 min calculated for 0.182 af (100% of inflow)
 Center-of-Mass det. time= 95.2 min (1,054.4 - 959.2)

Volume	Invert	Avail.Storage	Storage Description
#1	74.55'	522 cf	Bioretention Filter Layers (Conic) Listed below 2,608 cf Overall x 20.0% Voids
#2	73.30'	745 cf	Bioretention Storage Layers (Conic) Listed below (Recalc) 1,863 cf Overall x 40.0% Voids
		1,267 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
74.55	1,490	0	0	1,490
76.30	1,490	2,608	2,608	1,729

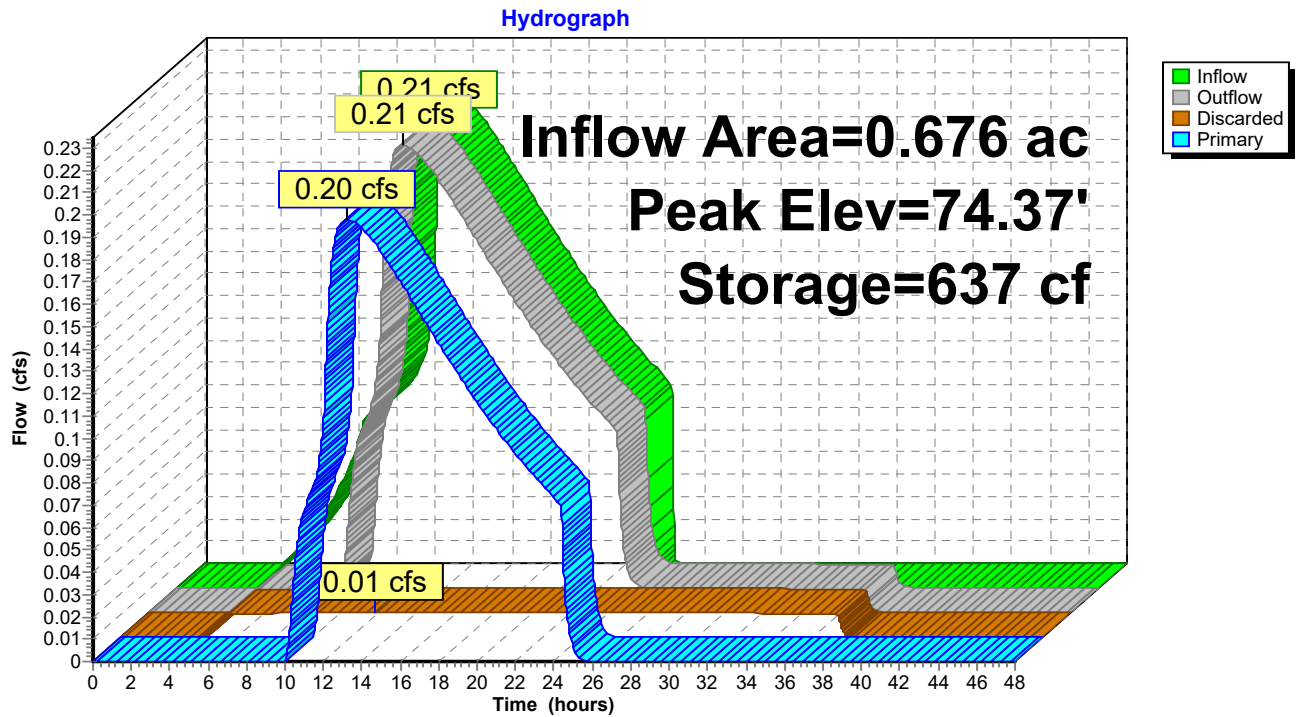
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
73.30	1,490	0	0	1,490
74.55	1,490	1,863	1,863	1,661

Device	Routing	Invert	Outlet Devices
#1	Primary	74.05'	8.0" Round 8" SDR-35 Pipe L= 75.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 74.05' / 73.90' S= 0.0020 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.35 sf
#2	Discarded	73.30'	0.300 in/hr Exfiltration into Groundwater over Wetted area Phase-In= 0.01'

Discarded OutFlow Max=0.01 cfs @ 13.23 hrs HW=74.37' (Free Discharge)
 ↑**2=Exfiltration into Groundwater** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.20 cfs @ 13.23 hrs HW=74.37' TW=74.14' (Dynamic Tailwater)
 ↑**1=8" SDR-35 Pipe** (Outlet Controls 0.20 cfs @ 1.76 fps)

Pond PP55: Proposed Bioretention Area "A" Subsurface



Summary for Pond PP56: Proposed Bioretention Area "A" Open Area

Inflow Area = 0.676 ac, 62.52% Impervious, Inflow Depth = 3.23" for 10-yr event
 Inflow = 2.23 cfs @ 12.03 hrs, Volume= 0.182 af
 Outflow = 0.21 cfs @ 13.08 hrs, Volume= 0.182 af, Atten= 91%, Lag= 62.8 min
 Primary = 0.21 cfs @ 13.08 hrs, Volume= 0.182 af
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
 Tertiary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 78.11' @ 13.08 hrs Surf.Area= 2,242 sf Storage= 2,667 cf
 Flood Elev= 78.80' Surf.Area= 2,970 sf Storage= 4,467 cf

Plug-Flow detention time= 130.3 min calculated for 0.182 af (100% of inflow)
 Center-of-Mass det. time= 130.3 min (959.2 - 828.9)

Volume	Invert	Avail.Storage	Storage Description
#1	76.30'	4,467 cf	Bioretention Area "A" (Conic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
76.30	825	0	0	825
78.80	2,970	4,467	4,467	3,001

Device	Routing	Invert	Outlet Devices
#1	Primary	76.30'	4.000 in/hr Exfiltration through Bioretention Mix over Wetted area Phase-In= 0.01'
#2	Secondary	78.20'	12.0" W x 4.0" H Vert. (3) 4"x12" Knockouts X 3.00 C= 0.600 Limited to weir flow at low heads
#3	Tertiary	78.50'	5.0' long x 5.0' breadth Overflow Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.21 cfs @ 13.08 hrs HW=78.11' TW=74.37' (Dynamic Tailwater)

↑1=**Exfiltration through Bioretention Mix** (Exfiltration Controls 0.21 cfs)

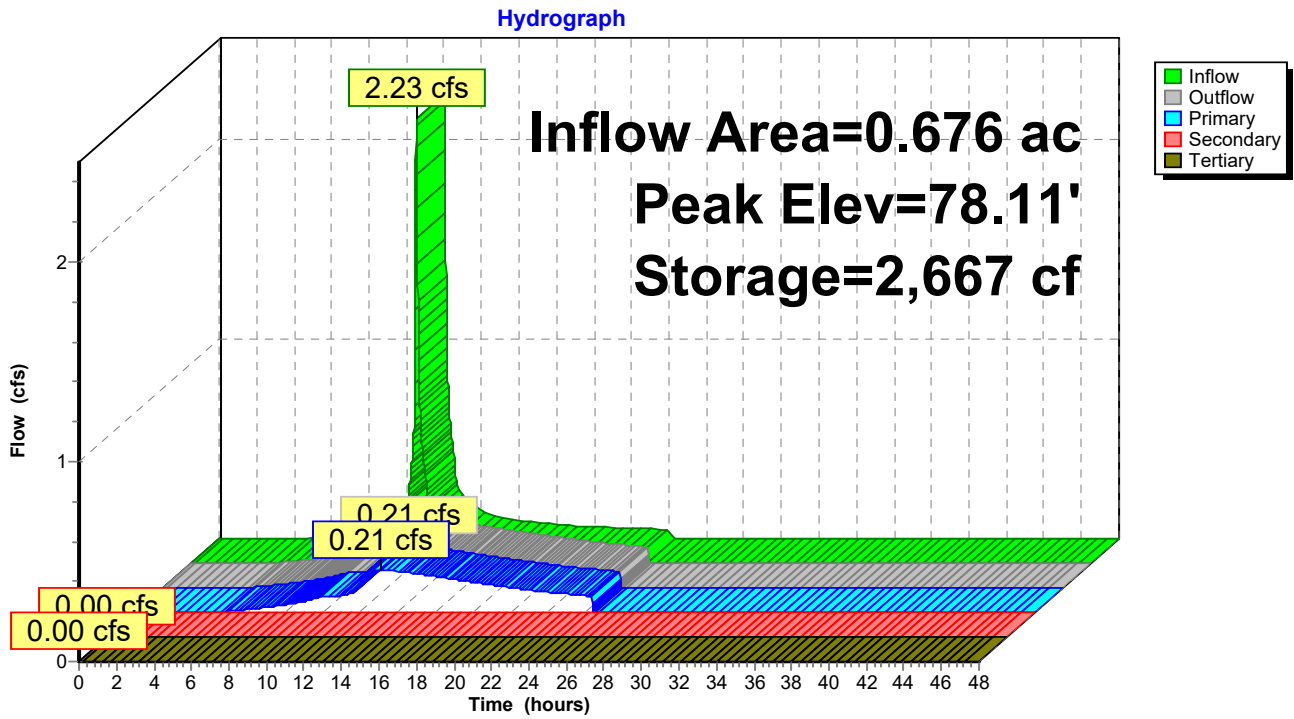
Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=76.30' TW=73.30' (Dynamic Tailwater)

↑2=(3) **4"x12" Knockouts** (Controls 0.00 cfs)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=76.30' TW=74.00' (Dynamic Tailwater)

↑3=**Overflow** (Controls 0.00 cfs)

Pond PP56: Proposed Bioretention Area "A" Open Area



Summary for Pond PP57: Proposed Catch Basin at Site Entrance

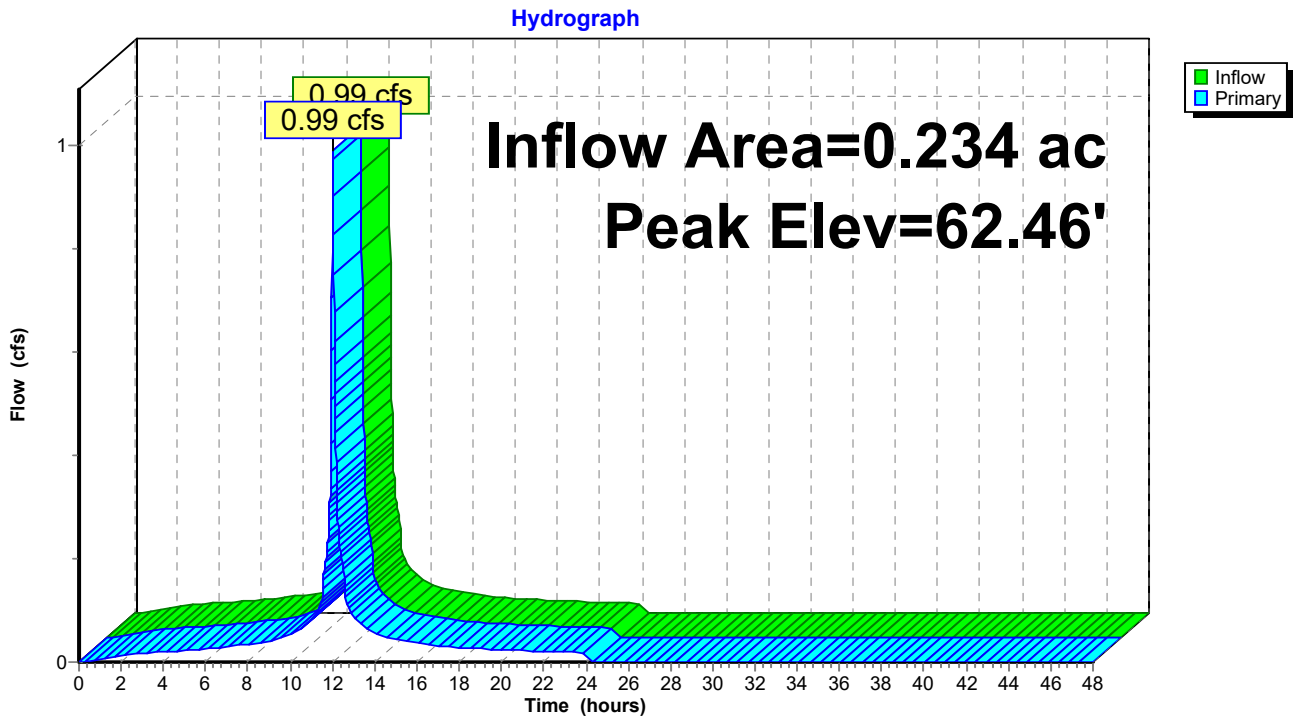
Inflow Area = 0.234 ac, 99.36% Impervious, Inflow Depth = 4.67" for 10-yr event
 Inflow = 0.99 cfs @ 12.03 hrs, Volume= 0.091 af
 Outflow = 0.99 cfs @ 12.03 hrs, Volume= 0.091 af, Atten= 0%, Lag= 0.0 min
 Primary = 0.99 cfs @ 12.03 hrs, Volume= 0.091 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3
 Peak Elev= 62.46' @ 12.03 hrs
 Flood Elev= 65.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	61.30'	12.0" Round 12" HDPE Pipe L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 61.30' / 61.10' S= 0.0050 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Device 1	61.46'	8.0" Vert. 8" Oriface C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.99 cfs @ 12.03 hrs HW=62.45' TW=62.11' (Dynamic Tailwater)
 1=12" HDPE Pipe (Passes 0.99 cfs of 1.75 cfs potential flow)
 2=8" Oriface (Orifice Controls 0.99 cfs @ 2.82 fps)

Pond PP57: Proposed Catch Basin at Site Entrance

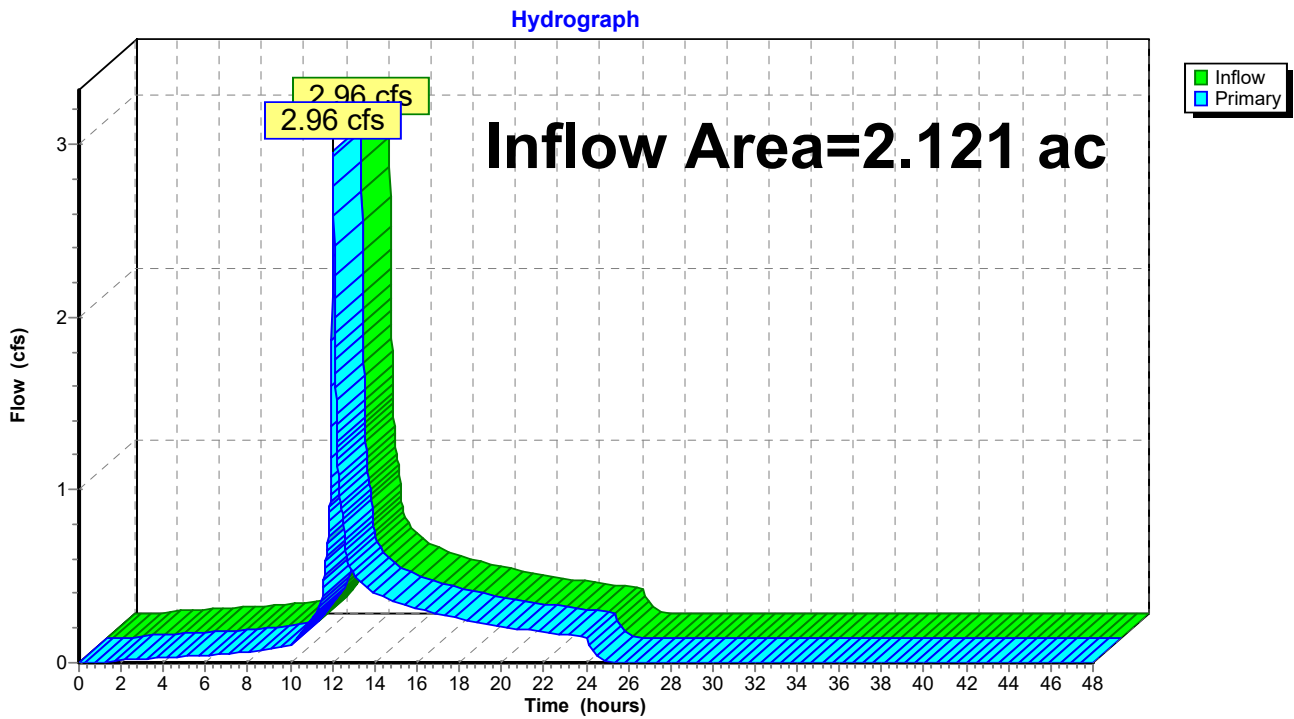


Summary for Link L101: Flow North Along Golfcourse to 9th Hole Pond

Inflow Area = 2.121 ac, 47.37% Impervious, Inflow Depth = 2.39" for 10-yr event
Inflow = 2.96 cfs @ 12.04 hrs, Volume= 0.422 af
Primary = 2.96 cfs @ 12.04 hrs, Volume= 0.422 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link L101: Flow North Along Golfcourse to 9th Hole Pond

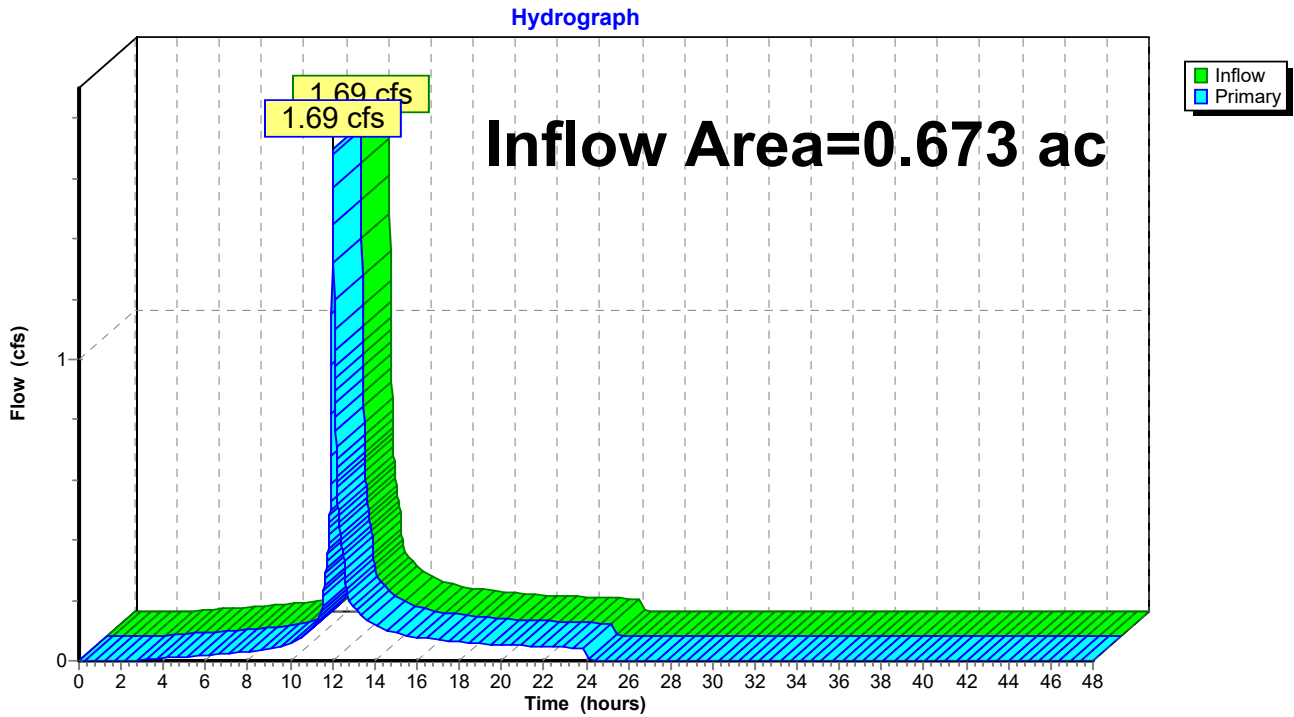


Summary for Link L102: Flow to Catch Basin on Jady Hill Avenue

Inflow Area = 0.673 ac, 37.33% Impervious, Inflow Depth = 2.59" for 10-yr event
Inflow = 1.69 cfs @ 12.03 hrs, Volume= 0.145 af
Primary = 1.69 cfs @ 12.03 hrs, Volume= 0.145 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link L102: Flow to Catch Basin on Jady Hill Avenue

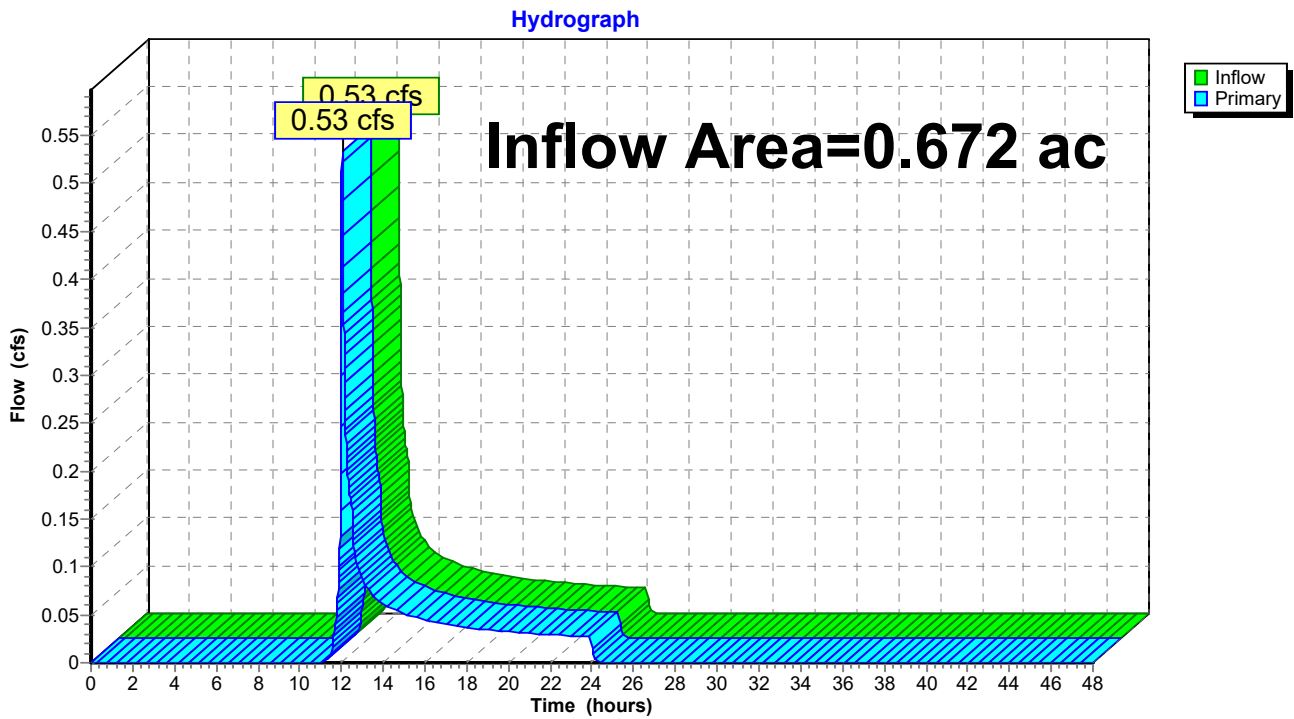


Summary for Link L103: Flow South to Hayes Park

Inflow Area = 0.672 ac, 0.00% Impervious, Inflow Depth = 1.00" for 10-yr event
Inflow = 0.53 cfs @ 12.04 hrs, Volume= 0.056 af
Primary = 0.53 cfs @ 12.04 hrs, Volume= 0.056 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link L103: Flow South to Hayes Park

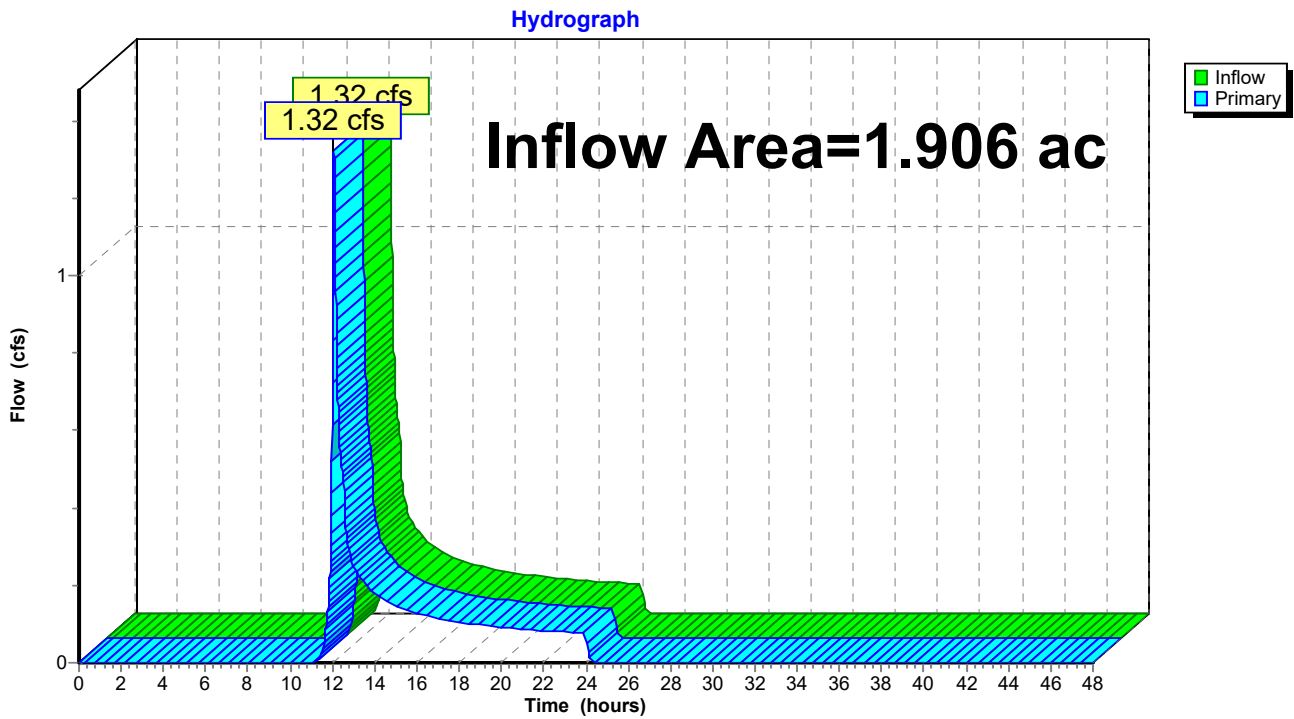


Summary for Link L104: Flows West towards Squamscott River

Inflow Area = 1.906 ac, 0.00% Impervious, Inflow Depth = 1.00" for 10-yr event
Inflow = 1.32 cfs @ 12.06 hrs, Volume= 0.158 af
Primary = 1.32 cfs @ 12.06 hrs, Volume= 0.158 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link L104: Flows West towards Squamscott River



Postdevelopment HydroCAD 07-03- NH-Exeter 08-23-22 24-hr S1 1-yr 1-inch Rainfall=1.00"

Prepared by {enter your company name here}

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Page 41

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=0.32"
Tc=5.0 min CN=90 Runoff=0.05 cfs 0.004 af

Subcatchment PS25B: Southside of Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=0.50"
Tc=5.0 min CN=94 Runoff=0.05 cfs 0.004 af

Subcatchment PS26: Northern Portion of Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=0.00"
Flow Length=235' Tc=7.1 min CN=62 Runoff=0.00 cfs 0.000 af

Subcatchment PS27A: Existing 1st Hole Tee Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=0.00"
Tc=5.0 min CN=65 Runoff=0.00 cfs 0.000 af

Subcatchment PS27B: Existing Northern Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=0.79"
Tc=5.0 min CN=98 Runoff=0.20 cfs 0.015 af

Subcatchment PS27C: Portion of Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=0.40"
Tc=5.0 min CN=92 Runoff=0.13 cfs 0.009 af

Subcatchment PS28: Middle Strip of Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=0.63"
Tc=5.0 min CN=96 Runoff=0.12 cfs 0.009 af

Subcatchment PS29: Southern Portion of Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=0.17"
Tc=5.0 min CN=85 Runoff=0.01 cfs 0.001 af

Subcatchment PS30: Woods to the South of Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=0.00"
Flow Length=233' Tc=5.0 min CN=56 Runoff=0.00 cfs 0.000 af

Subcatchment PS31: Woods to the West of Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=0.00"
Flow Length=345' Tc=7.4 min CN=56 Runoff=0.00 cfs 0.000 af

Subcatchment PS32: Proposed Parking Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=0.15"
Tc=5.0 min CN=84 Runoff=0.06 cfs 0.007 af

Reach PR75: Proposed Swale Running Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af
n=0.022 L=90.0' S=0.0944 '/' Capacity=37.88 cfs Outflow=0.00 cfs 0.000 af

Pond EP10: 9th Hole Pond Peak Elev=36.54' Storage=1,056 cf Inflow=0.34 cfs 0.024 af
Outflow=0.00 cfs 0.000 af

Pond PP50: Proposed Stone Storage Under Peak Elev=77.03' Storage=24 cf Inflow=0.05 cfs 0.004 af
Discarded=0.01 cfs 0.004 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.004 af

Pond PP51: Proposed Drip Edge in front of Peak Elev=80.09' Storage=15 cf Inflow=0.05 cfs 0.004 af
Discarded=0.00 cfs 0.002 af Primary=0.03 cfs 0.001 af Outflow=0.03 cfs 0.004 af

Pond PP52A: Proposed Manhole in 1st Hole Fairway Peak Elev=58.69' Inflow=0.34 cfs 0.024 af
12.0" Round Culvert n=0.010 L=265.0' S=0.0464 '/' Outflow=0.34 cfs 0.024 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=60.58' Inflow=0.34 cfs 0.024 af
12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=0.34 cfs 0.024 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=61.37' Storage=29 cf Inflow=0.34 cfs 0.026 af
Discarded=0.00 cfs 0.001 af Primary=0.34 cfs 0.024 af Secondary=0.00 cfs 0.000 af Outflow=0.34 cfs 0.026 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=63.23' Inflow=0.01 cfs 0.001 af
Outflow=0.01 cfs 0.001 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=73.47' Storage=104 cf Inflow=0.08 cfs 0.009 af
Discarded=0.01 cfs 0.009 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.009 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=76.31' Storage=9 cf Inflow=0.09 cfs 0.009 af
Primary=0.08 cfs 0.009 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=0.08 cfs 0.009 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=61.71' Inflow=0.20 cfs 0.015 af
Outflow=0.20 cfs 0.015 af

Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=0.34 cfs 0.024 af
Primary=0.34 cfs 0.024 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=0.13 cfs 0.009 af
Primary=0.13 cfs 0.009 af

Link L103: Flow South to Hayes Park Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Link L104: Flows West towards Squamscott River Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Total Runoff Area = 5.372 ac Runoff Volume = 0.050 af Average Runoff Depth = 0.11"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of	Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=2.19" Tc=5.0 min CN=90 Runoff=0.39 cfs 0.028 af
Subcatchment PS25B: Southside of	Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=2.56" Tc=5.0 min CN=94 Runoff=0.24 cfs 0.018 af
Subcatchment PS26: Northern Portion of	Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=0.49" Flow Length=235' Tc=7.1 min CN=62 Runoff=0.25 cfs 0.032 af
Subcatchment PS27A: Existing 1st Hole Tee	Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=0.61" Tc=5.0 min CN=65 Runoff=0.21 cfs 0.020 af
Subcatchment PS27B: Existing Northern	Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=2.99" Tc=5.0 min CN=98 Runoff=0.74 cfs 0.058 af
Subcatchment PS27C: Portion of	Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=2.37" Tc=5.0 min CN=92 Runoff=0.77 cfs 0.055 af
Subcatchment PS28: Middle Strip of	Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=2.77" Tc=5.0 min CN=96 Runoff=0.52 cfs 0.039 af
Subcatchment PS29: Southern Portion of	Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=1.77" Tc=5.0 min CN=85 Runoff=0.19 cfs 0.014 af
Subcatchment PS30: Woods to the South of	Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=0.29" Flow Length=233' Tc=5.0 min CN=56 Runoff=0.05 cfs 0.016 af
Subcatchment PS31: Woods to the West of	Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=0.29" Flow Length=345' Tc=7.4 min CN=56 Runoff=0.14 cfs 0.045 af
Subcatchment PS32: Proposed Parking	Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=1.70" Tc=5.0 min CN=84 Runoff=1.18 cfs 0.084 af
Reach PR75: Proposed Swale Running	Avg. Flow Depth=0.12' Max Vel=3.09 fps Inflow=0.14 cfs 0.071 af n=0.022 L=90.0' S=0.0944 '/' Capacity=37.88 cfs Outflow=0.14 cfs 0.071 af
Pond EP10: 9th Hole Pond	Peak Elev=37.93' Storage=9,260 cf Inflow=1.65 cfs 0.213 af Outflow=0.00 cfs 0.000 af
Pond PP50: Proposed Stone Storage Under	Peak Elev=77.61' Storage=526 cf Inflow=0.39 cfs 0.028 af Discarded=0.02 cfs 0.028 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.028 af
Pond PP51: Proposed Drip Edge in front of	Peak Elev=80.26' Storage=42 cf Inflow=0.24 cfs 0.018 af Discarded=0.00 cfs 0.005 af Primary=0.22 cfs 0.013 af Outflow=0.22 cfs 0.018 af
Pond PP52A: Proposed Manhole in 1st Hole Fairway	Peak Elev=59.04' Inflow=1.45 cfs 0.180 af 12.0" Round Culvert n=0.010 L=265.0' S=0.0464 '/' Outflow=1.45 cfs 0.180 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=60.99' Inflow=1.45 cfs 0.180 af
 12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=1.45 cfs 0.180 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=61.84' Storage=45 cf Inflow=1.45 cfs 0.182 af
 Discarded=0.00 cfs 0.002 af Primary=1.45 cfs 0.180 af Secondary=0.00 cfs 0.000 af Outflow=1.45 cfs 0.182 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=63.41' Inflow=0.19 cfs 0.084 af
 Outflow=0.19 cfs 0.084 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=74.32' Storage=607 cf Inflow=0.15 cfs 0.097 af
 Discarded=0.01 cfs 0.026 af Primary=0.14 cfs 0.071 af Outflow=0.15 cfs 0.097 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=77.38' Storage=1,273 cf Inflow=1.39 cfs 0.097 af
 Primary=0.15 cfs 0.097 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=0.15 cfs 0.097 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=62.05' Inflow=0.74 cfs 0.058 af
 Outflow=0.74 cfs 0.058 af

Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=1.65 cfs 0.213 af
 Primary=1.65 cfs 0.213 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=0.97 cfs 0.075 af
 Primary=0.97 cfs 0.075 af

Link L103: Flow South to Hayes Park Inflow=0.05 cfs 0.016 af
 Primary=0.05 cfs 0.016 af

Link L104: Flows West towards Squamscott River Inflow=0.14 cfs 0.045 af
 Primary=0.14 cfs 0.045 af

Total Runoff Area = 5.372 ac Runoff Volume = 0.410 af Average Runoff Depth = 0.92"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of	Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=3.79" Tc=5.0 min CN=90 Runoff=0.58 cfs 0.048 af
Subcatchment PS25B: Southside of	Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=4.22" Tc=5.0 min CN=94 Runoff=0.34 cfs 0.030 af
Subcatchment PS26: Northern Portion of	Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=1.38" Flow Length=235' Tc=7.1 min CN=62 Runoff=0.90 cfs 0.092 af
Subcatchment PS27A: Existing 1st Hole Tee	Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=1.59" Tc=5.0 min CN=65 Runoff=0.60 cfs 0.052 af
Subcatchment PS27B: Existing Northern	Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=4.67" Tc=5.0 min CN=98 Runoff=0.99 cfs 0.091 af
Subcatchment PS27C: Portion of	Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=4.00" Tc=5.0 min CN=92 Runoff=1.10 cfs 0.093 af
Subcatchment PS28: Middle Strip of	Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=4.44" Tc=5.0 min CN=96 Runoff=0.70 cfs 0.063 af
Subcatchment PS29: Southern Portion of	Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=3.28" Tc=5.0 min CN=85 Runoff=0.31 cfs 0.025 af
Subcatchment PS30: Woods to the South of	Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=1.00" Flow Length=233' Tc=5.0 min CN=56 Runoff=0.53 cfs 0.056 af
Subcatchment PS31: Woods to the West of	Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=1.00" Flow Length=345' Tc=7.4 min CN=56 Runoff=1.32 cfs 0.158 af
Subcatchment PS32: Proposed Parking	Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=3.19" Tc=5.0 min CN=84 Runoff=1.93 cfs 0.157 af
Reach PR75: Proposed Swale Running	Avg. Flow Depth=0.14' Max Vel=3.40 fps Inflow=0.20 cfs 0.152 af n=0.022 L=90.0' S=0.0944 '/' Capacity=37.88 cfs Outflow=0.20 cfs 0.152 af
Pond EP10: 9th Hole Pond	Peak Elev=38.79' Storage=18,366 cf Inflow=2.96 cfs 0.422 af Outflow=0.00 cfs 0.000 af
Pond PP50: Proposed Stone Storage Under	Peak Elev=78.30' Storage=1,119 cf Inflow=0.58 cfs 0.048 af Discarded=0.02 cfs 0.048 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.048 af
Pond PP51: Proposed Drip Edge in front of	Peak Elev=80.32' Storage=51 cf Inflow=0.34 cfs 0.030 af Discarded=0.00 cfs 0.005 af Primary=0.31 cfs 0.024 af Outflow=0.32 cfs 0.030 af
Pond PP52A: Proposed Manhole in 1st Hole Fairway	Peak Elev=59.21' Inflow=2.10 cfs 0.330 af 12.0" Round Culvert n=0.010 L=265.0' S=0.0464 '/' Outflow=2.10 cfs 0.330 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=61.20' Inflow=2.10 cfs 0.330 af
 12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=2.10 cfs 0.330 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=62.12' Storage=55 cf Inflow=2.11 cfs 0.331 af
 Discarded=0.00 cfs 0.002 af Primary=2.10 cfs 0.330 af Secondary=0.00 cfs 0.000 af Outflow=2.10 cfs 0.331 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=63.54' Inflow=0.42 cfs 0.178 af
 Outflow=0.42 cfs 0.178 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=74.37' Storage=637 cf Inflow=0.21 cfs 0.182 af
 Discarded=0.01 cfs 0.029 af Primary=0.20 cfs 0.152 af Outflow=0.21 cfs 0.182 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=78.11' Storage=2,667 cf Inflow=2.23 cfs 0.182 af
 Primary=0.21 cfs 0.182 af Secondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=0.21 cfs 0.182 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=62.46' Inflow=0.99 cfs 0.091 af
 Outflow=0.99 cfs 0.091 af

Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=2.96 cfs 0.422 af
 Primary=2.96 cfs 0.422 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=1.69 cfs 0.145 af
 Primary=1.69 cfs 0.145 af

Link L103: Flow South to Hayes Park Inflow=0.53 cfs 0.056 af
 Primary=0.53 cfs 0.056 af

Link L104: Flows West towards Squamscott River Inflow=1.32 cfs 0.158 af
 Primary=1.32 cfs 0.158 af

Total Runoff Area = 5.372 ac Runoff Volume = 0.865 af Average Runoff Depth = 1.93"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of	Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=5.08" Tc=5.0 min CN=90 Runoff=0.72 cfs 0.065 af
Subcatchment PS25B: Southside of	Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=5.53" Tc=5.0 min CN=94 Runoff=0.41 cfs 0.039 af
Subcatchment PS26: Northern Portion of	Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=2.26" Flow Length=235' Tc=7.1 min CN=62 Runoff=1.49 cfs 0.150 af
Subcatchment PS27A: Existing 1st Hole Tee	Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=2.53" Tc=5.0 min CN=65 Runoff=0.93 cfs 0.083 af
Subcatchment PS27B: Existing Northern	Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=6.00" Tc=5.0 min CN=98 Runoff=1.18 cfs 0.117 af
Subcatchment PS27C: Portion of	Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=5.31" Tc=5.0 min CN=92 Runoff=1.34 cfs 0.124 af
Subcatchment PS28: Middle Strip of	Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=5.77" Tc=5.0 min CN=96 Runoff=0.85 cfs 0.081 af
Subcatchment PS29: Southern Portion of	Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=4.53" Tc=5.0 min CN=85 Runoff=0.40 cfs 0.035 af
Subcatchment PS30: Woods to the South of	Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=1.74" Flow Length=233' Tc=5.0 min CN=56 Runoff=1.01 cfs 0.098 af
Subcatchment PS31: Woods to the West of	Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=1.74" Flow Length=345' Tc=7.4 min CN=56 Runoff=2.52 cfs 0.276 af
Subcatchment PS32: Proposed Parking	Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=4.42" Tc=5.0 min CN=84 Runoff=2.49 cfs 0.218 af
Reach PR75: Proposed Swale Running	Avg. Flow Depth=0.22' Max Vel=4.58 fps Inflow=0.66 cfs 0.220 af n=0.022 L=90.0' S=0.0944 '/' Capacity=37.88 cfs Outflow=0.66 cfs 0.220 af
Pond EP10: 9th Hole Pond	Peak Elev=39.37' Storage=26,174 cf Inflow=3.98 cfs 0.601 af Outflow=0.00 cfs 0.000 af
Pond PP50: Proposed Stone Storage Under	Peak Elev=78.96' Storage=1,683 cf Inflow=0.72 cfs 0.065 af Discarded=0.02 cfs 0.058 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.058 af
Pond PP51: Proposed Drip Edge in front of	Peak Elev=80.36' Storage=57 cf Inflow=0.41 cfs 0.039 af Discarded=0.00 cfs 0.005 af Primary=0.39 cfs 0.033 af Outflow=0.39 cfs 0.039 af
Pond PP52A: Proposed Manhole in 1st Hole Fairway	Peak Elev=59.34' Inflow=2.52 cfs 0.451 af 12.0" Round Culvert n=0.010 L=265.0' S=0.0464 '/' Outflow=2.52 cfs 0.451 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=61.37' Inflow=2.52 cfs 0.451 af
 12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=2.52 cfs 0.451 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=62.66' Storage=75 cf Inflow=2.57 cfs 0.453 af
 Discarded=0.00 cfs 0.002 af Primary=2.52 cfs 0.451 af Secondary=0.00 cfs 0.000 af Outflow=2.52 cfs 0.453 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=63.70' Inflow=0.74 cfs 0.255 af
 Outflow=0.74 cfs 0.255 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=74.69' Storage=787 cf Inflow=0.70 cfs 0.252 af
 Discarded=0.02 cfs 0.032 af Primary=0.66 cfs 0.220 af Outflow=0.68 cfs 0.252 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=78.33' Storage=3,202 cf Inflow=2.86 cfs 0.252 af
 Primary=0.23 cfs 0.227 af Secondary=0.47 cfs 0.025 af Tertiary=0.00 cfs 0.000 af Outflow=0.70 cfs 0.252 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=63.14' Inflow=1.18 cfs 0.117 af
 Outflow=1.18 cfs 0.117 af

Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=3.98 cfs 0.601 af
 Primary=3.98 cfs 0.601 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=2.28 cfs 0.206 af
 Primary=2.28 cfs 0.206 af

Link L103: Flow South to Hayes Park Inflow=1.01 cfs 0.098 af
 Primary=1.01 cfs 0.098 af

Link L104: Flows West towards Squamscott River Inflow=2.52 cfs 0.276 af
 Primary=2.52 cfs 0.276 af

Total Runoff Area = 5.372 ac Runoff Volume = 1.285 af Average Runoff Depth = 2.87"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS25A: Northside of	Runoff Area=6,660 sf 78.53% Impervious Runoff Depth=6.30" Tc=5.0 min CN=90 Runoff=0.84 cfs 0.080 af
Subcatchment PS25B: Southside of	Runoff Area=3,655 sf 89.06% Impervious Runoff Depth=6.77" Tc=5.0 min CN=94 Runoff=0.48 cfs 0.047 af
Subcatchment PS26: Northern Portion of	Runoff Area=34,690 sf 1.51% Impervious Runoff Depth=3.17" Flow Length=235' Tc=7.1 min CN=62 Runoff=2.04 cfs 0.210 af
Subcatchment PS27A: Existing 1st Hole Tee	Runoff Area=17,120 sf 4.79% Impervious Runoff Depth=3.49" Tc=5.0 min CN=65 Runoff=1.25 cfs 0.114 af
Subcatchment PS27B: Existing Northern	Runoff Area=10,190 sf 99.36% Impervious Runoff Depth=7.25" Tc=5.0 min CN=98 Runoff=1.36 cfs 0.141 af
Subcatchment PS27C: Portion of	Runoff Area=12,175 sf 83.08% Impervious Runoff Depth=6.54" Tc=5.0 min CN=92 Runoff=1.56 cfs 0.152 af
Subcatchment PS28: Middle Strip of	Runoff Area=7,370 sf 93.55% Impervious Runoff Depth=7.01" Tc=5.0 min CN=96 Runoff=0.97 cfs 0.099 af
Subcatchment PS29: Southern Portion of	Runoff Area=4,035 sf 63.94% Impervious Runoff Depth=5.72" Tc=5.0 min CN=85 Runoff=0.47 cfs 0.044 af
Subcatchment PS30: Woods to the South of	Runoff Area=29,290 sf 0.00% Impervious Runoff Depth=2.54" Flow Length=233' Tc=5.0 min CN=56 Runoff=1.48 cfs 0.142 af
Subcatchment PS31: Woods to the West of	Runoff Area=83,020 sf 0.00% Impervious Runoff Depth=2.54" Flow Length=345' Tc=7.4 min CN=56 Runoff=3.73 cfs 0.404 af
Subcatchment PS32: Proposed Parking	Runoff Area=25,800 sf 58.76% Impervious Runoff Depth=5.61" Tc=5.0 min CN=84 Runoff=2.98 cfs 0.277 af
Reach PR75: Proposed Swale Running	Avg. Flow Depth=0.28' Max Vel=5.40 fps Inflow=1.27 cfs 0.285 af n=0.022 L=90.0' S=0.0944 ' Capacity=37.88 cfs Outflow=1.27 cfs 0.285 af
Pond EP10: 9th Hole Pond	Peak Elev=39.86' Storage=33,870 cf Inflow=5.35 cfs 0.778 af Outflow=0.00 cfs 0.000 af
Pond PP50: Proposed Stone Storage Under	Peak Elev=79.64' Storage=2,273 cf Inflow=0.84 cfs 0.080 af Discarded=0.02 cfs 0.062 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.062 af
Pond PP51: Proposed Drip Edge in front of	Peak Elev=80.39' Storage=62 cf Inflow=0.48 cfs 0.047 af Discarded=0.00 cfs 0.005 af Primary=0.45 cfs 0.042 af Outflow=0.45 cfs 0.047 af
Pond PP52A: Proposed Manhole in 1st Hole Fairway	Peak Elev=59.66' Inflow=3.30 cfs 0.567 af 12.0" Round Culvert n=0.010 L=265.0' S=0.0464 ' Outflow=3.30 cfs 0.567 af

Pond PP52B: Proposed Manhole near Cart Storage Peak Elev=63.00' Inflow=3.30 cfs 0.567 af
 12.0" Round Culvert n=0.012 L=420.0' S=0.0042 '/' Outflow=3.30 cfs 0.567 af

Pond PP53: Proposed Rock Infiltration Area in Peak Elev=63.57' Storage=112 cf Inflow=2.97 cfs 0.569 af
 Discarded=0.00 cfs 0.002 af Primary=3.30 cfs 0.567 af Secondary=0.00 cfs 0.000 af Outflow=3.31 cfs 0.569 af

Pond PP54: Proposed Catch Basin at Southernmost Site Peak Elev=64.20' Inflow=1.41 cfs 0.329 af
 Outflow=1.41 cfs 0.329 af

Pond PP55: Proposed Bioretention Area "A" Peak Elev=75.36' Storage=987 cf Inflow=1.52 cfs 0.319 af
 Discarded=0.02 cfs 0.034 af Primary=1.27 cfs 0.285 af Outflow=1.29 cfs 0.319 af

Pond PP56: Proposed Bioretention Area "A" Peak Elev=78.46' Storage=3,522 cf Inflow=3.41 cfs 0.319 af
 Primary=0.24 cfs 0.259 af Secondary=1.28 cfs 0.060 af Tertiary=0.00 cfs 0.000 af Outflow=1.52 cfs 0.319 af

Pond PP57: Proposed Catch Basin at Site Entrance Peak Elev=64.31' Inflow=1.36 cfs 0.141 af
 Outflow=1.36 cfs 0.141 af

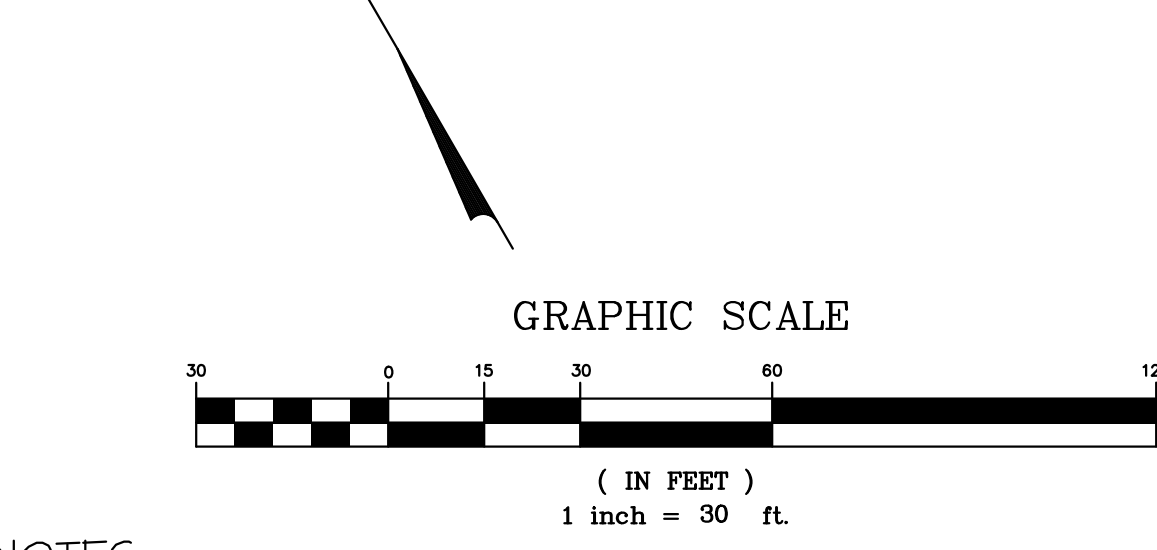
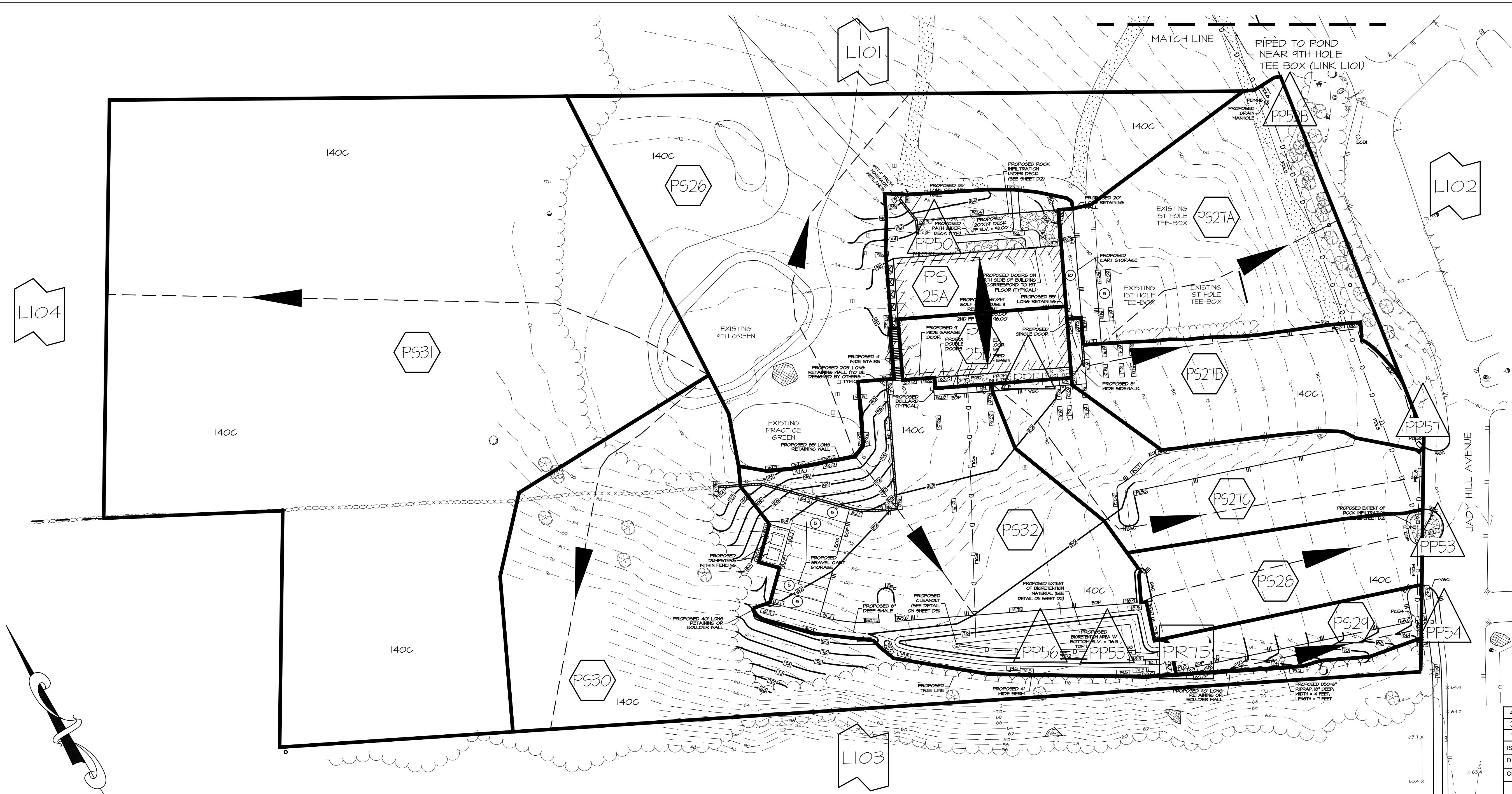
Link L101: Flow North Along Golfcourse to 9th Hole Pond Inflow=5.35 cfs 0.778 af
 Primary=5.35 cfs 0.778 af

Link L102: Flow to Catch Basin on Jady Hill Avenue Inflow=2.81 cfs 0.266 af
 Primary=2.81 cfs 0.266 af

Link L103: Flow South to Hayes Park Inflow=1.48 cfs 0.142 af
 Primary=1.48 cfs 0.142 af

Link L104: Flows West towards Squamscott River Inflow=3.73 cfs 0.404 af
 Primary=3.73 cfs 0.404 af

Total Runoff Area = 5.372 ac Runoff Volume = 1.712 af Average Runoff Depth = 3.82"
76.62% Pervious = 4.116 ac 23.38% Impervious = 1.256 ac



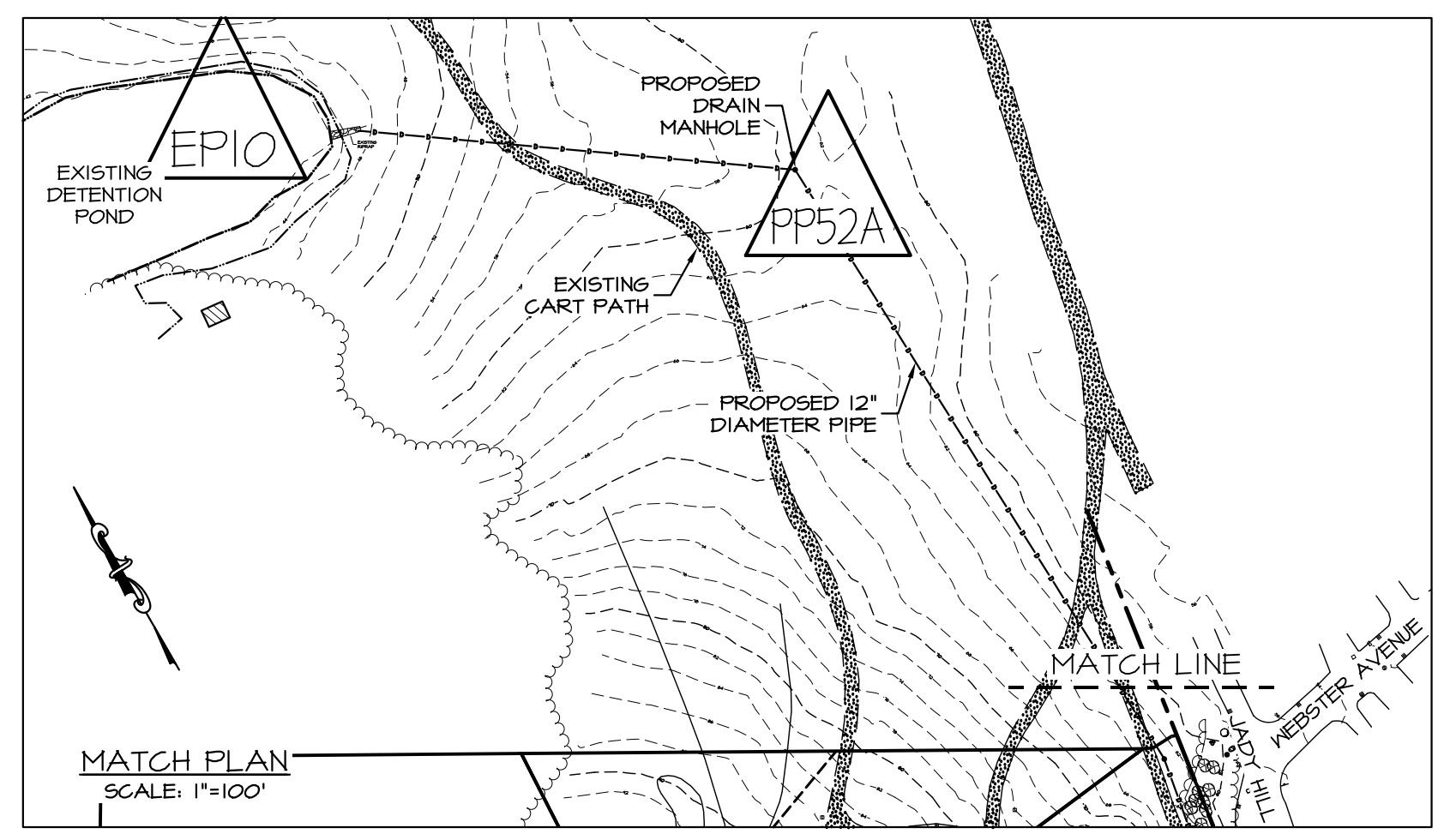
NOTES:

- OWNER OF RECORD:
TAX MAP 52, LOT 1
EXETER COUNTRY CLUB
P.O. BOX 1088
EXETER, NH 03833
RCRD BK1406 P60240
- THE INTENT OF THIS PLAN IS TO CALCULATE POST-DEVELOPMENT SUBCATCHMENT AREAS AND FLOW PATHS FOR MODELING VARIOUS STORM EVENTS IN PREPARATION FOR SITE IMPROVEMENTS.

IMPERVIOUS CALCULATIONS			
TOTAL AREA OF DRAINAGE ANALYSIS = 234,005 SF			
ITEM	EXISTING	PROPOSED	DIFFERENCE
TOTAL IMPERVIOUS AREA	48,015 SF	54,545 SF	11,530 SF (INCREASE)
TOTAL IMPERVIOUS %	20.5%	25.5%	5.0% (INCREASE)
UNTREATED IMPERVIOUS AREA	48,015 SF	34,060 SF	9,155 SF (DECREASE)
UNTREATED IMPERVIOUS %	20.5%	16.7%	3.8% (DECREASE)

SOIL LEGEND	
MAP UNIT SYMBOL	MAP UNIT NAME
140C	CHATFIELD-HOLLIS-CANTON COMPLEX, 8 TO 15 PERCENT SLOPES, ROCKY

NOTE: SOIL INFORMATION ON THIS PLAN WAS DELINEATED VIA THE USDA - NRCS WEB SOIL SURVEY ON AUGUST 23, 2022.



4	JULY 7, 2023	FOR APPROVAL	
3	MAY 23, 2023	FOR APPROVAL	
1	JAN 24, 2023	FOR APPROVAL	
ISS. DATE:	DESCRIPTION OF ISSUE:		CHK.
DRAWN:	JJM	DESIGN:	JJM
CHECKED:	BDS	CHECKED:	BDS



CLIENT:
BLIND TIGER, LLC
3 WRIGHT LANE
EXETER, NH 03833

TITLE:
POSTDEVELOPMENT SUBCATCHMENTS FOR EXETER COUNTRY CLUB 58 JADY HILL AVENUE (SITE) EXETER, NH 03833

PROJECT:	SCALE:	SHEET:
21-157	1"=30'	SW2